

BRENTWOOD BOULEVARD SPECIFIC PLAN

SCH# 2007082136

RECIRCULATED DRAFT ENVIRONMENTAL IMPACT REPORT

**VOLUME II OF III
APPENDICES D-F**

PREPARED FOR
THE CITY OF BRENTWOOD

OCTOBER 2011

PREPARED BY

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APPENDIX D

Traffic Impact Analysis

**Brentwood Boulevard Specific Plan
Brentwood, California**

April 11th, 2008

Prepared for:

The City of Brentwood, California

Prepared by:



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EXECUTIVE SUMMARY

This report documents the results of a traffic impact analysis completed for the Brentwood Boulevard Specific Plan (the “proposed project” or “project”), which consists of approximately 396 acres located along the Brentwood Boulevard corridor in the City of Brentwood, California. The purpose of this impact analysis is to identify potential environmental impacts to transportation facilities as required by the California Environmental Quality Act (CEQA).

The following intersections, listed with existing traffic control, are included in this traffic impact analysis:

1. Brentwood Boulevard @ Delta Road (Two-Way Stop Control)
2. Delta Road @ Sellers Avenue (All-Way Stop Control)
3. Brentwood Boulevard @ E. Sims Road (Two-Way Stop Control)
4. Brentwood Boulevard @ Lone Tree Way (South) (Signal)
5. Arroyo Seco Road @ Lone Tree Way (Two-Way Stop Control)
6. Brentwood Boulevard @ Sunrise Drive (Two-Way Stop Control)
7. Brentwood Boulevard @ Gregory Lane (Two-Way Stop Control)
8. Brentwood Boulevard @ Hanson Lane (Two-Way Stop Control)
9. Brentwood Boulevard @ Homecoming Way (Two-Way Stop Control)
10. Brentwood Boulevard @ Grant Street/Sunset Road (South) (Signal)
11. Brentwood Boulevard @ Sunset Court (Two-Way Stop Control)
12. Brentwood Boulevard @ Havenwood Avenue (Two-Way Stop Control)
13. Brentwood Boulevard @ Applewood Common (Signal)
14. Brentwood Boulevard @ Sand Creek Road (Signal)
15. Brentwood Boulevard @ Technology Way (South) (Signal)
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17. Brentwood Boulevard @ Village Drive (Two-Way Stop Control)
18. Brentwood Boulevard @ Central Boulevard/Sycamore Avenue (South) (Signal)
20. Second Street @ Central Street (Signal)
21. Central Boulevard @ Walnut Boulevard (Signal)
22. Brentwood Boulevard @ Pine Street (Two-Way Stop Control)
23. Brentwood Boulevard @ Maple Street (Two-Way Stop Control)
24. Brentwood Boulevard @ Oak Street (Signal)
25. Oak Street @ Walnut Boulevard (Signal)
26. Brentwood Boulevard @ Second Street (Signal)
27. Brentwood Boulevard @ Chestnut Street (Two-Way Stop Control)
28. Brentwood Boulevard @ Balfour Road (Signal)
29. Sellers Avenue @ Sunset Road (All-Way Stop Control)

The following roadway segments are also included in this traffic impact analysis:

1. Brentwood Boulevard between E. Sims Road and Delta Road
2. Brentwood Boulevard between Grant Street/Sunset Road and Sunset Court
3. Brentwood Boulevard between Central Boulevard and Spruce Street
4. Brentwood Boulevard between Oak Avenue and Pine Avenue
5. Delta Road west of Brentwood Boulevard
6. Lone Tree Way west of Brentwood Boulevard
7. Sunset Road east of Brentwood Boulevard
8. Central Boulevard west of Brentwood Boulevard
9. Grant Street west of Brentwood Boulevard
10. Sycamore Avenue east of Brentwood Boulevard

The level of service analysis was conducted for the study intersections for both weekday AM and PM peak-hours for the following scenarios:

- A. Existing (2007) Conditions
- B. Existing plus Approved Projects Conditions
- C. Existing plus Approved Projects plus Proposed Project Conditions
- D. Cumulative (2030) Conditions
- E. Cumulative plus Proposed Project Conditions

Significant findings of this study include:

- The proposed project is expected to generate new 31,202 daily trips, including 3,704 new AM peak-hour trips and 4,072 new PM peak-hour trips.
- The addition of the Proposed Project to the Existing plus Approved Projects Conditions results in a significant impact at fourteen study intersections and one study roadway segment. These impacts can be mitigated to be *less than significant*.
- The addition of the Proposed Project to the Cumulative Conditions does not result in any significant impacts.
- The addition of the Proposed Project to Existing plus Approved Projects and Cumulative Conditions is not expected to result in any significant impacts to bicycle, pedestrian, or transit facilities.

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POTW – Publicly Owned Treatment Works
PPM – Parts Per Million
PS – Pump Station
PSIG – Pounds Per Square Inch Gauge
RBWTP – Randall-Bold Water Treatment Plant
RCRA – Resource Conservation and Recovery Act
RMPP – Risk Management and Prevention Plan
ROG – Reactive Organic Gases
ROW – Right-of-Way
RPS – Renewable Portfolio Standard
SB – Senate Bill
SCH – State Clearinghouse
SDWA – Safe Water Drinking Act
SEL – Sound Exposure Level
SHPO – State Historic Preservation Office
SJVAB – San Joaquin Valley Air Basin
SLIC – Spills, Leaks, Investigations, and Cleanup
SOI – Sphere of Influence
SO_x – Sulfates
SR – State Route
TAC – Toxic Air Contaminants
TAZ – Traffic Analysis Zone
TDT – Tri Delta Transit
TSO – Traffic Service Objective
TSS – Total Suspended Solids
TWSC – Two-Way Stop-Controlled
UBC – Uniform Building Code
UPRR – Union Pacific Railroad
USACE – United States Army Corps of Engineers
USEPA – United States Environmental Protection Agency
UST – Underground Storage Tank
UWMP – Urban Water Management Plan
WWTP – Wastewater Treatment Plant

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INTRODUCTION

This report documents the results of a traffic impact analysis completed for the Brentwood Boulevard Specific Plan (the “proposed project” or “project”), which consists of approximately 396 acres located along the Brentwood Boulevard corridor in the City of Brentwood, California. The purpose of this impact analysis is to identify potential environmental impacts to transportation facilities as required by the California Environmental Quality Act (CEQA).

The remaining sections of this report document the proposed project, analysis methodologies, impacts and mitigation, and general study conclusions.

PROJECT DESCRIPTION

The proposed project consists of approximately 396 acres located along the Brentwood Boulevard corridor in the City of Brentwood. The project location is shown in Figure 1.

The following intersections, listed with existing traffic control, are included in this traffic impact analysis¹:

1. Brentwood Boulevard @ Delta Road (Two-Way Stop Control)
2. Delta Road @ Sellers Avenue (All-Way Stop Control)
3. Brentwood Boulevard @ E. Sims Road (Two-Way Stop Control)
4. Brentwood Boulevard @ Lone Tree Way (South) (Signal)
5. Arroyo Seco Road @ Lone Tree Way (Two-Way Stop Control)
6. Brentwood Boulevard @ Sunrise Drive (Two-Way Stop Control)
7. Brentwood Boulevard @ Gregory Lane (Two-Way Stop Control)
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28. Brentwood Boulevard @ Balfour Road (Signal)
29. Sellers Avenue @ Sunset Road (All-Way Stop Control)

¹ Per email from Winston Rhodes, City of Brentwood, May 21, 2007.

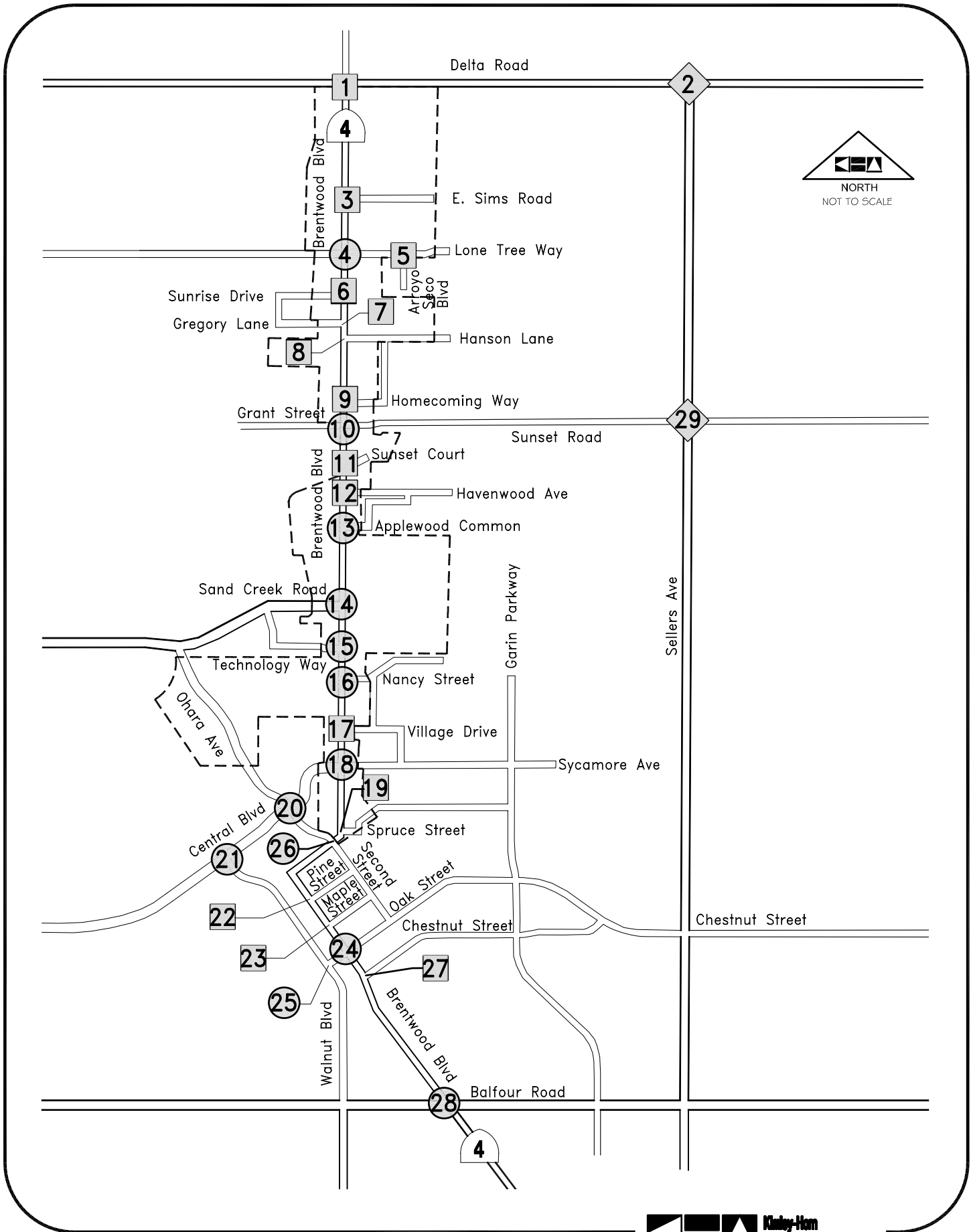


FIGURE 1
 PROJECT LOCATION AND STUDY INTERSECTIONS

The following roadway segments are also included in this traffic impact analysis:

1. Brentwood Boulevard between E. Sims Road and Delta Road
2. Brentwood Boulevard between Grant Street/Sunset Road and Sunset Court
3. Brentwood Boulevard between Central Boulevard and Spruce Street
4. Brentwood Boulevard between Oak Avenue and Pine Avenue
5. Delta Road west of Brentwood Boulevard
6. Lone Tree Way west of Brentwood Boulevard
7. Sunset Road east of Brentwood Boulevard
8. Central Boulevard west of Brentwood Boulevard
9. Grant Street west of Brentwood Boulevard
10. Sycamore Avenue east of Brentwood Boulevard

Figure 2 illustrates the study intersections, existing traffic control, and existing lane configurations.

PROJECT AREA TRANSPORTATION FACILITIES

Roadways

The following are descriptions of the primary roadways in the vicinity of the project.

Brentwood Boulevard is a north-south arterial roadway which traverses through the project area. Within the City, Brentwood Boulevard is also designated as State Route 4 (SR-4) which is a State highway that provides connectivity between communities in the Sierra Nevada Mountains, the San Joaquin Valley, and the San Francisco Bay Area. As a part of SR-4, Brentwood Boulevard is a primary truck route through east Contra Costa County. Within the project area, Brentwood Boulevard is generally a two-lane roadway without paved shoulders or on-street parking. South of Applewood Common, there are a number of locations that have been widened to include four-lanes, sidewalks, a landscaped median, and left turn bays. The roadway is fronted by a large variety of commercial and industrial uses with numerous driveways to adjacent parcels. Within the general project area, Brentwood Boulevard currently serves approximately 20,000 vehicles per day² (vpd).

Sellers Avenue is a north-south rural collector roadway that parallels Brentwood Boulevard to the east. Sellers Avenue terminates at the Contra Costa Canal to the north and Marsh Creek Road to the south. Through the project area, this roadway accommodates approximately 5,000 vpd³ and generally provides one travel lane in each direction.

Delta Road is an east-west collector roadway located immediately north end of the project area within the City of Oakley. Delta Road becomes Neroly Road west of Brentwood Boulevard (SR-4) and terminates at Holland Tract Road to the east. Delta Road accommodates approximately 4,200 vpd⁴ east of Brentwood Boulevard with one travel lane in each direction.

Lone Tree Way is an arterial roadway west of Brentwood Boulevard located at the north end of the project area. Lone Tree Way runs north-south from SR-4 in Antioch to James Donlon Boulevard, where it then becomes an east-west roadway until it terminates just east of Brentwood Boulevard. This roadway provides regional connectivity from the project area to the City of Antioch and is fronted by, and provides direct access to, many commercial land uses. Through the project area, Lone Tree Way accommodates approximately 10,800 vpd⁴ and is planned for widening to four lanes from O'Hara Avenue to Brentwood Boulevard.

² Caltrans Traffic and Vehicle Data Systems Unit, <http://www.dot.ca.gov/hq/traffops/saferesr/trafdata/2005all.htm>.

³ City of Brentwood, Traffic Volume Counts,
<http://www.ci.brentwood.ca.us/departement/pw/engineering/traffic/trafficalming.cfm#count>.

⁴ Kimley-Horn and Associates, Inc.

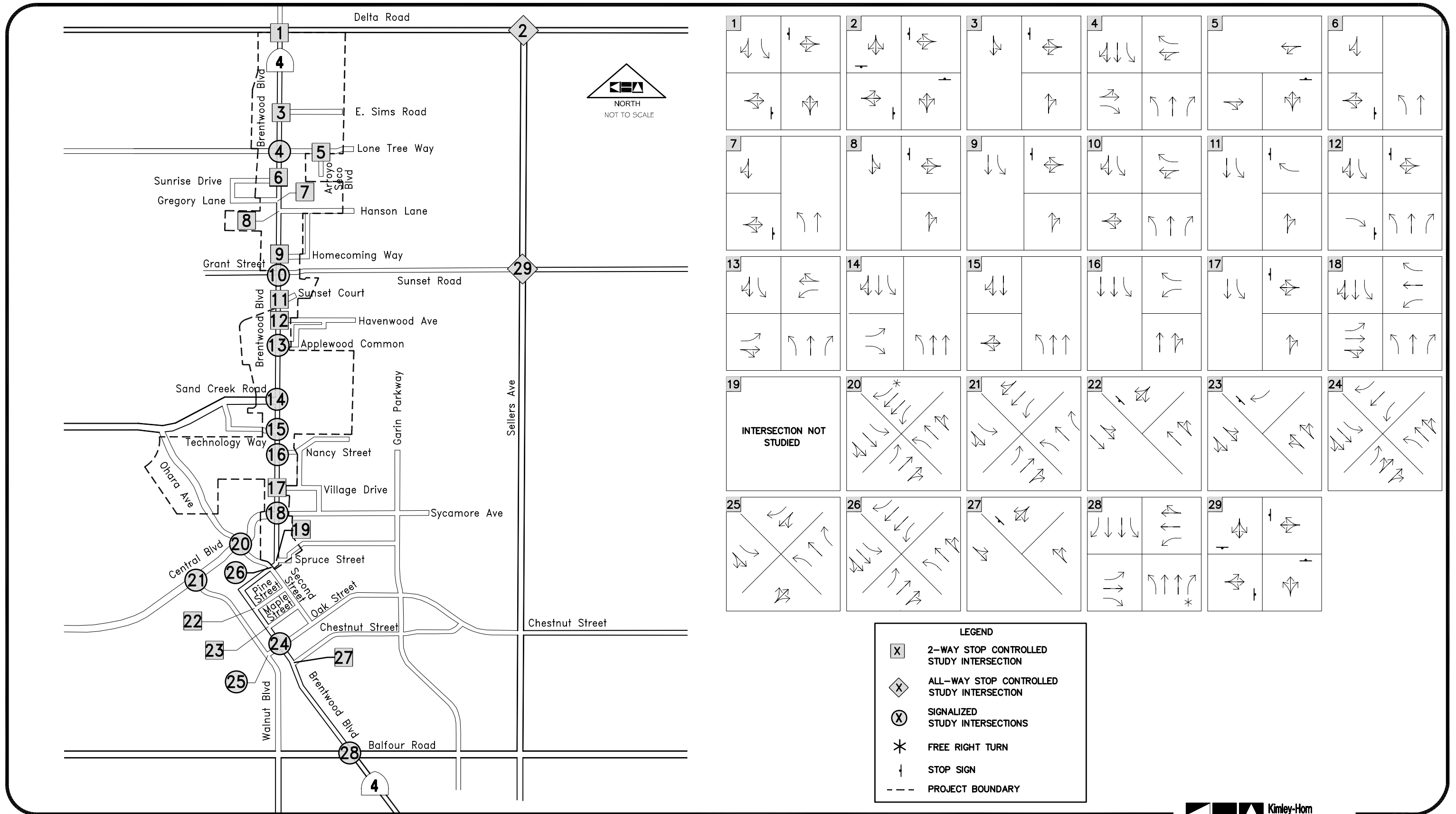


FIGURE 2
EXISTING STUDY INTERSECTION GEOMETRY AND TRAFFIC CONTROL

Sand Creek Road is an east-west collector roadway which connects the SR-4 Bypass to the west and Brentwood Boulevard to the east. Sand Creek Road accommodates approximately 8,100 vpd⁵ within the project area while providing one travel lane in each direction. Sand Creek Road is planned to be extended from Brentwood Boulevard to Sellers Avenue. The City has a development agreement in place for the near-term construction of Sand Creek Road from Brentwood Boulevard to Garin Parkway.

Central Boulevard is an east-west collector roadway which extends from Apple Hill Drive (West) to the west and Brentwood Boulevard to the east, where it becomes Sycamore Avenue. West of Apple Hill Drive (East), Central Boulevard becomes a two lane local road. However, to the east, it is a four lane divided roadway which provides direct access to shopping centers and other commercial uses.

Balfour Road is an east-west arterial roadway located south of the project area. Balfour Road terminates at Deer Valley Road to the west and becomes Fallman Boulevard east of Bixler Road, providing access from downtown Brentwood to/from unincorporated areas within Contra Costa County. Balfour Road accommodates approximately 10,900 vpd⁵ east of Brentwood Boulevard and approximately 30,100 vpd west of Brentwood Boulevard.

State Route 4 Bypass (Bypass) is a roadway construction project that will replace the existing State Route 4 from just south of the Main Street Interchange in the City of Antioch to the existing intersection with Marsh Creek Road. Phase 1 of the project is being constructed in three segments. Segment 1 extends from just east of the SR4/Hillcrest Avenue Interchange in the City of Antioch to Lone Tree Way and will consist of a 6-lane freeway between existing SR4 and the Laurel Road Interchange and a 4-lane freeway from Laurel Road to Lone Tree Way. Phase 1 of Segment 2 has been constructed as a 2-lane Expressway from Lone Tree Way to Balfour Road and will eventually be converted to a full freeway with interchanges at Sand Creek Road and Balfour Road. Segment 3 will extend from Balfour Road south to Marsh Creek Road as a 2-lane Expressway, then along Marsh Creek Road (East-West Connector) as a 2-lane conventional highway, connecting to existing SR4 (Byron Highway). The southerly limits of the project (now called the Vasco Road Extension) are from Marsh Creek Road to Vasco Road at Walnut Boulevard⁶.

Other roadways addressed in this analysis (i.e. Gregory Lane, Nancy Street, etc.) are local roads that provide connectivity from Brentwood Boulevard to surrounding neighborhoods and/or other portions of the City.

Existing Bicycle and Pedestrian Facilities

The project area generally lacks pedestrian and bicycle facilities, especially in the northern portions along Brentwood Boulevard. In many locations, bicyclists and pedestrians must use the shoulder of the roadway, which varies in width. A notable exception is the portion of Brentwood Boulevard between Applewood Common and Woodfield Lane. This portion of the roadway is improved with Class II bike lanes and sidewalks. In addition, Class I bike trails maintained by East Bay Regional Park District exist along the Marsh Creek Trail and the East Bay Municipal Utility District's Mokelumne Aqueduct right-of-way.

Transit Facilities

Transit service in the City of Brentwood is provided by Tri-Delta Transit (TDT). Tri Delta Transit currently operates 6 local routes (383, 384, 385, 386, 391 & Dimes-A-ride) and 2 express commuter routes (300, DX) on weekdays and a single local route (393) on weekends that serve the City of Brentwood. Two of the six local routes are trunk line services that operate on the SR4/Brentwood Boulevard corridor through Brentwood⁷. Bus routes in the vicinity of the project are illustrated in Figure 3.

⁵ City of Brentwood, Traffic Volume Counts,
<http://www.ci.brentwood.ca.us/department/pw/engineering/traffic/trafficalming.cfm#count>.

⁶ SR4 Bypass Authority Project Information website. Available: http://www.sr4bypass.org/Information/Projectinfo_general.htm

⁷ Email from Tom Harais to Kimley-Horn and Associates, November 20, 2007.

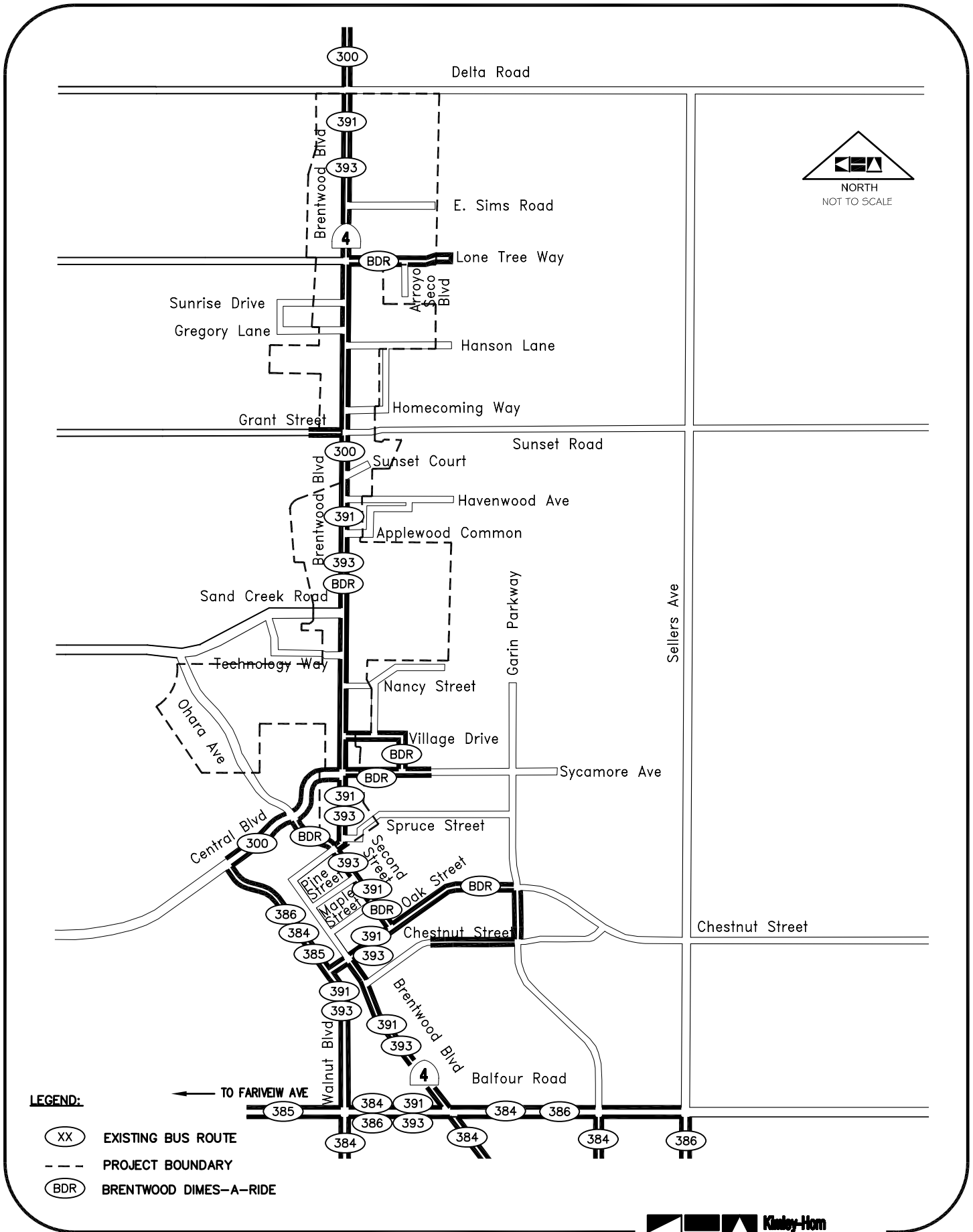


FIGURE 3
EXISTING TRANSIT SERVICE

ASSESSMENT OF PROPOSED PROJECT

Proposed Project Trip Generation

The number of trips anticipated to be generated by the proposed project were derived using data included in *Trip Generation, 7th Edition*, and the *Trip Generation Handbook, Second Edition*, both published by the Institute of Transportation Engineers (ITE). ITE land uses were selected to best represent the land uses specified in the Brentwood Boulevard Specific Plan. Where applicable, the regression equation method was utilized for trip generation calculations per the guidelines recommended by ITE⁸. The anticipated trip generation for this project approved by the City⁹ is shown in Table 1.

Table 1 – Proposed Project Trip Generation

ITE Land Use (Code)	Size	Total Daily Trips	AM Peak Hour				PM Peak Hour					
			Total Trips	IN		OUT		Total Trips	IN		OUT	
				%	Trips	%	Trips		%	Trips	%	Trips
General Light Industrial (110)	539.7 ksf	3,930	548	88%	482	12%	66	608	12%	73	88%	535
Single Family-Detached Housing (210)	421 DU	3,902	304	25%	76	75%	228	391	63%	246	37%	145
Apartment (220)	1,825 DU	11,119	898	20%	180	80%	718	1,021	65%	664	35%	357
City Park (411)	8.1 ac	13										
General Office Building (710)	1,940.7 ksf	13,089	2,011	88%	1,770	12%	241	2,252	17%	383	83%	1,869
Shopping Center (820)	381.4 ksf	16,212	349	61%	213	39%	136	1,515	48%	727	52%	788
Subtotal New Trips:		48,264	4,111		2,721		1,390	5,788		2,093		3,695
Internal Reduction ¹ (PM, Daily)	14% 17%	-8,171						-796		-288		-508
Pass-By Reduction ¹ (LU 820)	34%	-5,512	-119		-72		-46	-515		-247		-268
Alternate Mode Reduction	7%	-3,379	-288		-190		-97	-405		-147		-259
Net New External Trips:		31,202	3,704		2,458		1,246	4,072		1,412		2,660

Source: *Trip Generation, 7th Edition*, ITE.
¹ Based on methodology published in *Trip Generation Handbook, Second Edition*, ITE.

As shown in Table 1, the proposed project land uses are estimated to generate 31,202 total new daily trips with 3,704 new trips occurring during the AM peak-hour and 4,072 new trips occurring during the PM peak-hour.

Project Trip Distribution

The distribution of project generated traffic for Existing plus Proposed Project Conditions was developed based on Base Year (2000) output from the regional travel demand model provided by the Contra Costa Transportation Authority (CCTA), existing traffic data, and general knowledge of project area traffic patterns. The project trip distribution percentages were approved by the City¹⁰ and are illustrated in Figure 4. The resulting AM and PM peak-hour traffic volumes attributed to the proposed project at the study area intersections are illustrated in Figure 5.

Project trips for the Cumulative Plus proposed Project conditions were distributed using future year (2030) output from the CCTA travel demand model. The model employs a methodology that matches trip generators and trip attractors over a wide region and includes the effects of the SR-4 Bypass.

⁸ *Trip Generation Handbook, Second Edition*, ITE.

⁹ Email from Winston Rhodes, City of Brentwood, October 9, 2007.

¹⁰ Per email from Winston Rhodes, City of Brentwood, October 9, 2007.

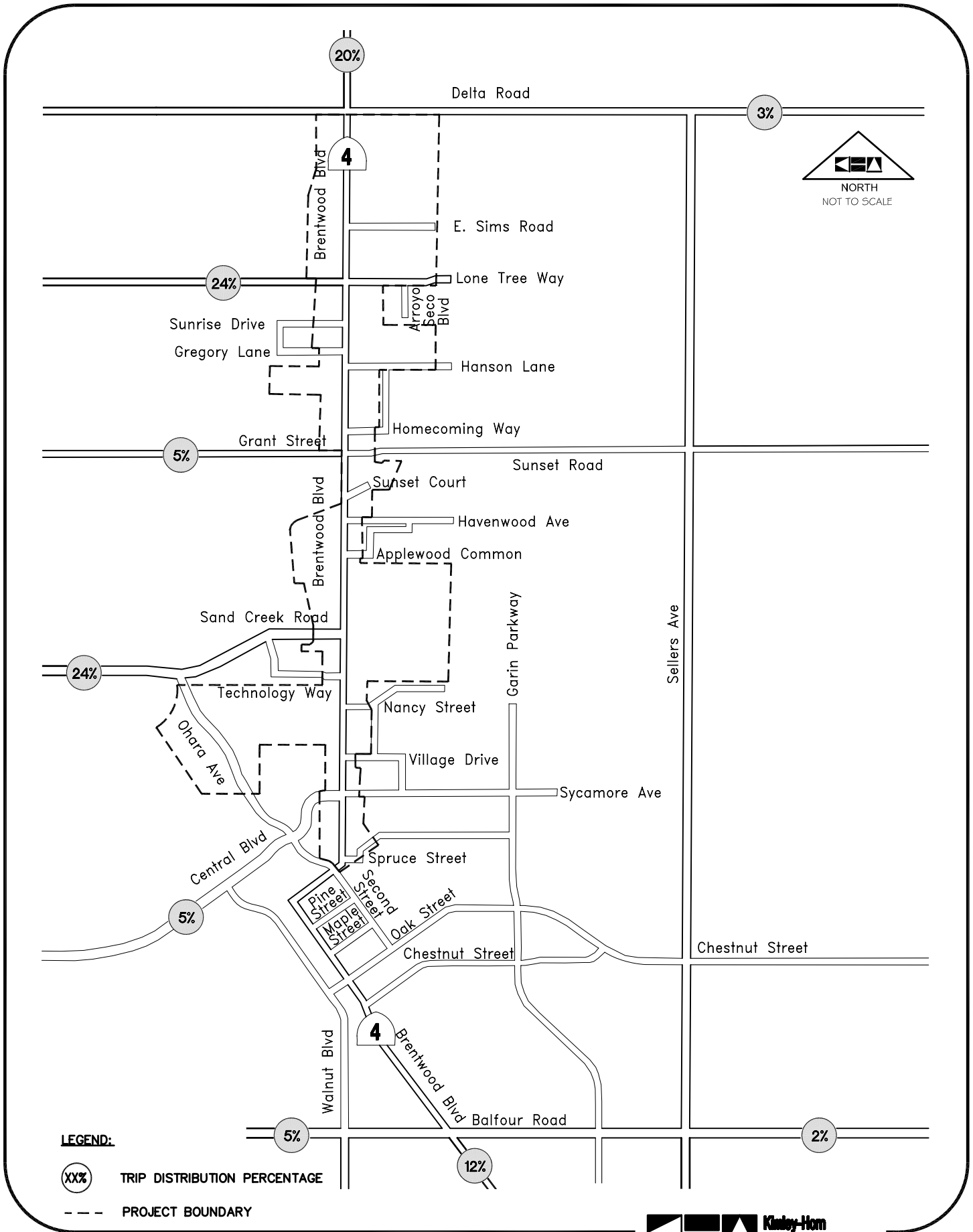


FIGURE 4
DISTRIBUTION FOR PROPOSED PROJECTS

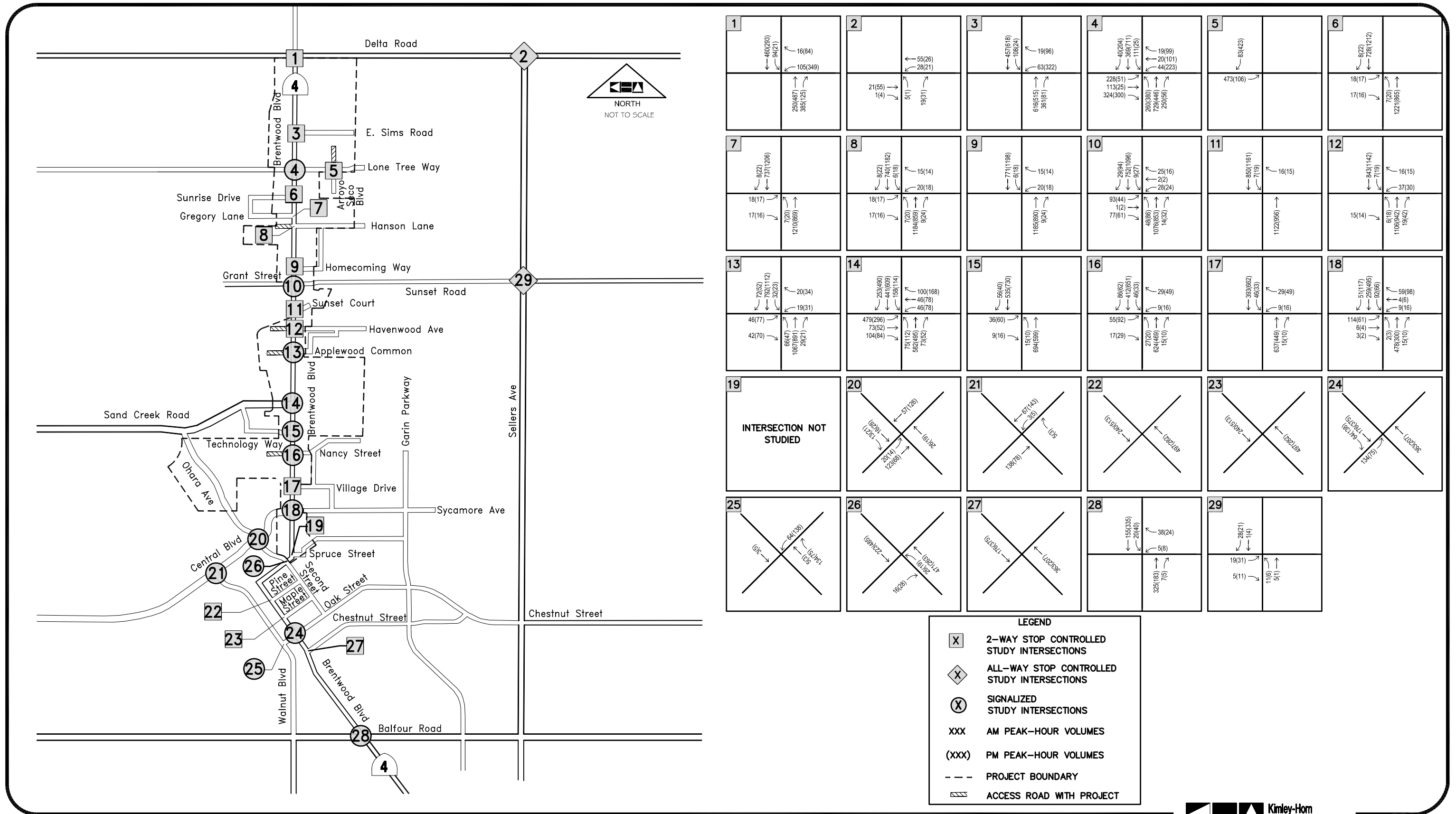


FIGURE 5
PROPOSED PROJECT TRIP ASSIGNMENT



TRAFFIC IMPACT ANALYSIS METHODOLOGY

Analysis of significant environmental impacts at intersections is based on the concept of Level of Service (LOS). The LOS of an intersection is a qualitative measure used to describe operational conditions. LOS ranges from A (best), which represents minimal delay, to F (worst), which represents heavy delay and a facility that is operating at or near its functional capacity.

Contra Costa County Measure C established a sales tax to be used to fund transportation improvements in Contra Costa County. The measure includes a growth management program and requires Contra Costa Transportation Authority (CCTA) to develop a comprehensive transportation plan and update it every other year. To receive a share of the sales tax generated by Measure C, local jurisdictions must adhere to the LOS standards that Measure C applies to local streets and roads. The Measure C standards are applied to streets and roads for which the jurisdictions are responsible. Each jurisdiction must take appropriate action to ensure that the LOS standards are met, including routes of regional significance. Designated regional routes include all the freeways and state highways, and the most significant arterials in Contra Costa County.

The CCTA classifies several roadways within or near the Project area as routes of regional significance. These routes include Brentwood Boulevard, Walnut Boulevard, Lone Tree Way, and Balfour Road¹¹. As such, intersections along these routes require analysis utilizing Growth Management Program procedures outlined in the CCTA Technical Procedures, dated July 19, 2006. The CCTA Technical Procedures require the analysis of intersections based on the Intersection Capacity Utilization (ICU) methodology. The ICU methodology describes the operation of an intersection in terms of Level of Service (LOS) based on corresponding volume to capacity v/c ratio. Based on City of Brentwood and CCTA requirements, this traffic analysis was completed using the CCTALOS module within Traffix® software at signalized intersections, and the *Highway Capacity Manual* (HCM) module within Traffix® at unsignalized intersections. Table 2 presents LOS definitions for signalized intersection as defined by the ICU methodology.

The HCM includes procedures for analyzing two-way stop controlled (TWSC) and all-way stop controlled (AWSC). The TWSC procedure defines LOS as a function of average control delay for each minor street approach movement. Conversely, the AWSC procedure defines LOS as a function of average control delay with each intersection approach analyzed independently. Table 2 presents LOS for unsignalized intersections as defined by the HCM.

Roadway segment LOS definitions for two-lane and multilane highways are based on the HCM. Table 3 and presents the applicable roadway segment LOS definitions for two-lane and multilane highway facilities.

Peak-hour traffic signal warrant evaluation is consistent with the peak-hour volume warrant (Warrant 3B) methodologies noted in Section 4C of the *California Manual on Uniform Traffic Control Devices (CMUTCD)*, September 26, 2006.

The LOS analysis was conducted for the study intersections and roadway segments for weekday AM and PM peak-hours for the following scenarios:

- A. Existing (2007) Conditions
- B. Existing plus Approved Projects Conditions (listed below)
- C. Existing plus Approved Projects plus Proposed Project Conditions
- D. Cumulative (2030) Conditions
- E. Cumulative plus Proposed Project Conditions

¹¹ *East County Action Plan*, Contra Costa Transportation Authority, June 29, 2000.

Table 2 - Intersection Level of Service Criteria

Level of Service	Description	Signalized (Intersection Volume to Capacity ratio, v/c)	Unsignalized (Average Control Delay per Vehicle, sec/veh.)
A	Free flow with no delays. Users are virtually unaffected by others in the traffic stream	≤ 0.6	≤ 10
B	Stable traffic. Traffic flows smoothly with few delays.	0.61 – 0.70	> 10 – 15
C	Stable flow but the operation of individual users becomes affected by other vehicles. Modest delays.	0.71 – 0.80	> 15 – 25
D	Approaching unstable flow. Operation of individual users becomes significantly affected by other vehicles. Delays may be more than one cycle during peak hours.	0.81 – 0.90	> 25 – 35
E	Unstable flow with operating conditions at or near the capacity level. Long delays and vehicle queuing.	0.91 – 1.00	> 35 – 50
F	Forced or breakdown flow that causes reduced capacity. Stop and go traffic conditions. Excessive long delays and vehicle queuing.	> 1.00	> 50

Sources: *Contra Costa Transportation Authority Technical Procedures 2006 and Transportation Research Board, Highway Capacity Manual 2000, National Research Council, 2000.*

Table 3 – Two Lane Roadway Segment Level of Service Criteria

Facility Type	Percent Time-Spent-Following (PTSF)				
	A	B	C	D	E
2-Lane, Class II Highway	≤ 40	>40-55	>55-70	>70-85	>85
Multilane Highway	490	810	1170	1550	1900

Source: Highway Capacity Manual, 2000

EXISTING CONDITIONS

Analysis of existing traffic conditions at the study intersections was based on peak-hour traffic counts conducted in May and June of 2007. These counts were conducted while local schools were in regular session. The weekday AM and PM peak-hour intersection turning movement traffic counts were conducted between the hours of 7 a.m. and 9 a.m., and 4 p.m., and 6 p.m., respectively. The existing peak-hour turn movement volumes are presented in Figure 6. Traffic count sheets are presented in Appendix A and the analysis worksheets for this scenario are provided in Appendix B.

Intersections

Table 4 presents the existing peak-hour intersection operating conditions and signal warrants for the study intersections. As indicated in Table 4, the study intersections operate from LOS A to LOS F during the AM and PM peak- hours.

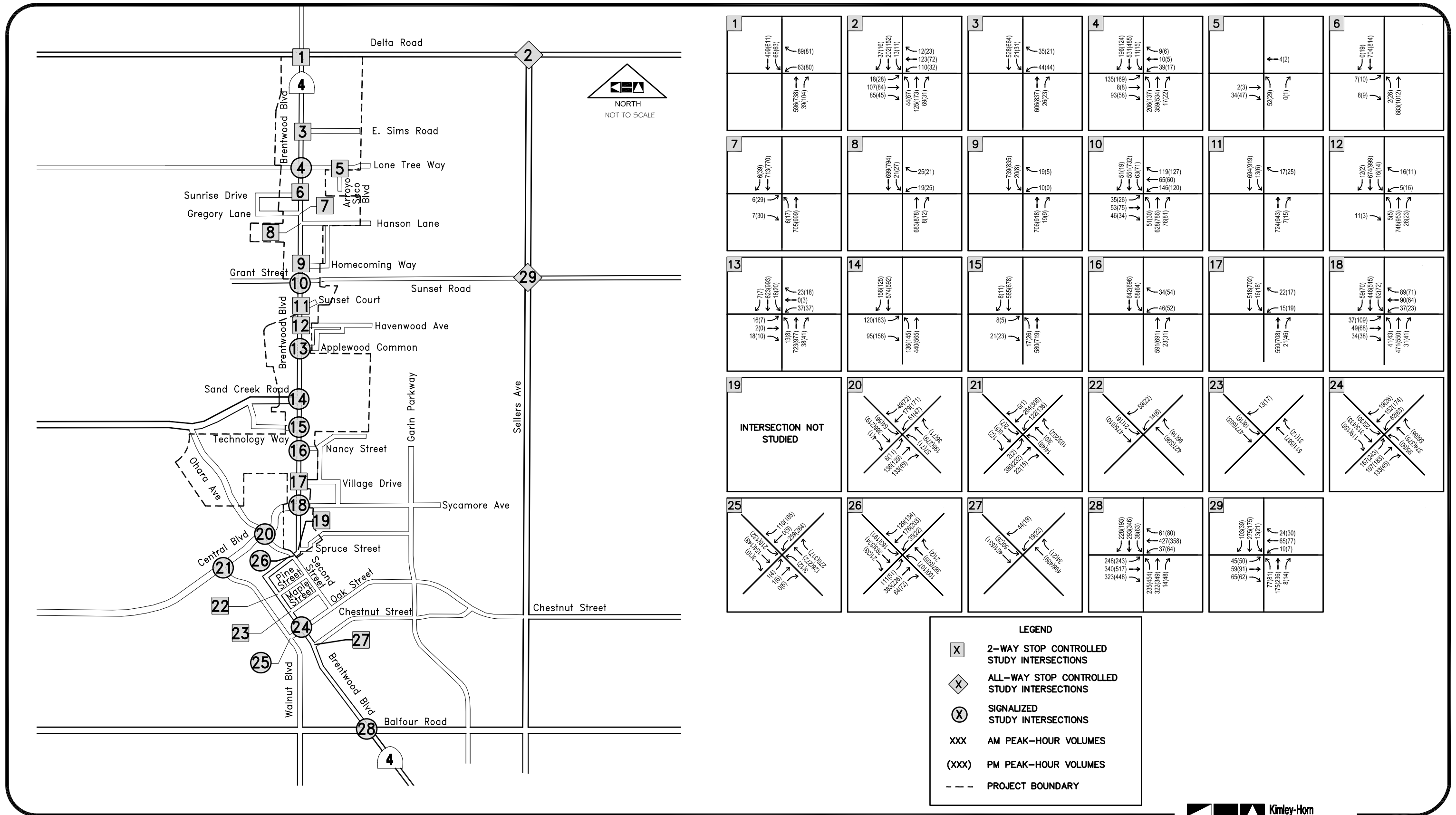


FIGURE 6
EXISTING PEAK-HOUR TRAFFIC VOLUMES

Table 4 – Existing Intersection Levels of Service

#	Intersection	Traffic Control	AM Peak-Hour		PM Peak-Hour		Meets Signal Warrant ¹
			Delay (sec.) or V/C ratio*	LOS	Delay (sec.) or V/C ratio*	LOS	
1	Brentwood Blvd. @ Delta Rd.	TWSC	48.0 (WB)	E	178.4 (WB)	F	Yes
2	Delta Rd. @ Sellers Ave.	AWSC	11.6	B	9.9	A	Yes
3	Brentwood Blvd. @ E. Sims Rd.	TWSC	30.7 (WB)	D	82.2(WB)	F	No
4	Brentwood Blvd. @ Lone Tree Wy.	Signal	0.462	A	0.453	A	N/A
5	Arroyo Seco Rd. @ Lone Tree Wy.	TWSC	8.8 (NB)	A	8.8 (NB)	A	No
6	Brentwood Blvd. @ Sunrise Dr.	TWSC	25.3 (EB)	D	72.4 (EB)	F	No
7	Brentwood Blvd. @ Gregory Ln.	TWSC	26.1 (EB)	D	108.4 (EB)	F	No
8	Brentwood Blvd. @ Hanson Ln.	TWSC	31.3 (WB)	D	92.0 (WB)	F	No
9	Brentwood Blvd. @ Homecoming Wy.	TWSC	27.9 (WB)	D	20.1 (WB)	C	No
10	Brentwood Blvd. @ Grant St./Sunset Rd.	Signal	0.628	B	0.710	C	N/A
11	Brentwood Blvd. @ Sunset Ct.	TWSC	15.4 (WB)	C	20.8 (WB)	C	No
12	Brentwood Blvd. @ Havenwood Ave.	TWSC	23.4 (WB)	C	170.9 (WB)	F	No
13	Brentwood Blvd. @ Applewood Common	Signal	0.484	A	0.638	B	N/A
14	Brentwood Blvd. @ Sand Creek Rd.	Signal	0.361	A	0.399	A	N/A
15	Brentwood Blvd. @ Technology Wy.	Signal	0.187	A	0.218	A	N/A
16	Brentwood Blvd. @ Nancy St.	Signal	0.239	A	0.277	A	N/A
17	Brentwood Blvd. @ Village Dr.	TWSC	19.9 (WB)	C	36.3 (WB)	E	No
18	Brentwood Blvd. @ Central Blvd./Sycamore Ave.	Signal	0.384	A	0.462	A	N/A
20	Second St. @ Central Blvd.	Signal	0.266	A	0.222	A	N/A
21	Central Blvd. @ Walnut Blvd.	Signal	0.205	A	0.202	A	N/A
22	Brentwood Blvd. @ Pine St.	TWSC	11.8 (EB)	B	13.1 (WB)	B	No
23	Brentwood Blvd. @ Maple St.	TWSC	10.0 (WB)	B	10.3 (EB)	B	No
24	Brentwood Blvd. @ Oak St.	Signal	0.391	A	0.438	A	N/A
25	Oak St. @ Walnut Blvd.	Signal	0.368	A	0.420	A	N/A
26	Brentwood Blvd. @ Second St.	Signal	0.375	A	0.359	A	N/A
27	Brentwood Blvd. @ Chestnut St.	TWSC	15.9 (WB)	C	17.7 (WB)	C	No
28	Brentwood Blvd. @ Balfour Rd.	Signal	0.529	A	0.732	C	N/A
29	Sellers Ave. @ Sunset Rd.	AWSC	11.9	B	11.3	B	No

* Control delay for worst minor approach (worst minor movement) for TWSC.
 1. Per the California Manual on Uniform Traffic Control Devices (CMUTCD), September 26, 2006, Section 4C.

Roadway Segments

Table 5 presents the existing roadway segment levels of service for the study roadway segments. As indicated in Table 5, the roadway segments operate at LOS A to LOS E.

Table 5 – Existing Roadway Segment Levels of Service

#	Intersection	Facility Type	# of Lanes	AM Peak-Hour		PM Peak-Hour	
				PTSF (2 lane) pc/hr/ln (4 lane) ¹	LOS	PTSF (2 lane) pc/hr/ln (4 lane) ¹	LOS
1	Brentwood Blvd. between E. Sims Rd. and Delta Rd.*	Arterial	2	77.0%	E	82.3%	E
2	Brentwood Blvd. Grant St./Sunset Rd. Sunset Ct.*	Arterial	2	80.7%	E	87.4%	E
3	Brentwood Blvd. between Central Blvd. and Spruce St.*	Arterial	4	316 / 329	A / A	426 / 404	A / A
4	Brentwood Blvd. between Oak Ave. and Pine Ave.*	Arterial	4	305 / 272	A / A	332 / 336	A / A
5	Delta Rd. east of Brentwood Blvd.**	Collector	2	34.5%	A	26.6%	A
6	Lone Tree Wy. west of Brentwood Blvd.**	Collector	2	57.7%	C	59.6%	C
7	Sunset Rd. east of Brentwood Blvd.**	Collector	2	61.5%	C	61.8%	C
8	Central Blvd. west of Brentwood Blvd.**	Arterial	4	70 / 160	A / A	129 / 192	A / A
9	Grant St. west of Brentwood Blvd.**	Collector	2	54.3%	B	44.9%	B
10	Sycamore Ave. east of Brentwood Blvd.**	Collector	2	52.5%	B	54.3%	B

Source: *Highway Capacity Manual, 2000.*

¹ PTSF = Percent Time Spent Following, pc/hr/ln = passenger cars per hour per lane.

* Results for this roadway segment are presented in **NB / SB** format (where applicable).

** Results for this roadway segment are presented in **EB / WB** format (where applicable).

EXISTING PLUS APPROVED PROJECTS CONDITIONS

Peak-hour traffic associated with several development projects in the vicinity of the proposed project was added to the existing traffic volumes. These projects, which are in various stages of planning or development, are assumed to be developed and fully occupied prior to implementation of the Proposed Project. Project information for all approved projects to be included in this analysis was provided by the City¹².

A summary of pending or approved projects that were determined to contribute traffic to the study intersections is provided in Table 6.

¹² City of Brentwood Project Status Report, May 5, 2007.

Table 6 – Pending and Approved Projects

Project Name	Land Use Type	Size
Palmilla	Residential	471 Single Family, 108 Multi-family
Towncentre Commons	Residential	16 Multi-family
Garin Corners	Residential	168 Single Family
Mission Grove	Residential	140 Single Family
Magnolia	Residential	34 Single Family
Barrington	Residential	494 Single Family
The Parc at Cedarwood	Residential	177 Single Family
Brentwood Senior Commons	Residential	80 Multi-Family
Sycamore Glen	Residential	9 Single Family
Hampton Inn	Hotel	59 Rooms
Brentwood Center II	Retail	8,800 Sq. Ft.
Office Condo Buildings	Office	37,776 Sq. Ft.
Sand Creek Plaza	Retail	8,595 Retail
The Plaza at Balfour	Retail	33,200 Sq. Ft.
Garin Commercial	Retail /Office	44,300 Sq. Ft. Retail, 55,500 Sq. Ft. Office
Best Western Motel	Motel	45 Rooms
RayWilliams	Office	30,000 Sq. Ft.

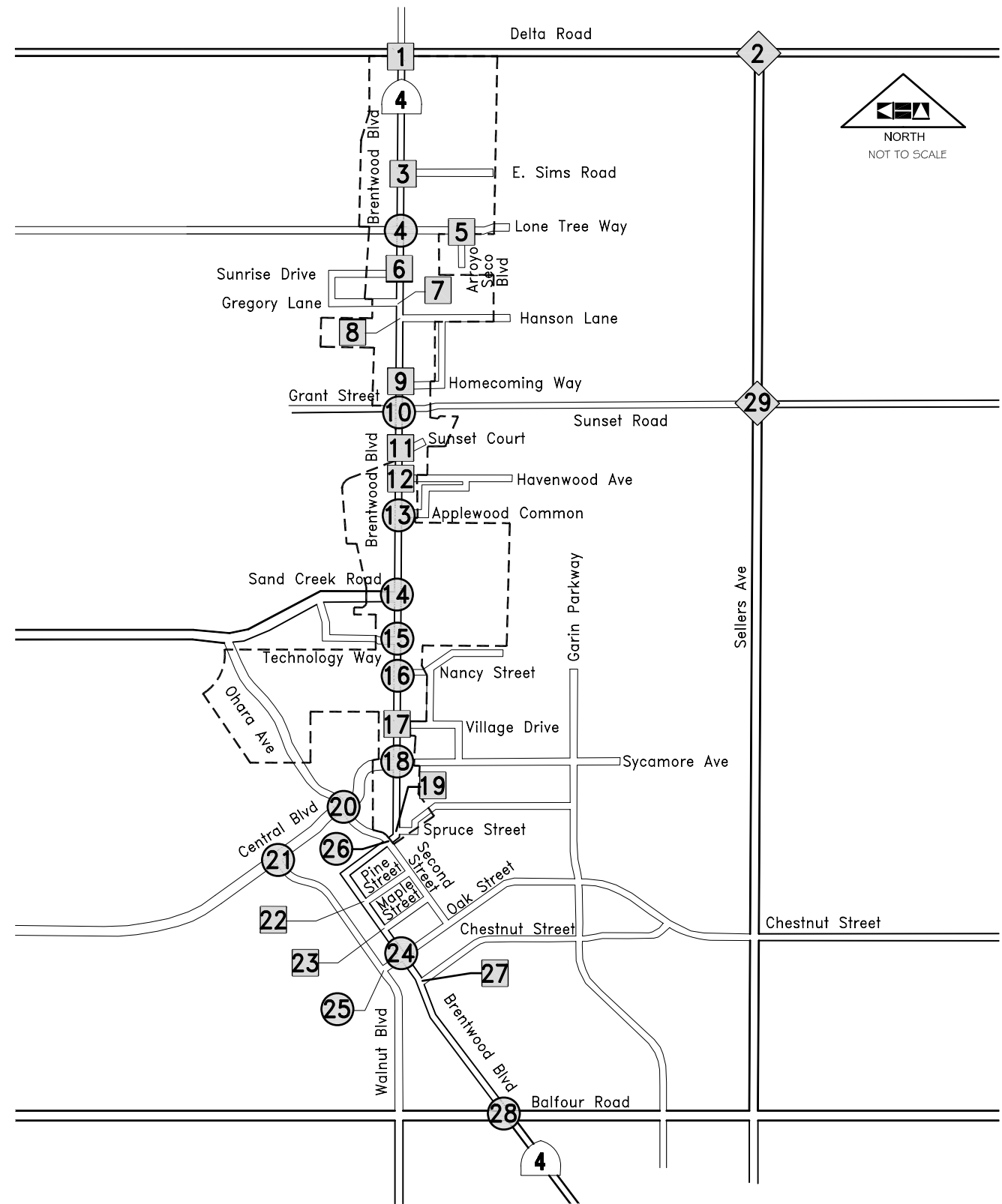
Figure 7 provides the AM and PM peak-hour traffic volumes at the study intersections for this analysis scenario. Analysis worksheets for this scenario are provided in Appendix C.

Intersections

Table 7 presents the peak-hour intersection operating conditions and signal warrants for the study intersections. As indicated in Table 7, the study intersections operate from LOS A to LOS F during the AM and PM peak- hours.

Roadway Segments

Table 8 presents the roadway segment levels of service for the study roadway segments. As indicated in Table 8, the roadway segments operate at LOS A to LOS E.



1	2	3	4	5	6
7	8	9	10	11	12
13	14	15	16	17	18
19	20	21	22	23	24
25	26	27	28	29	

LEGEND

- X 2-WAY STOP CONTROLLED STUDY INTERSECTIONS
- ◇ ALL-WAY STOP CONTROLLED STUDY INTERSECTIONS
- ⊗ SIGNALIZED STUDY INTERSECTIONS
- XXX AM PEAK-HOUR VOLUMES
- (XXX) PM PEAK-HOUR VOLUMES
- - - PROJECT BOUNDARY



FIGURE 7
EXISTING PLUS APPROVED PROJECTS PEAK-HOUR TRAFFIC VOLUMES

Table 7 – Existing plus Approved Projects Intersection Levels of Service

#	Intersection	Traffic Control	AM Peak-Hour		PM Peak-Hour		Meets Signal Warrant ¹
			Delay (sec.) or V/C ratio [*]	LOS	Delay (sec.) or V/C ratio [*]	LOS	
1	Brentwood Blvd. @ Delta Rd.	TWSC	105.2 (WB)	F	443.2 (WB)	F	Yes
2	Delta Rd. @ Sellers Ave.	AWSC	11.6	B	9.9	A	Yes
3	Brentwood Blvd. @ E. Sims Rd.	TWSC	60.3 (WB)	F	293.1 (WB)	F	Yes
4	Brentwood Blvd. @ Lone Tree Wy.	Signal	0.530	A	0.535	A	N/A
5	Arroyo Seco Rd. @ Lone Tree Wy.	TWSC	8.9 (NB)	A	8.9 (NB)	A	No
6	Brentwood Blvd. @ Sunrise Dr.	TWSC	39.8 (EB)	E	457.4 (EB)	F	No
7	Brentwood Blvd. @ Gregory Ln.	TWSC	34.4 (EB)	D	703.9 (EB)	F	No
8	Brentwood Blvd. @ Hanson Ln.	TWSC	88.0 (WB)	F	711.1 (WB)	F	No
9	Brentwood Blvd. @ Homecoming Wy.	TWSC	85.1 (WB)	F	326.3 (WB)	F	No
10	Brentwood Blvd. @ Grant St./Sunset Rd.	Signal	0.784	C	0.909	E	N/A
11	Brentwood Blvd. @ Sunset Ct.	TWSC	19.0 (WB)	C	32.4 (WB)	D	No
12	Brentwood Blvd. @ Havenwood Ave.	TWSC	89.4 (WB)	F	> 1000 (WB)	F	No
13	Brentwood Blvd. @ Applewood Common	Signal	0.620	B	0.866	D	N/A
14	Brentwood Blvd. @ Sand Creek Rd.	Signal	0.481	A	0.560	A	N/A
15	Brentwood Blvd. @ Technology Wy.	Signal	0.256	A	0.330	A	N/A
16	Brentwood Blvd. @ Nancy St.	Signal	0.328	A	0.401	A	N/A
17	Brentwood Blvd. @ Village Dr.	TWSC	39.7 (WB)	E	218.8 (WB)	F	No
18	Brentwood Blvd. @ Central Blvd./Sycamore Ave.	Signal	0.537	A	0.702	C	N/A
20	Second St. @ Central Blvd.	Signal	0.331	A	0.272	A	N/A
21	Central Blvd. @ Walnut Blvd.	Signal	0.243	A	0.280	A	N/A
22	Brentwood Blvd. @ Pine St.	TWSC	13.5 (WB)	B	20.6 (WB)	C	No
23	Brentwood Blvd. @ Maple St.	TWSC	10.6 (WB)	B	11.9 (WB)	B	No
24	Brentwood Blvd. @ Oak St.	Signal	0.460	A	0.532	A	N/A
25	Oak St. @ Walnut Blvd.	Signal	0.394	A	0.460	A	N/A
26	Brentwood Blvd. @ Second St.	Signal	0.429	A	0.457	A	N/A
27	Brentwood Blvd. @ Chestnut St.	TWSC	25.5 (WB)	D	40.9 (WB)	E	No
28	Brentwood Blvd. @ Balfour Rd.	Signal	0.605	B	0.793	C	N/A
29	Sellers Ave. @ Sunset Rd.	AWSC	12.0	B	11.6	B	No

* Control delay for worst minor approach (worst minor movement) for TWSC.
 1. Per the California Manual on Uniform Traffic Control Devices (CMUTCD), September 26, 2006, Section 4C.

Table 8 – Existing plus Approved Projects Roadway Segment Levels of Service

#	Intersection	Facility Type	# of Lanes	AM Peak-Hour		PM Peak-Hour	
				PTSF (2 lane) pc/hr/ln (4 lane) ¹	LOS	PTSF (2 lane) pc/hr/ln (4 lane) ¹	LOS
1	Brentwood Blvd. between E. Sims Rd. and Delta Rd.*	Arterial	2	81.5%	E	86.5%	E
2	Brentwood Blvd. Grant St./Sunset Rd. Sunset Ct.*	Arterial	2	87.0%	E	93.4%	E
3	Brentwood Blvd. between Central Blvd. and Spruce St.*	Arterial	4	392 / 436	A / A	629 / 527	B / B
4	Brentwood Blvd. between Oak Ave. and Pine Ave.*	Arterial	4	377 / 379	A / A	489 / 455	A / A
5	Delta Rd. east of Brentwood Blvd.**	Collector	2	34.5%	A	26.6%	A
6	Lone Tree Wy. west of Brentwood Blvd.**	Collector	2	66.9%	C	65.2%	C
7	Sunset Rd. east of Brentwood Blvd.**	Collector	2	65.1%	C	68.8%	C
8	Central Blvd. west of Brentwood Blvd.**	Arterial	4	140 / 208	A / A	203 / 290	A / A
9	Grant St. west of Brentwood Blvd.**	Collector	2	54.3%	B	44.9%	B
10	Sycamore Ave. east of Brentwood Blvd.**	Collector	2	60.9%	C	62.9%	C

Source: *Highway Capacity Manual, 2000.*

¹ PTSF = Percent Time Spent Following, pc/hr/ln = passenger cars per hour per lane.

* Results for this roadway segment are presented in **NB / SB** format (where applicable).

** Results for this roadway segment are presented in **EB / WB** format (where applicable).

EXISTING PLUS APPROVED PROJECTS PLUS PROPOSED PROJECT CONDITIONS

Traffic associated with the proposed project was added to the Existing plus Approved Projects traffic volumes to establish the Existing plus Approved Projects plus Proposed Project traffic conditions. It is important to note that trips attributed to existing land uses were not discounted with the addition of the proposed project trips. Furthermore, the SR-4 Bypass, which will result in removing vehicle trips from Brentwood Boulevard, is expected to be fully operational prior to the full build-out of the proposed project. However, the SR-4 Bypass was not included in this analysis scenario due to uncertainties in the timing of its completion. Due to these assumptions, the traffic volumes for this analysis scenario are anticipated to be greater than what is expected upon full build-out of the proposed project and, therefore, result in a conservative analysis. Traffic volumes for this scenario are presented in Figure 8.

Figure 9 presents the revised lane geometries for this scenario, which have been modified due to anticipated extensions of some east-west side streets¹³. Analysis worksheets for this scenario are provided in Appendix D.

Intersections

Table 9 presents the peak-hour intersection operating conditions for the study intersections. As indicated in Table 9, the study intersections operate from LOS A to LOS F during the AM and PM peak- hours.

¹³ Draft Brentwood Boulevard Specific Plan Land Use and Circulation Map, August 2, 2007.

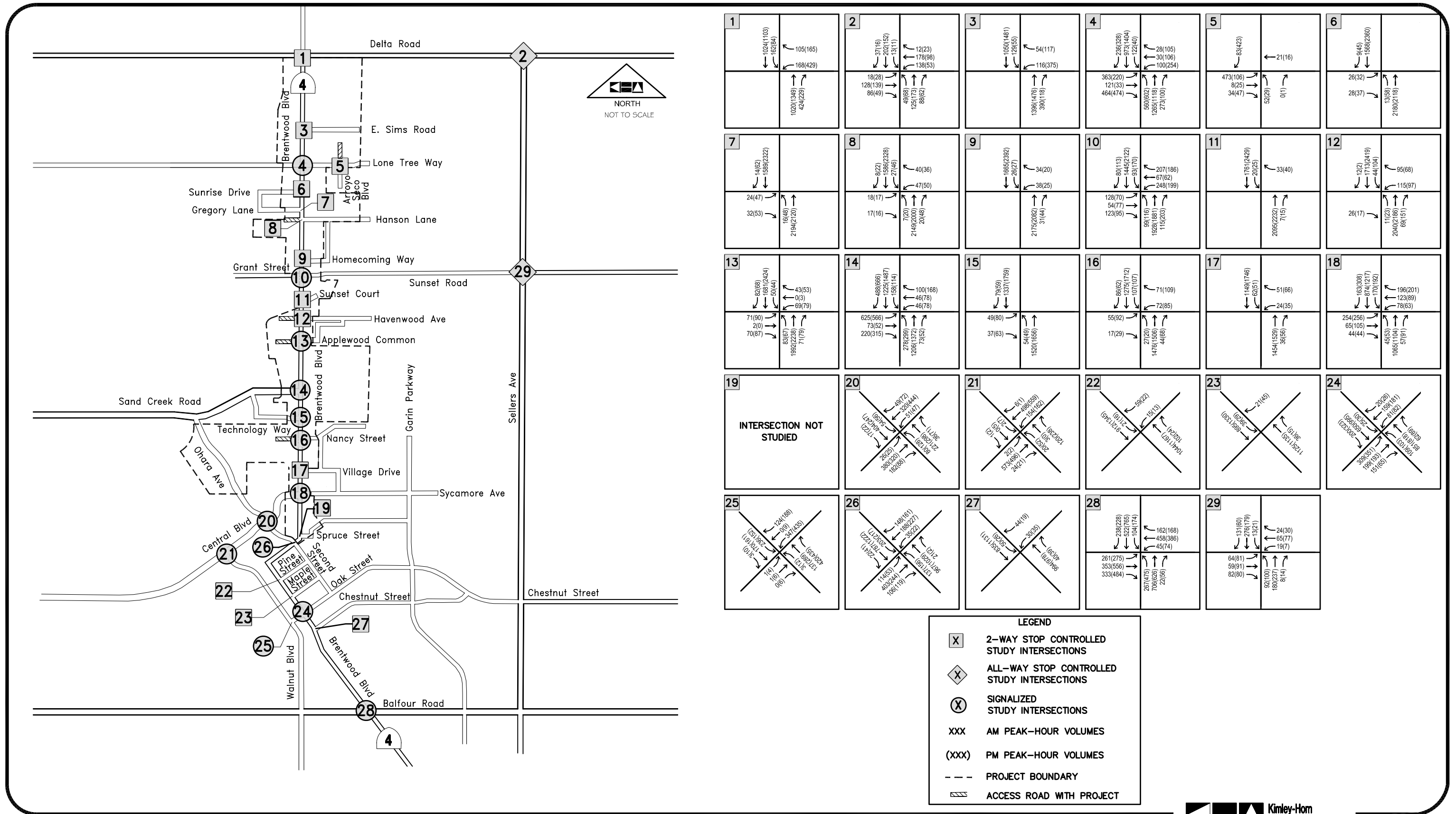


FIGURE 8
EXISTING PLUS APPROVED PROJECT PLUS PROPOSED PROJECT PEAK-HOUR TRAFFIC VOLUMES



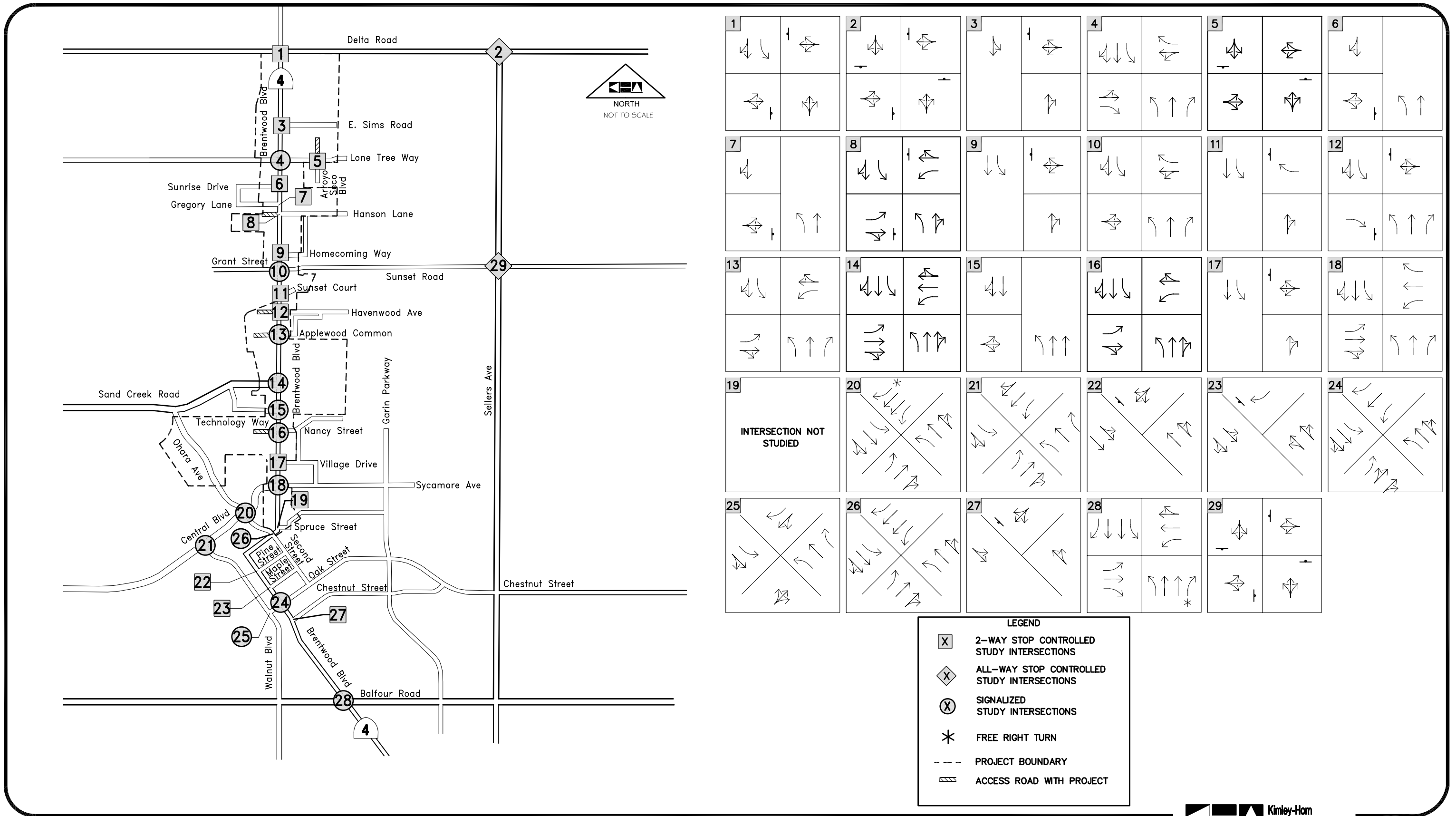


FIGURE 9
 EXISTING PLUS APPROVED PROJECTS PLUS PROPOSED PROJECT
 INTERSECTION GEOMETRY AND TRAFFIC CONTROL

Table 9 – Existing plus Approved Projects plus Proposed Project Intersection Levels of Service

#	Intersection	Traffic Control	AM Peak-Hour		PM Peak-Hour		Meets Signal Warrant ¹
			Delay (sec.) or V/C ratio [*]	LOS	Delay (sec.) or V/C ratio [*]	LOS	
1	Brentwood Blvd. @ Delta Rd.	TWSC	> 1000 (WB)	F	> 1000 (WB)	F	Yes
2	Delta Rd. @ Sellers Ave.	AWSC	13.5	B	11.1	B	Yes
3	Brentwood Blvd. @ E. Sims Rd.	TWSC	> 1000 (WB)	F	> 1000 (WB)	F	Yes
4	Brentwood Blvd. @ Lone Tree Wy.	Signal	1.213	F	1.261	F	N/A
5	Arroyo Seco Rd. @ Lone Tree Wy.	TWSC	41.4 (NB)	E	19.0 (NB)	C	No
6	Brentwood Blvd. @ Sunrise Dr.	TWSC	> 1000 (EB)	F	> 1000 (EB)	F	Yes
7	Brentwood Blvd. @ Gregory Ln.	TWSC	> 1000 (EB)	F	> 1000 (EB)	F	Yes
8	Brentwood Blvd. @ Hanson Ln.	TWSC	> 1000 (WB)	F	> 1000 (WB)	F	No
9	Brentwood Blvd. @ Homecoming Wy.	TWSC	> 1000 (WB)	F	> 1000 (WB)	F	Yes
10	Brentwood Blvd. @ Grant St./Sunset Rd.	Signal	1.601	F	1.730	F	N/A
11	Brentwood Blvd. @ Sunset Ct.	TWSC	297.1 (WB)	F	668.4 (WB)	F	No
12	Brentwood Blvd. @ Havenwood Ave.	TWSC	> 1000 (WB)	F	> 1000 (WB)	F	Yes
13	Brentwood Blvd. @ Applewood Common	Signal	0.911	E	1.059	F	N/A
14	Brentwood Blvd. @ Sand Creek Rd.	Signal	1.127	F	1.278	F	N/A
15	Brentwood Blvd. @ Technology Wy.	Signal	0.472	A	0.603	B	N/A
16	Brentwood Blvd. @ Nancy St.	Signal	0.602	B	0.672	B	N/A
17	Brentwood Blvd. @ Village Dr.	TWSC	> 1000 (WB)	F	> 1000 (WB)	F	Yes
18	Brentwood Blvd. @ Central Blvd./Sycamore Ave.	Signal	0.937	E	0.954	E	N/A
20	Second St. @ Central Blvd.	Signal	0.377	A	0.295	A	N/A
21	Central Blvd. @ Walnut Blvd.	Signal	0.287	A	0.305	A	N/A
22	Brentwood Blvd. @ Pine St.	TWSC	24.3 (WB)	C	41.3 (WB)	E	No
23	Brentwood Blvd. @ Maple St.	TWSC	13.3 (WB)	B	13.6 (WB)	B	No
24	Brentwood Blvd. @ Oak St.	Signal	0.567	A	0.707	C	N/A
25	Oak St. @ Walnut Blvd.	Signal	0.438	A	0.546	A	N/A
26	Brentwood Blvd. @ Second St.	Signal	0.574	A	0.576	A	N/A
27	Brentwood Blvd. @ Chestnut St.	TWSC	71.3 (WB)	F	150.3 (WB)	F	No
28	Brentwood Blvd. @ Balfour Rd.	Signal	0.666	B	0.902	E	N/A
29	Sellers Ave. @ Sunset Rd.	AWSC	13.1	B	12.5	B	No

Shaded cells indicate significant impact. *
 Control delay for worst minor approach (worst minor movement) for TWSC.
 1. Per the California Manual on Uniform Traffic Control Devices (CMUTCD), September 26, 2006, Section 4C.

Roadway Segments

Table 10 presents the roadway segment levels of service for the study roadway segments. As indicated in Table 10, the roadway segments operate at LOS A to LOS E.

Table 10 – Existing plus Approved Projects plus Proposed Project Roadway Segment Levels of Service

#	Intersection	Facility Type	# of Lanes	AM Peak-Hour		PM Peak-Hour	
				PTSF (2 lane) pc/hr/ln (4 lane) ¹	LOS	PTSF (2 lane) pc/hr/ln (4 lane) ¹	LOS
1	Brentwood Blvd. between E. Sims Rd. and Delta Rd.*	Arterial	2	93.9%	E	95.9%	E
2	Brentwood Blvd. Grant St./Sunset Rd. Sunset Ct.*	Arterial	2	99.0%	E	100.0%	E
3	Brentwood Blvd. between Central Blvd. and Spruce St.*	Arterial	4	680 / 559	B / B	840 / 784	C / B
4	Brentwood Blvd. between Oak Ave. and Pine Ave.*	Arterial	4	667 / 522	B / B	641 / 732	B / B
5	Delta Rd. east of Brentwood Blvd.**	Collector	2	44.3%	B	54.3%	B
6	Lone Tree Wy. west of Brentwood Blvd.**	Collector	2	86.4%	E	86.3%	E
7	Sunset Rd. east of Brentwood Blvd.**	Collector	2	67.7%	C	71.7%	D
8	Central Blvd. west of Brentwood Blvd.**	Arterial	4	213 / 241	A / A	243 / 374	A / A
9	Grant St. west of Brentwood Blvd.**	Collector	2	65.0%	C	61.7%	C
10	Sycamore Ave. east of Brentwood Blvd.**	Collector	2	65.0%	C	68.8%	C

Source: *Highway Capacity Manual, 2000*.

Shaded cells indicate significant impact.

¹ PTSF = Percent Time Spent Following, pc/hr/ln = passenger cars per hour per lane.

* Results for this roadway segment are presented in **NB / SB** format (where applicable).

** Results for this roadway segment are presented in **EB / WB** format (where applicable).

CUMULATIVE (2030) CONDITIONS

Year 2030 AM and PM peak-hour roadway segment traffic volumes were obtained from the CCTA Regional Travel Demand model. The model estimates vehicle traffic expected for build-out of the land uses designated in the General Plan. The model then assigns those traffic volumes to various roadway facilities anticipated to be in place prior to the year for which traffic is being forecasted. The SR-4 Bypass is assumed to be in place under cumulative conditions.

The City’s General Plan¹⁴ designates a number of improvements for the project area as noted below. It is assumed the following improvements will be implemented over time with the implementation of the General Plan.

- Widen Brentwood Boulevard to 4-lanes

¹⁴ *City of Brentwood General Plan, 2001-2021*, November 2001.

- Sand Creek Road east from Brentwood to Sellers Avenue.

Existing turning movement percentages were used to establish turning movement volumes for Cumulative traffic conditions. In some cases, the use of existing turning movement percentages were not deemed appropriate due to anticipated roadway network changes. In such cases, traffic volumes were adjusted to reflect the probable effects on the circulation patterns in the vicinity of the proposed project. Traffic volumes for this scenario are presented in Figure 10.

Figure 11 presents the revised lane geometries for this scenario, which takes into account the extension of existing roadways due to the Proposed Project, as presented in Figure 1. Analysis worksheets for this scenario are provided in Appendix E.

Intersections

Table 11 presents the peak-hour intersection operating conditions for the study intersections. As indicated in Table 11, the study intersections operate from LOS A to LOS F during the AM and PM peak- hours.

Roadway Segments

Table 12 presents the roadway segment levels of service for the study roadway segments. As indicated in Table 12, the roadway segments operate at LOS A to LOS E.

CUMULATIVE PLUS PROPOSED PROJECT CONDITIONS

For the Cumulative Plus Proposed Project Conditions, the CCTA Regional Travel Demand Model was revised to include the land uses designated in the proposed project. Land use data for all Traffic Analysis Zones (TAZ) within the proposed project area was obtained from the City¹⁵. The CCTA model was then adjusted to reflect the land use changes as specified in the Brentwood Boulevard Specific Plan. Furthermore, AM and PM peak-hour roadway segment traffic volumes were extracted from the adjusted model to aid in the development of Cumulative plus Proposed Project traffic volumes. Traffic volumes for this scenario are presented in Figure 12. Analysis worksheets for this scenario are provided in Appendix F.

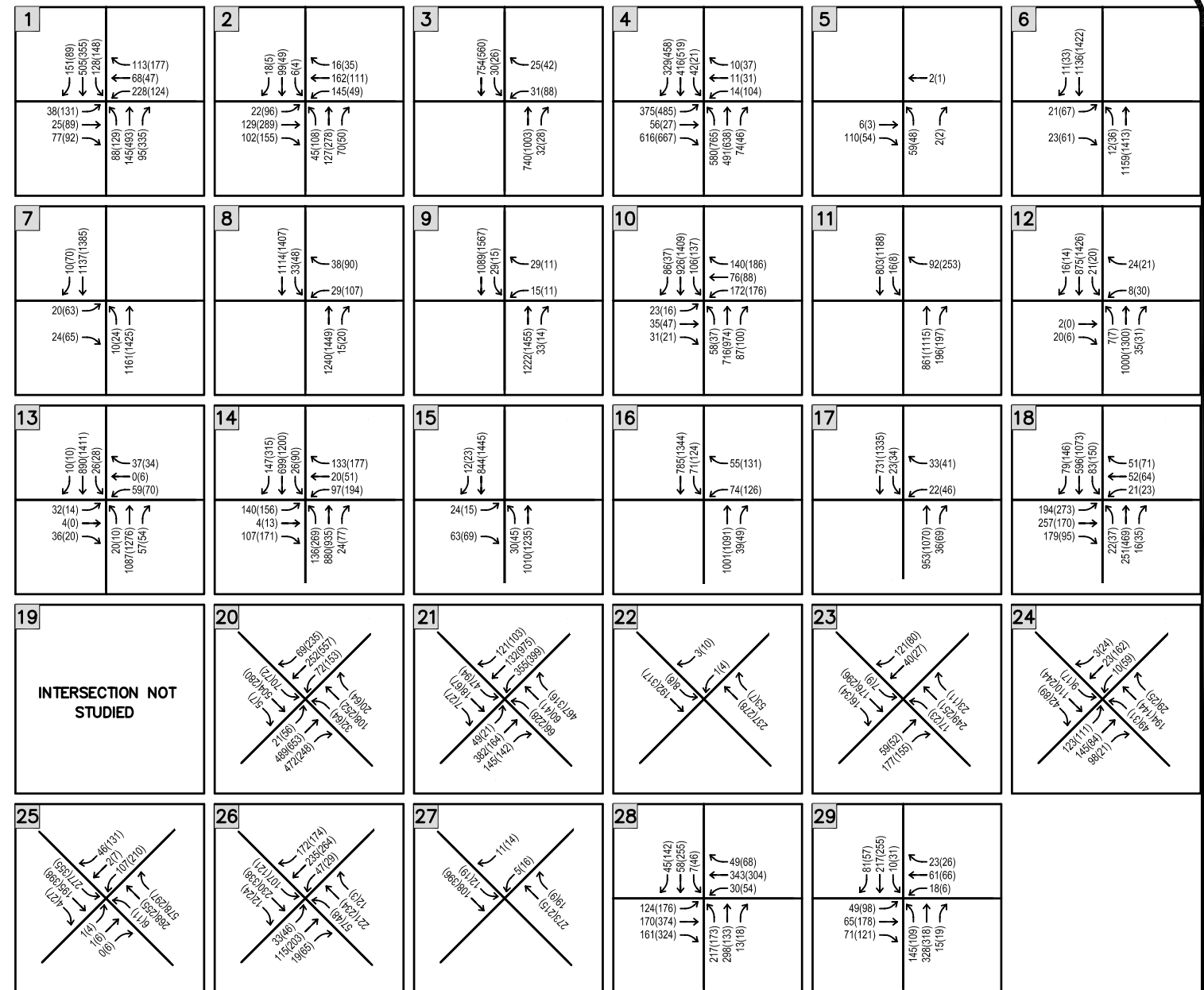
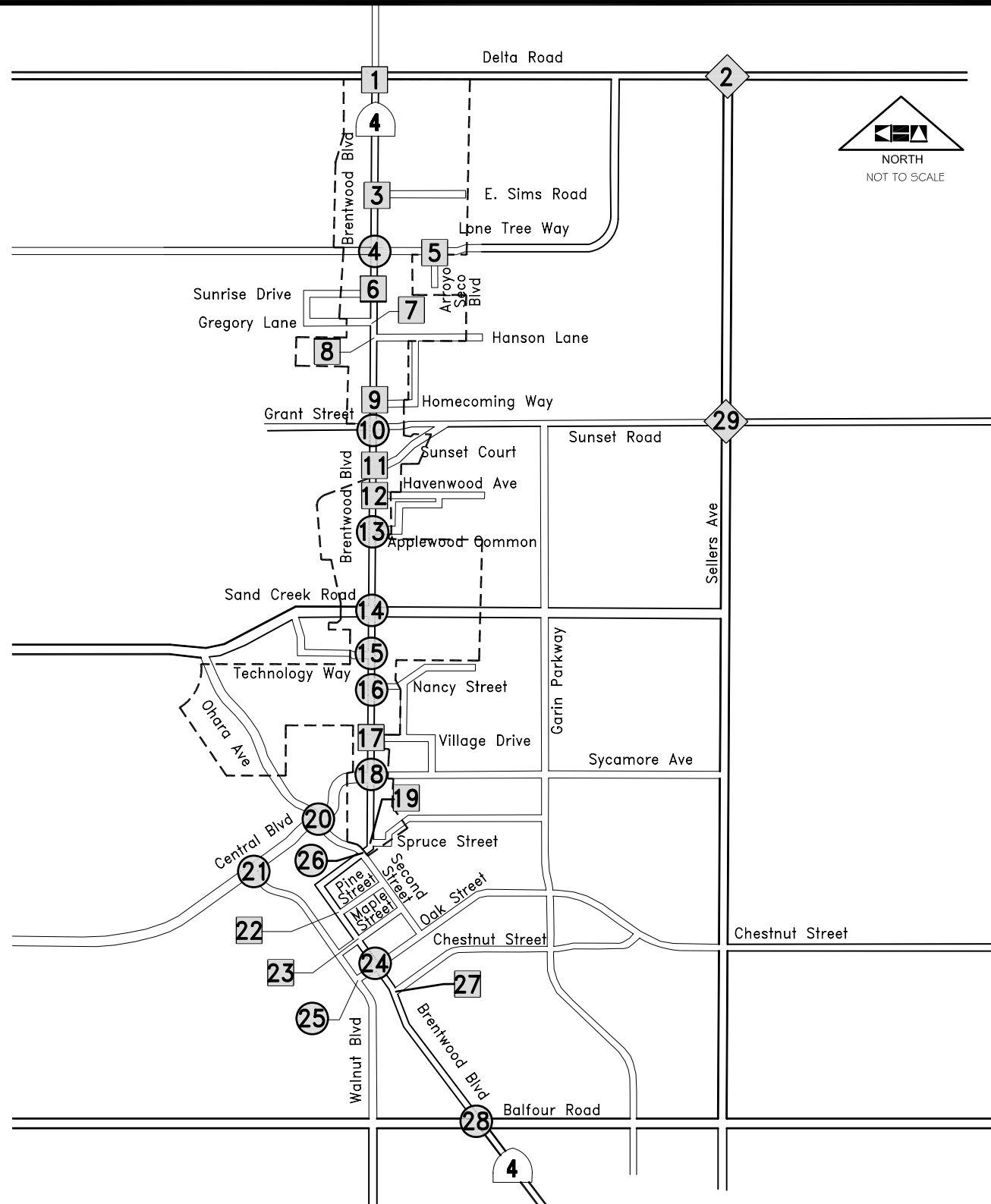
Intersections

Table 13 presents the peak-hour intersection operating conditions for the study intersections. As indicated in Table 13, the study intersections operate from LOS A to LOS F during the AM and PM peak- hours.

Roadway Segments

Table 14 presents the roadway segment levels of service for the study roadway segments. As indicated in Table 14, the roadway segments operate at LOS A to LOS E.

¹⁵ Per email from Heidi Kline, City of Brentwood, September 4, 2007.



LEGEND

- X 2-WAY STOP CONTROLLED STUDY INTERSECTIONS
- ◇ ALL-WAY STOP CONTROLLED STUDY INTERSECTIONS
- ⊗ SIGNALIZED STUDY INTERSECTIONS
- XXX AM PEAK-HOUR VOLUMES
- (XXX) PM PEAK-HOUR VOLUMES
- - - PROJECT BOUNDARY



FIGURE 10
CUMULATIVE PEAK-HOUR TRAFFIC VOLUMES

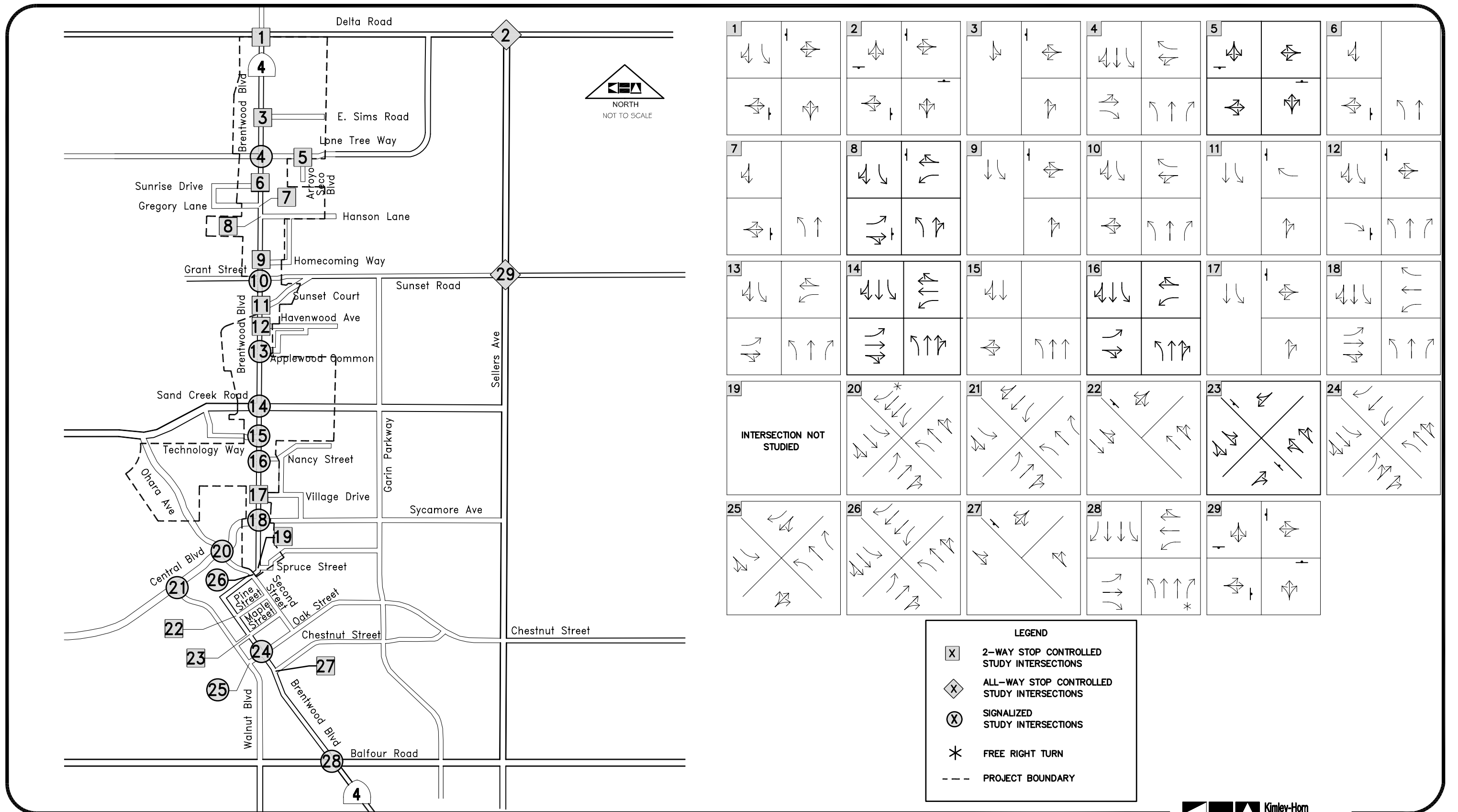


FIGURE 11
 CUMULATIVE LANE GEOMETRIES AND TRAFFIC CONTROL



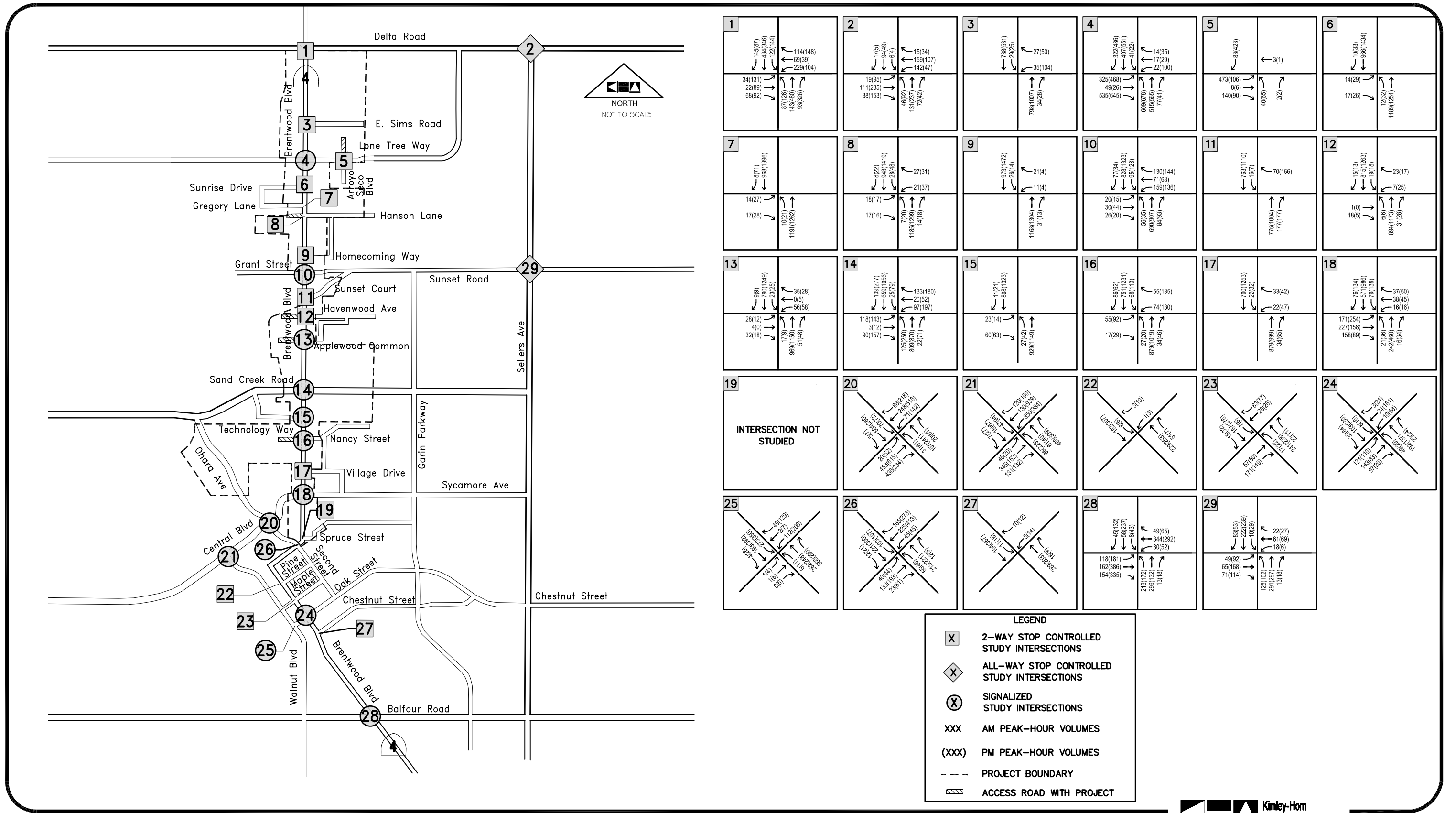


FIGURE 12
CUMULATIVE PLUS PROPOSED PROJECT PEAK-HOUR TRAFFIC VOLUMES

Table 11 – Cumulative Levels of Service

#	Intersection	Traffic Control	AM Peak-Hour		PM Peak-Hour		Meets Signal Warrant ¹
			Delay (sec.) or V/C ratio *	LOS	Delay (sec.) or V/C ratio *	LOS	
1	Brentwood Blvd. @ Delta Rd.	TWSC	> 1000 (WB)	F	> 1000 (WB)	F	Yes
2	Delta Rd. @ Sellers Ave.	AWSC	12.0	B	26.5	D	Yes
3	Brentwood Blvd. @ E. Sims Rd.	TWSC	35.4 (WB)	E	129.1 (WB)	F	Yes
4	Brentwood Blvd. @ Lone Tree Wy.	Signal	0.854	D	1.152	F	N/A
5	Arroyo Seco Rd. @ Lone Tree Wy.	TWSC	> 1000 (NB)	F	> 1000 (NB)	F	No
6	Brentwood Blvd. @ Sunrise Dr.	TWSC	114.8 (EB)	F	> 1000 (EB)	F	Yes
7	Brentwood Blvd. @ Gregory Ln.	TWSC	107.4 (EB)	F	> 1000 (EB)	F	Yes
8	Brentwood Blvd. @ Hanson Ln.	TWSC	139.1 (WB)	F	> 1000 (WB)	F	Yes
9	Brentwood Blvd. @ Homecoming Wy.	TWSC	95.8 (WB)	F	368.8 (WB)	F	No
10	Brentwood Blvd. @ Grant St./Sunset Rd.	Signal	0.853	D	1.110	F	N/A
11	Brentwood Blvd. @ Sunset Ct.	TWSC	21.3 (WB)	C	150.1 (WB)	F	Yes
12	Brentwood Blvd. @ Havenwood Ave.	TWSC	45.7 (WB)	E	> 1000 (WB)	F	No
13	Brentwood Blvd. @ Applewood Common	Signal	0.735	C	0.918	E	N/A
14	Brentwood Blvd. @ Sand Creek Rd.	Signal	0.504	A	0.843	D	N/A
15	Brentwood Blvd. @ Technology Wy.	Signal	0.285	A	0.462	A	N/A
16	Brentwood Blvd. @ Nancy St.	Signal	0.387	A	0.480	A	N/A
17	Brentwood Blvd. @ Village Dr.	TWSC	39.3 (WB)	E	510.6 (WB)	F	No
18	Brentwood Blvd. @ Central Blvd./Sycamore Ave.	Signal	0.348	A	0.572	A	N/A
20	Second St. @ Central Blvd.	Signal	0.508	A	0.505	A	N/A
21	Central Blvd. @ Walnut Blvd.	Signal	0.471	A	0.535	A	N/A
22	Brentwood Blvd. @ Pine St.	TWSC	9.6 (WB)	A	10.0 (WB)	A	No
23	Brentwood Blvd. @ Maple St.	TWSC	12.0 (WB)	B	13.1 (WB)	B	No
24	Brentwood Blvd. @ Oak St.	Signal	0.171	A	0.266	A	N/A
25	Oak St. @ Walnut Blvd.	Signal	0.521	A	0.511	A	N/A
26	Brentwood Blvd. @ Second St.	Signal	0.271	A	0.234	A	N/A
27	Brentwood Blvd. @ Chestnut St.	TWSC	10.3 (WB)	B	11.9 (WB)	B	No
28	Brentwood Blvd. @ Balfour Rd.	Signal	0.343	A	0.442	A	N/A
29	Sellers Ave. @ Sunset Rd.	AWSC	15.7	C	22.0	C	No

* Control delay for worst minor approach (worst minor movement) for TWSC.
 1. Per the California Manual on Uniform Traffic Control Devices (CMUTCD), September 26, 2006, Section 4C.

Table 12 – Cumulative Roadway Segment Levels of Service

#	Intersection	Facility Type	# of Lanes	AM Peak-Hour		PM Peak-Hour	
				PTSF (2 lane) pc/hr/ln (4 lane) ¹	LOS	PTSF (2 lane) pc/hr/ln (4 lane) ¹	LOS
1	Brentwood Blvd. between E. Sims Rd. and Delta Rd.*	Arterial	2	75.8%	E	82.5%	E
2	Brentwood Blvd. Grant St./Sunset Rd. Sunset Ct.*	Arterial	2	84.1%	E	92.0%	E
3	Brentwood Blvd. between Central Blvd. and Spruce St.*	Arterial	4	168 / 193	A / A	364 / 256	A / A
4	Brentwood Blvd. between Oak Ave. and Pine Ave.*	Arterial	4	169 / 95	A / A	154 / 189	A / A
5	Delta Rd. east of Brentwood Blvd.**	Collector	2	53.6%	B	57.7%	C
6	Lone Tree Wy. west of Brentwood Blvd.**	Arterial	4	549 / 507	A / A	645 / 699	B / B
7	Sunset Rd. east of Brentwood Blvd.**	Collector	2	63.6%	C	66.1%	C
8	Central Blvd. west of Brentwood Blvd.**	Arterial	4	369 / 225	A / A	324 / 627	A / B
9	Grant St. west of Brentwood Blvd.**	Collector	2	55.1%	C	46.0%	B
10	Sycamore Ave. east of Brentwood Blvd.**	Collector	2	61.5%	C	62.4%	C

Source: *Highway Capacity Manual, 2000.*

¹ PTSF = Percent Time Spent Following, pc/hr/ln = passenger cars per hour per lane.

* Results for this roadway segment are presented in **NB / SB** format (where applicable).

** Results for this roadway segment are presented in **EB / WB** format (where applicable).

Table 13 – Cumulative plus Proposed Project Levels of Service

#	Intersection	Traffic Control	AM Peak-Hour		PM Peak-Hour		Meets Signal Warrant ¹
			Delay (sec.) or V/C ratio *	LOS	Delay (sec.) or V/C ratio *	LOS	
1	Brentwood Blvd. @ Delta Rd.	TWSC	862.5 (WB)	F	> 1000 (WB)	F	Yes
2	Delta Rd. @ Sellers Ave.	AWSC	11.6	B	21.1	C	Yes
3	Brentwood Blvd. @ E. Sims Rd.	TWSC	40.8 (WB)	E	166.7 (WB)	F	Yes
4	Brentwood Blvd. @ Lone Tree Wy.	Signal	0.840	D	1.103	F	N/A
5	Arroyo Seco Rd. @ Lone Tree Wy.	TWSC	> 1000 (NB)	F	> 1000 (NB)	F	No
6	Brentwood Blvd. @ Sunrise Dr.	TWSC	62.9 (EB)	F	550.1 (EB)	F	No
7	Brentwood Blvd. @ Gregory Ln.	TWSC	62.6 (EB)	F	455.7 (EB)	F	No
8	Brentwood Blvd. @ Hanson Ln.	TWSC	160.8 (WB)	F	> 1000 (WB)	F	No
9	Brentwood Blvd. @ Homecoming Wy.	TWSC	58.4 (WB)	F	133.5 (WB)	F	No
10	Brentwood Blvd. @ Grant St./Sunset Rd.	Signal	0.768	C	1.015	F	N/A
11	Brentwood Blvd. @ Sunset Ct.	TWSC	17.7 (WB)	C	40.2 (WB)	E	Yes
12	Brentwood Blvd. @ Havenwood Ave.	TWSC	32.4 (WB)	D	513.5 (WB)	F	No
13	Brentwood Blvd. @ Applewood Common	Signal	0.657	B	0.810	D	N/A
14	Brentwood Blvd. @ Sand Creek Rd.	Signal	0.470	A	0.770	C	N/A
15	Brentwood Blvd. @ Technology Wy.	Signal	0.273	A	0.423	A	N/A
16	Brentwood Blvd. @ Nancy St.	Signal	0.385	A	0.542	A	N/A
17	Brentwood Blvd. @ Village Dr.	TWSC	33.1 (WB)	D	352.5 (WB)	F	No
18	Brentwood Blvd. @ Central Blvd./Sycamore Ave.	Signal	0.322	A	0.520	A	N/A
20	Second St. @ Central Blvd.	Signal	0.485	A	0.478	A	N/A
21	Central Blvd. @ Walnut Blvd.	Signal	0.456	A	0.519	A	N/A
22	Brentwood Blvd. @ Pine St.	TWSC	9.6 (WB)	A	9.7 (WB)	A	No
23	Brentwood Blvd. @ Maple St.	TWSC	11.8 (WB)	B	12.6 (WB)	B	No
24	Brentwood Blvd. @ Oak St.	Signal	0.168	A	0.257	A	N/A
25	Oak St. @ Walnut Blvd.	Signal	0.511	A	0.501	A	N/A
26	Brentwood Blvd. @ Second St.	Signal	0.214	A	0.273	A	N/A
27	Brentwood Blvd. @ Chestnut St.	TWSC	10.3 (WB)	B	11.6 (WB)	B	No
28	Brentwood Blvd. @ Balfour Rd.	Signal	0.340	A	0.442	A	N/A
29	Sellers Ave. @ Sunset Rd.	AWSC	13.8	B	18.5	C	No

* Control delay for worst minor approach (worst minor movement) for TWSC.

1. Per the California Manual on Uniform Traffic Control Devices (CMUTCD), September 26, 2006, Section 4C.

Table 14 – Cumulative plus Proposed Project Roadway Segment Levels of Service

#	Intersection	Facility Type	# of Lanes	AM Peak-Hour		PM Peak-Hour	
				PTSF (2 lane) pc/hr/ln (4 lane) ¹	LOS	PTSF (2 lane) pc/hr/ln (4 lane) ¹	LOS
1	Brentwood Blvd. between E. Sims Rd. and Delta Rd.*	Arterial	2	75.5%	E	81.6%	E
2	Brentwood Blvd. Grant St./Sunset Rd. Sunset Ct.*	Arterial	2	83.1%	E	90.8%	E
3	Brentwood Blvd. between Central Blvd. and Spruce St.*	Arterial	4	162 / 185	A / A	356 / 227	A / A
4	Brentwood Blvd. between Oak Ave. and Pine Ave.*	Arterial	4	163 / 89	A / A	146 / 178	A / A
5	Delta Rd. east of Brentwood Blvd.**	Collector	2	51.7%	B	54.9%	B
6	Lone Tree Wy. west of Brentwood Blvd.**	Arterial	4	520 / 479	A / A	619 / 670	B / B
7	Sunset Rd. east of Brentwood Blvd.**	Collector	2	62.7%	C	63.6%	C
8	Central Blvd. west of Brentwood Blvd.**	Arterial	4	326 / 221	A / A	301 / 584	A / B
9	Grant St. west of Brentwood Blvd.**	Collector	2	54.0%	B	43.5%	B
10	Sycamore Ave. east of Brentwood Blvd.**	Collector	2	60.0%	C	61.4%	C

Source: *Highway Capacity Manual, 2000.*

¹ PTSF = Percent Time Spent Following, pc/hr/ln = passenger cars per hour per lane.

* Results for this roadway segment are presented in **NB / SB** format (where applicable).

** Results for this roadway segment are presented in **EB / WB** format (where applicable).

IMPACTS AND MITIGATION

Standards of Significance

Impacts to intersections are created when traffic associated with the proposed project causes the intersection, which operates at acceptable LOS without the proposed project, to fall below a specific threshold. Per the City’s General Plan¹⁶, the City must “maintain a level of service standard of ‘D’ or better throughout the vehicular street system.” Furthermore, according to the General Plan, a significant impact would occur if the addition of the proposed project results in any of the following:

- Causes LOS at a signalized intersection to deteriorate from an acceptable level (LOS D or better with a V/C ratio equal to or less than 0.85) to an unacceptable level (LOS D or worse with a V/C ratio greater than 0.85);
- Causes the V/C ratio at a signalized intersection operating at an unacceptable level (V/C ratio greater than 0.85) to increase by more than 0.01;
- Causes LOS at an unsignalized intersection to degrade to worse than mid-LOS D (average delay of 30 seconds per vehicle) or causes an unsignalized intersection to meet traffic signal warrants based on

¹⁶ *City of Brentwood General Plan, 2001-2021, Circulation Element, January 2006.*

Warrant 3B (peak-hour volume warrant) as presented in the *California Manual of Uniform Traffic Control Devices (CMUTCD)*, dated September 26, 2006;

- Conflicts with existing, planned, or possible future transit, bicycle, and/or pedestrian facilities and services;
- Conflicts or creates demand for public transit services above that which is provided, or planned;
- The path of travel between the project area and transit stops would not meet current California Title 24 handicap accessibility standards; or
- Does not provide connections to bicycle and pedestrian circulation systems of the surrounding area.

Traffic Service Objectives (TSOs) have been developed by the East County Regional Transportation Committee known as TRANSPLAN. TRANSPLAN has included TSOs within the East County Action Plan and defines a significant impact threshold of LOS E for unsignalized intersections along SR-4 (non-freeway segments) between SR-160 and Balfour Road¹⁷.

Existing plus Approved Projects plus Proposed Project Conditions

Intersections

Brentwood Boulevard @ Lone Tree Way

Impact

As shown in Table 9, the addition of the project trips causes this signalized intersection to degrade from LOS A to LOS F during both AM and PM peak-hours. ***This is a significant impact.***

Mitigation

The significant impact at this intersection can be mitigated with lane modifications which include the addition of a left turn and through lane to the northbound approach, resulting in two left turn lanes, two through lanes, and a right turn lane. At the southbound approach, the existing through-right lane should be converted to a through lane and a right turn lane should be added, resulting in one left turn lane, two through lanes, and a right turn lane. In addition, the eastbound and westbound approaches should be modified by converting the existing through-right lanes to through lanes and adding a right turn lane, resulting in a left turn lane, a through lane, and a right turn lane. Additionally, the eastbound right turn lane should be constructed with free movement. As shown in Table 15, incorporation of these lane modifications results in the intersection operating at LOS C and LOS D during the AM and PM peak-hours, respectively. Therefore, ***this impact is less than significant.***

Figure 13 illustrates these recommended lane modifications.

It should be noted that future project applicants within the Brentwood Boulevard Specific Plan boundaries, shall be required to pay the applicable Transportation Development Fee prior to the issuance of building permits or building occupancy. Additionally project specific impacts will be determined and required project improvements will be constructed and/or funded by the various developments to adequately mitigate project impacts.

¹⁷ Per letter from John Sighamony, TRANSPLAN, September 25, 2007.

Arroyo Seco Road at Lone Tree Way

Impact

As shown in Table 9, the addition of the project trips caused this unsignalized intersection, which operates at LOS A during the AM peak hour without the project, to degrade to LOS E. ***This is a significant impact.***

Mitigation

The significant impact at this intersection can be mitigated with four way stop control. As shown in Table 15, the addition of four way stop control results in the intersection operating at LOS B during the AM peak hour and LOS A during the PM peak hour.

It should be noted future project applicants within the Brentwood Boulevard Specific Plan boundaries, shall be required to pay the applicable Transportation Development Fee prior to the issuance of building permits or building occupancy. Additionally project specific impacts will be determined and required project improvements will be constructed and/or funded by the various developments to adequately mitigate project impacts.

As a result of the above, ***this impact is less than significant.***

Brentwood Boulevard at Sunrise Drive

Impact

As shown in Table 9, the addition of the project trips causes this unsignalized intersection to satisfy the peak-hour traffic signal warrant. ***This is a significant impact.***

Mitigation

Operations at this intersection can be improved with signalized traffic control and lane modifications. The lane modifications include the addition of a northbound through lane, resulting in a left turn lane and two through lanes. At the southbound approach, the existing through-right lane should be converted to a through lane, and through lane and a right turn lane should be added, resulting in two through lanes and a right turn lane. The eastbound approach should be modified to include two left turn lanes and a right turn lane. As shown in Table 15, the traffic control and lane modifications results in the intersection operating at LOS C and LOS D (v/c of 0.898) during the AM and PM peak-hours, respectively. The impact noted above does not consider the effects of the SR-4 Bypass or the proposed project replacing existing land uses within the project area. Both of these events is expected to reduce the volumes on up to 22%. With the opening of the Bypass and the replacement of existing land uses, the impact is ***less than significant.***

Figure 13 illustrates these recommended lane modifications.

It should be noted that future project applicants within the Brentwood Boulevard Specific Plan boundaries, shall be required to pay the applicable Transportation Development Fee prior to the issuance of building permits or building occupancy. Additionally project specific impacts will be determined and required project improvements will be constructed and/or funded by the various developments to adequately mitigate project impacts.

Brentwood Boulevard at Gregory Lane

Impact

As shown in Table 9, the addition of the project trips causes the LOS to change from LOS D to LOS F in the AM peak-hour. In addition, the intersection operates at LOS F during the PM peak-hour without the project

and the project causes the intersection to satisfy the peak-hour traffic signal warrant. ***This is a significant impact.***

Mitigation

Traffic operations at this location can be improved by restricting access to Gregory Lane to right-in and right-out only. Restricting left turns results in redistribution of volumes, which is shown in Figure 14. The redirection of left turns resulted in the intersection failing to meet signal warrants in either the AM or PM peak hours. The reconfiguration of the intersection includes removal of the left turn lane and addition of a thru lane at the northbound approach, resulting in two through lanes. At the southbound approach, a through lane and a right turn lane should be added resulting in a two through lanes and right turn lane. At the eastbound approach, left turns should be restricted, resulting in a right turn only lane. As shown in Table 15, the traffic control and lane modifications results in the intersection operating at LOS C and LOS F (62 second delay) during the AM and PM peak-hours, respectively. The impact noted above does not consider the effects of the SR-4 Bypass or the proposed project replacing existing land uses within the project area. Both of these events is expected to reduce the volumes up to 22%. With the opening of the Bypass and the replacement of existing land uses, the impact is ***less than significant.***

Figure 13 illustrates these recommended lane modifications.

It should be noted that future project applicants within the Brentwood Boulevard Specific Plan boundaries, shall be required to pay the applicable Transportation Development Fee prior to the issuance of building permits or building occupancy. Additionally project specific impacts will be determined and required project improvements will be constructed and/or funded by the various developments to adequately mitigate project impacts.

Brentwood Boulevard at Homecoming Way

Impact

As shown in Table 9, the addition of the project trips causes this unsignalized intersection, which operates at LOS F during the AM and PM peak-hours without the project, to satisfy the peak-hour traffic signal warrant for both peak-hours. ***This is a significant impact.***

Mitigation

Operations at this intersection can be improved by restricting access to Homecoming Way to right-in and right-out only. Restricting left turns results in redistribution of volumes, which is shown in Figure 14. The redirection of left turns resulted in the intersection failing to meet signal warrants in either the AM or PM peak hours. The reconfiguration of the intersection includes the addition of a through lane at the northbound approach, resulting in a through lane and a through-right lane. At the southbound approach, the left turn lane should be removed and a through lane should be added, resulting in two through lanes. At the westbound approach, left turns should be restricted, resulting in a right turn only lane. As shown in Table 15, the traffic control and lane modifications results in the intersection operating at LOS E during both the AM (36 second delay) and PM (38 second delay) peak-hours. The impact noted above does not consider the effects of the SR-4 Bypass or the proposed project replacing existing land uses within the project area. Both of these events is expected to reduce the volumes up to 22%. With the opening of the Bypass and the replacement of existing land uses, the impact is ***less than significant.***

Figure 13 illustrates these recommended lane modifications.

It should be noted that future project applicants within the Brentwood Boulevard Specific Plan boundaries, shall be required to pay the applicable Transportation Development Fee prior to the issuance of building permits or building occupancy. Additionally project specific impacts will be determined and required project improvements will be constructed and/or funded by the various developments to adequately mitigate project impacts.

Brentwood Boulevard at Grant Street / Sunset Road

Impact

As shown in Table 9, the addition of the project trips causes this signalized intersection to degrade from LOS C to LOS F during the AM peak-hour. Additionally, the intersection operates below LOS D without the project during the PM peak- hour and the addition of project trips causes the volume to capacity ratio to increase by more than .01. ***This is a significant impact.***

Mitigation

The significant impact at this intersection can be mitigated with lane modifications which include the addition of a through lane at the northbound approach, resulting in one left turn lane, two through lanes, and a right turn lane. At the southbound approach, the existing through-right lane should be converted to a through lane, and a left turn lane, a through lane, and a right turn lane should be added, resulting in two left turn lanes, two through lanes, and a right turn lane. Also, a left turn lane and a right turn lane should be added at the eastbound approach, resulting in a left turn lane, a through lane, and a right turn lane. At the westbound approach, the existing through-left lane should be converted to a through lane. Also, two left turn lanes should be added at the westbound approach, resulting in two left turn lanes, a through lane, and a right turn lane. As shown in Table 15, the traffic control and lane modifications results in the intersection operating at LOS C during the AM peak hour and LOS D during the PM peak-hour. Therefore, ***this impact is less than significant.***

Figure 13 illustrates these recommended lane modifications.

It should be noted that future project applicants within the Brentwood Boulevard Specific Plan boundaries, shall be required to pay the applicable Transportation Development Fee prior to the issuance of building permits or building occupancy. Additionally project specific impacts will be determined and required project improvements will be constructed and/or funded by the various developments to adequately mitigate project impacts.

Brentwood Boulevard at Sunset Court

Impact

As shown in Table 9, the addition of the project trips causes this unsignalized intersection to degrade from LOS C to LOS F during the AM peak-hour, and from LOS D to LOS F during the PM peak-hour. ***This is a significant impact.***

Mitigation

The significant impact at this intersection can be partially mitigated by restricting access to Sunset Court to right-in and right-out only. Restricting left turns results in redistribution of volumes, which is shown in Figure 14. The reconfiguration of the intersection includes addition of a through lane to the northbound approach, resulting in a through lane and a through right lane. At the southbound approach, the left turn lane should be removed and a through lane added, resulting in two through lanes. As shown in Table 15, the traffic control and lane modifications results in the intersection operating at LOS D (32 second delay) during the AM peak

hour and LOS E (44 second delay) during the PM peak hour. The impact noted above does not consider the effects of the SR-4 Bypass or the proposed project replacing existing land uses within the project area. Both of these events is expected to reduce the volumes by up to 22%. With the opening of the Bypass and the replacement of existing land uses, the impact is *less than significant*.

Figure 13 illustrates these recommended lane modifications.

It should be noted that future project applicants within the Brentwood Boulevard Specific Plan boundaries, shall be required to pay the applicable Transportation Development Fee prior to the issuance of building permits or building occupancy. Additionally project specific impacts will be determined and required project improvements will be constructed and/or funded by the various developments to adequately mitigate project impacts.

Brentwood Boulevard at Havenwood Avenue

Impact

As shown in Table 9, the addition of the project trips causes this unsignalized intersection, which operates at LOS F during the AM and PM peak-hours without the project, to satisfy the peak-hour traffic signal warrant for both peak-hours. ***This is a significant impact.***

Mitigation

The significant impact at this intersection can be partially mitigated by restricting access to Havenwood Avenue to right-in and right-out only. Restricting left turns results in redistribution of volumes, which is shown in Figure 14. The redirection of left turns resulted in the intersection failing to meet signal warrants in either the AM or PM peak hours. The reconfiguration of the intersection includes removal of the existing left turn lane and addition of a through lane at the northbound approach, resulting in two through lanes and a right turn lane. At the southbound approach, the existing left turn lane should be removed and a through lane should be added, resulting in a through lane and a through-right lane. At both the eastbound and westbound approaches, left turns should be restricted to result in right turn only lanes. As shown in Table 15, the traffic control and lane modifications results in the intersection operating at LOS E during the AM (35 second delay) and PM (43 second delay) peak hours, respectively. The impact noted above does not consider the effects of the SR-4 Bypass or the proposed project replacing existing land uses within the project area. Both of these events is expected to reduce the volumes up to 22%. With the opening of the Bypass and the replacement of existing land uses, the impact is *less than significant*.

Figure 13 illustrates these recommended lane modifications.

Brentwood Boulevard at Applewood Common

Impact

As shown in Table 9, the addition of the project trips causes this signalized intersection to degrade from LOS B to LOS F during the AM peak-hour, and degrades operations during the PM peak-hour from an unacceptable LOS D to LOS F. ***This is a significant impact.***

Mitigation

The significant impact at this intersection can be mitigated with lane modifications which include adding a through lane to the northbound approach, resulting in one left turn lane, two through lanes, and one right turn lane. At the southbound approach, the existing through-right lane should be converted to a through lane, and a through lane and a right turn lane should be added, resulting in one left turn lane, two through lanes, and one right turn lane. At the eastbound approach, the existing through-right lane should be converted to a

through lane, and a right turn lane should be added, resulting in a left turn lane, a through lane, and a right turn lane. At the westbound approach, the existing through-right lane should be converted to a through lane, and a left turn lane and a right turn lane should be added, resulting in two left turn lanes, one through lane, and a right turn lane. As shown in Table 15, incorporation of these lane modifications results in the intersection operating at LOS C and LOS D during the AM and PM peak-hours, respectively. The impact noted above does not consider the effects of the SR-4 Bypass or the proposed project replacing existing land uses within the project area. Both of these events is expected to reduce the volumes up to 22%. Therefore, ***this impact is less than significant.***

Figure 13 illustrates these recommended lane modifications.

It should be noted that future project applicants within the Brentwood Boulevard Specific Plan boundaries, shall be required to pay the applicable Transportation Development Fee prior to the issuance of building permits or building occupancy. Additionally project specific impacts will be determined and required project improvements will be constructed and/or funded by the various developments to adequately mitigate project impacts.

Brentwood Boulevard at Sand Creek Road

Impact

As shown in Table 9, the addition of the project trips causes this signalized intersection to degrade from LOS A to LOS F during the AM and PM peak-hours. ***This is a significant impact.***

Mitigation

The significant impact at this intersection can be mitigated with lane modifications which include the addition of a right turn lane to the northbound approach, assuming that Sand Creek Road will be extended to the east as needed for project access. This will result in a left turn lane, two through lanes, and a right turn lane at the northbound approach. Also, the existing through-right lane should be converted to a through lane, and a right turn lane should be added at the southbound approach, resulting in a left turn lane, two through lanes, and a right turn lane. A left-turn lane and two through lanes should be added at the eastbound approach, resulting in two left turn lanes, two through lanes, and a right turn lane. A left turn lane, a through lane, a through-right lane, and a right turn lane are assumed at the westbound approach, which does not currently exist. As shown in Table 15, incorporation of these lane modifications results in the intersection operating at LOS C and LOS D during the AM and PM peak-hours, respectively. Therefore, ***this impact is less than significant.***

Figure 13 illustrates these recommended lane modifications.

It should be noted that future project applicants within the Brentwood Boulevard Specific Plan boundaries, shall be required to pay the applicable Transportation Development Fee prior to the issuance of building permits or building occupancy. Additionally project specific impacts will be determined and required project improvements will be constructed and/or funded by the various developments to adequately mitigate project impacts.

Brentwood Boulevard at Village Drive

Impact

As shown in Table 9, the addition of the project trips causes this unsignalized intersection, which operates at LOS E and LOS F during the AM and PM peak-hours, respectively, without the project, to satisfy the peak-hour traffic signal warrant. ***This is a significant impact.***

Mitigation

The significant impact at this intersection can be mitigated by restricting access to Village Drive to right-in and right-out only. Restricting left turns results in redistribution of volumes, which is shown in Figure 14. The redirection of left turns resulted in the intersection failing to meet signal warrants in either the AM or PM peak hours. The reconfiguration of the intersection includes addition of a through lane at the northbound approach, resulting in a through lane and a through-right lane. At the southbound approach, the left turn lane should be removed, resulting in a single through lane. The westbound approach should be restricted to right turns only. As shown in Table 15, the lane modifications results in the intersection operating at LOS C during the AM and PM peak hours. Therefore, ***this impact is less than significant.***

Figure 13 illustrates these recommended lane modifications.

It should be noted that future project applicants within the Brentwood Boulevard Specific Plan boundaries, shall be required to pay the applicable Transportation Development Fee prior to the issuance of building permits or building occupancy. Additionally project specific impacts will be determined and required project improvements will be constructed and/or funded by the various developments to adequately mitigate project impacts.

Brentwood Boulevard at Central Boulevard / Sycamore Avenue

Impact

As shown in Table 9, the addition of the project trips causes this signalized intersection to degrade from LOS A to LOS E during the AM peak-hour, and from LOS C to LOS E during the PM peak-hour. ***This is a significant impact.***

Mitigation

The significant impact at this intersection can be mitigated with lane modifications of the northbound approach which include converting the existing right turn lane into a through-right lane, resulting in a left turn lane, a through lane, and a through-right lane. As shown in Table 15, incorporation of this lane modification results in the intersection operating at LOS B during the AM and PM peak-hours. Therefore, ***this impact is less than significant.***

Figure 13 illustrates these recommended lane modifications.

It should be noted that future project applicants within the Brentwood Boulevard Specific Plan boundaries, shall be required to pay the applicable Transportation Development Fee prior to the issuance of building permits or building occupancy. Additionally project specific impacts will be determined and required project improvements will be constructed and/or funded by the various developments to adequately mitigate project impacts.

Brentwood Boulevard at Pine Street

Impact

As shown in Table 9, the addition of project trip causes this signalized intersection to degrade from LOS C in the PM peak hour to LOS E. ***This is a significant impact.***

Mitigation

The significant impact at this intersection can be mitigated by restricting access to Pine Street to right-in and right-out only. Restricting left turns results in redistribution of volumes, which is shown in Figure 14. The reconfiguration of the intersection includes restriction of left turns at southbound and westbound approaches. This results in two through lanes for the southbound approach, and a right turn lane for the westbound approach. As shown in Table 15, incorporation of these lane modification results in the intersection operating at LOS B during the AM and PM peak-hours. Therefore, ***this impact is less than significant.***

Brentwood Boulevard at Chestnut Street

Impact

As shown in Table 9, the addition of the project trips causes this unsignalized intersection to degrade from LOS D to LOS F during the AM peak-hour. ***This is a significant impact.***

Mitigation

The significant impact at this intersection can be mitigated by restricting access to Chestnut Street to right-in and right-out only. Restricting left turns results in redistribution of volumes, which is shown in Figure 14. The reconfiguration of the intersection includes restriction of left turns at the southbound and westbound approaches, resulting in a through-right lane at the northbound approach, one through lane at the southbound approach, and a right turn only lane at the westbound approach. As shown in Table 15, incorporation of these lane modifications results in the intersection operating at LOS C during both the AM and PM peak-hours. Therefore, ***this impact is less than significant.***

It should be noted that future project applicants within the Brentwood Boulevard Specific Plan boundaries, shall be required to pay the applicable Transportation Development Fee prior to the issuance of building permits or building occupancy. Additionally project specific impacts will be determined and required project improvements will be constructed and/or funded by the various developments to adequately mitigate project impacts.

Brentwood Boulevard at Balfour Road

Impact

As shown in Table 9, the addition of the project trips causes this signalized intersection to degrade from LOS C to LOS E during the PM peak-hour. ***This is a significant impact.***

Mitigation

The significant impact at this intersection can be mitigated with lane modifications which include the addition of a northbound left turn lane resulting in two left turn lanes, two through lanes, and one right turn lane. As shown in Table 15, incorporation of this lane modification results in the intersection operating at LOS C during the PM peak-hour. Therefore, ***this impact is less than significant.***

Figure 13 illustrates this recommended lane modification.

It should be noted that future project applicants within the Brentwood Boulevard Specific Plan boundaries, shall be required to pay the applicable Transportation Development Fee prior to the issuance of building permits or building occupancy. Additionally project specific impacts will be determined and required project improvements will be constructed and/or funded by the various developments to adequately mitigate project impacts.

Roadway Segments

Lone Tree Way west of Brentwood Boulevard

Impact

As shown in Table 10, the addition of the project causes this study roadway segment, which operates at LOS C during both AM and PM peak-hours without the proposed project, to degrade to LOS E for both peak-hours. ***This is a significant impact.***

Mitigation

The significant impact along this roadway segment during the AM and PM peak-hours can be mitigated with conversion to a four lane roadway. As shown in Table 16, this improvement is expected to allow this study roadway segment to operate at LOS A or LOS B under both peak-hours and the impact will be ***less than significant.***

It should be noted that future project applicants within the Brentwood Boulevard Specific Plan boundaries, shall be required to pay the applicable Transportation Development Fee prior to the issuance of building permits or building occupancy. Additionally project specific impacts will be determined and required project improvements will be constructed and/or funded by the various developments to adequately mitigate project impacts.

Bicycle and Pedestrian Facilities

The City's General Plan states, "as roadways are widened and improved, the City is requiring designs incorporating six to eight-foot wide bike lanes. . . Sidewalks are part of the City's standard plans for arterial, collector, and local streets". Build-out of the specific plan land uses are expected to be required to include City's requirements that accomplish the above mentioned goals regarding the implementation of bicycle and pedestrian facilities. As a result, impacts would be expected to be ***less than significant.***

Transit Facilities

Tri Delta Transit has published a short range transit plan that provides information regarding ridership trends for its bus routes¹⁸. The short range plan indicates Route 391, which runs along Brentwood Boulevard has an average of 15.8 passengers per revenue hour and runs on 30 minute headways during the peak hour. Similarly, Route 300, which is an express route, has an average ridership of 9.8 passengers per revenue hour and operates on 20 minute headways. It is assumed buses operated on these routes have a seating capacity of 39 passengers. As such, the proposed project is not expected to exceed the capacity of the transit service and the impact is expected to be ***less than significant.***

Cumulative plus Proposed Project Conditions

Intersections

There were no significant impacts due to the addition of the project trips under this analysis scenario.

Roadway Segments

There were no significant impacts due to the addition of the project trips under this analysis scenario.

¹⁸ *Tri Delta Transit Short Range Transit Plan FY 2005-2015, Final*, Eastern Contra Costa Transit Authority, January 2006.

Bicycle and Pedestrian Facilities

The City's General Plan states, "as roadways are widened and improved, the City is requiring designs incorporating six to eight-foot wide bike lanes...Sidewalks are part of the City's standard plans for arterial, collector, and local streets". Build-out of the specific plan land uses are expected to be required to include City's requirements that accomplish the abovementioned goals regarding the implementation of bicycle and pedestrian facilities. As a result, impacts would be expected to be *less than significant*.

Transit Facilities

Tri Delta Transit has published a short range transit plan that notes "significant service expansion is warranted to address commute travel congestion on Highway 4 [and]...rapid development areas in...Brentwood..." The short range plan identifies the need to "refine and expand local routes in Brentwood..." prior to 2010. However, the plan also notes "estimated revenues are not sufficient to support system expansion." In addition, the Transit service to the City of Brentwood will be enhanced in 2010 with the expected opening of the eBART system. At such time, the #300 commuter route will be come redundant with the BART service¹⁹.

As a result, of the eBART line, the short range plan recommends "BART feeder bus services, including Tri Delta Transit routes #300, #390 and #391 should be discontinued. The resources should be redeployed into local service to support existing routes as necessary and new feeder routes." Per City staff, eBART service to Hillcrest is expected no sooner than 2015.

As noted for the Existing Plus Approved Project Plus the Proposed Project Scenario, transit ridership is currently significantly below capacity. In addition, it is anticipated the adjustments to service will be based on ridership trends, including those in the project area. As a result, the impact of the project on transit services is expected to be *less than significant*.

¹⁹ *Tri Delta Transit Short Range Transit Plan FY 2005-2015, Final*, Eastern Contra Costa Transit Authority, January 2006.

Table 15 – Existing plus Approved Projects plus Proposed Project Intersection Levels of Service With Mitigations

#	Intersection	Traffic Control	AM Peak-Hour		PM Peak-Hour		Meets Signal Warrant ¹
			Delay (sec.) or V/C ratio*	LOS	Delay (sec.) or V/C ratio*	LOS	
4	Brentwood Blvd. @ Lone Tree Wy.	Signal	0.720	C	0.800	D	N/A
5	Arroyo Seco Rd. @ Lone Tree Wy.	AWSC	13.3	B	9.8	A	No
6	Brentwood Blvd. @ Sunrise Dr.	Signal	0.748	C	0.898	D	N/A
7	Brentwood Blvd. @ Gregory Ln.	TWSC	19.5	C	62.6	F	No
9	Brentwood Blvd. @ Homecoming Wy.	TWSC	36.1	E	38.6	E	No
10	Brentwood Blvd. @ Grant St./Sunset Rd.	Signal	0.784	C	0.851	D	N/A
11	Brentwood Blvd. @ Sunset Ct.	TWSC	32.0	D	43.9	E	No
12	Brentwood Blvd. @ Havenwood Ave.	TWSC	35.3	E	42.8	E	No
13	Brentwood Blvd. @ Applewood Common	Signal	0.714	C	0.840	D	N/A
14	Brentwood Blvd. @ Sand Creek Rd.	Signal	0.762	C	0.847	D	N/A
17	Brentwood Blvd. @ Village Dr.	TWSC	18.4	C	22.3	C	No
18	Brentwood Blvd. @ Central Blvd./Sycamore Ave.	Signal	0.644	B	0.675	B	N/A
22	Brentwood Blvd. @ Pine St.	TWSC	13.9	B	13.5	B	No
27	Brentwood Blvd. @ Chestnut St.	TWSC	19.6	C	18.0	C	No
28	Brentwood Blvd. @ Balfour Rd.	Signal	0.623	B	0.772	C	N/A

* Control delay for worst minor approach (worst minor movement) for TWSC.
 1. Per the California Manual on Uniform Traffic Control Devices (CMUTCD), September 26, 2006, Section 4C.

Table 16 - Existing plus Approved Projects plus Proposed Project Segment Levels of Service With Mitigations

#	Intersection	Facility Type	# of Lanes	AM Peak-Hour		PM Peak-Hour	
				PTSF (2 lane) pc/hr/ln (4 lane) ¹	LOS	PTSF (2 lane) pc/hr/ln (4 lane) ¹	LOS
6	Lone Tree Wy. west of Brentwood Blvd.**	Arterial	4	524 / 456	B / A	402 / 573	A / B

Source: *Highway Capacity Manual, 2000.*
¹ PTSF = Percent Time Spent Following, pc/hr/ln = passenger cars per hour per lane.

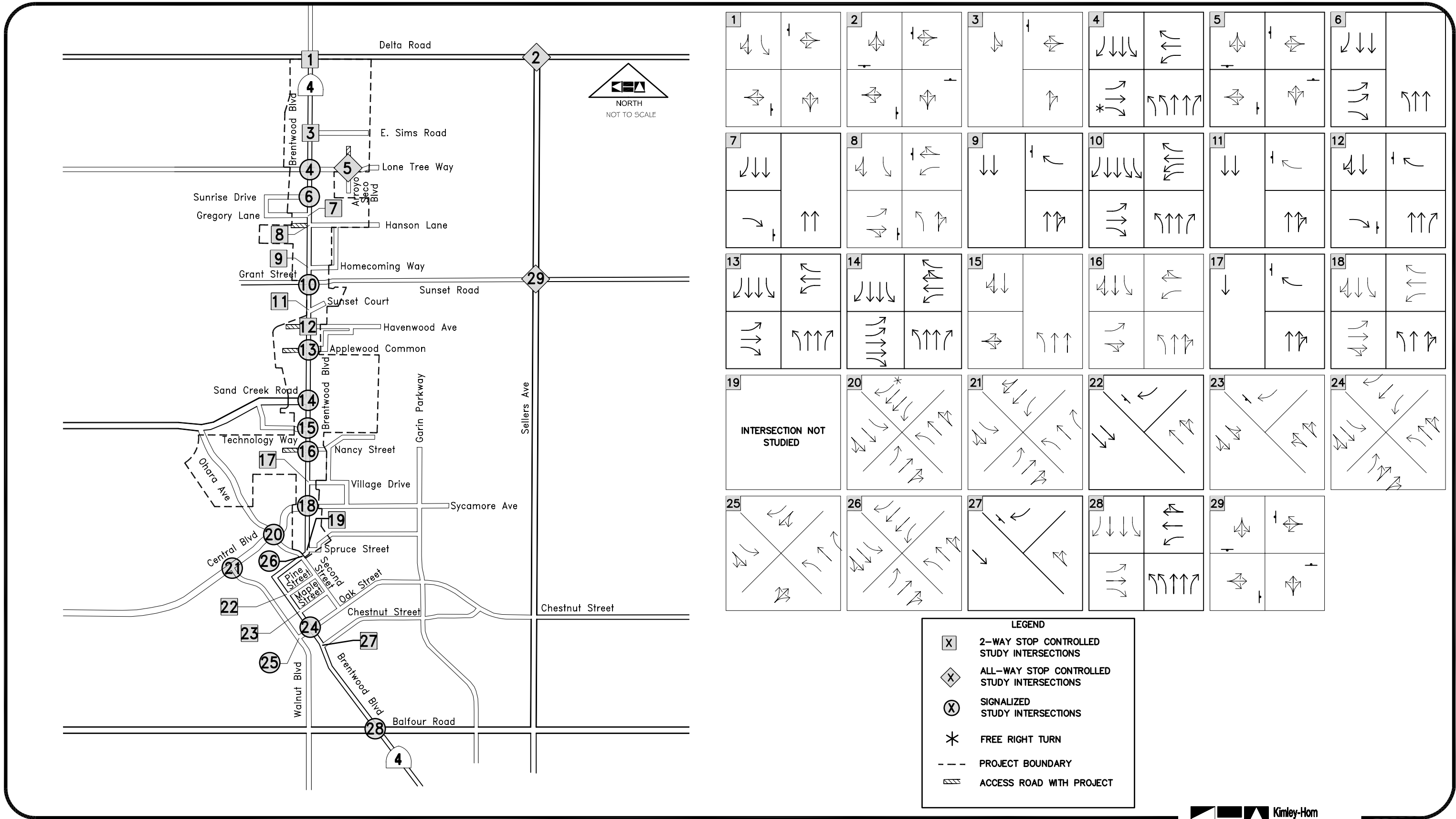


FIGURE 13
 EXISTING PLUS APPROVED PROJECTS PLUS PROPOSED PROJECT MITIGATIONS



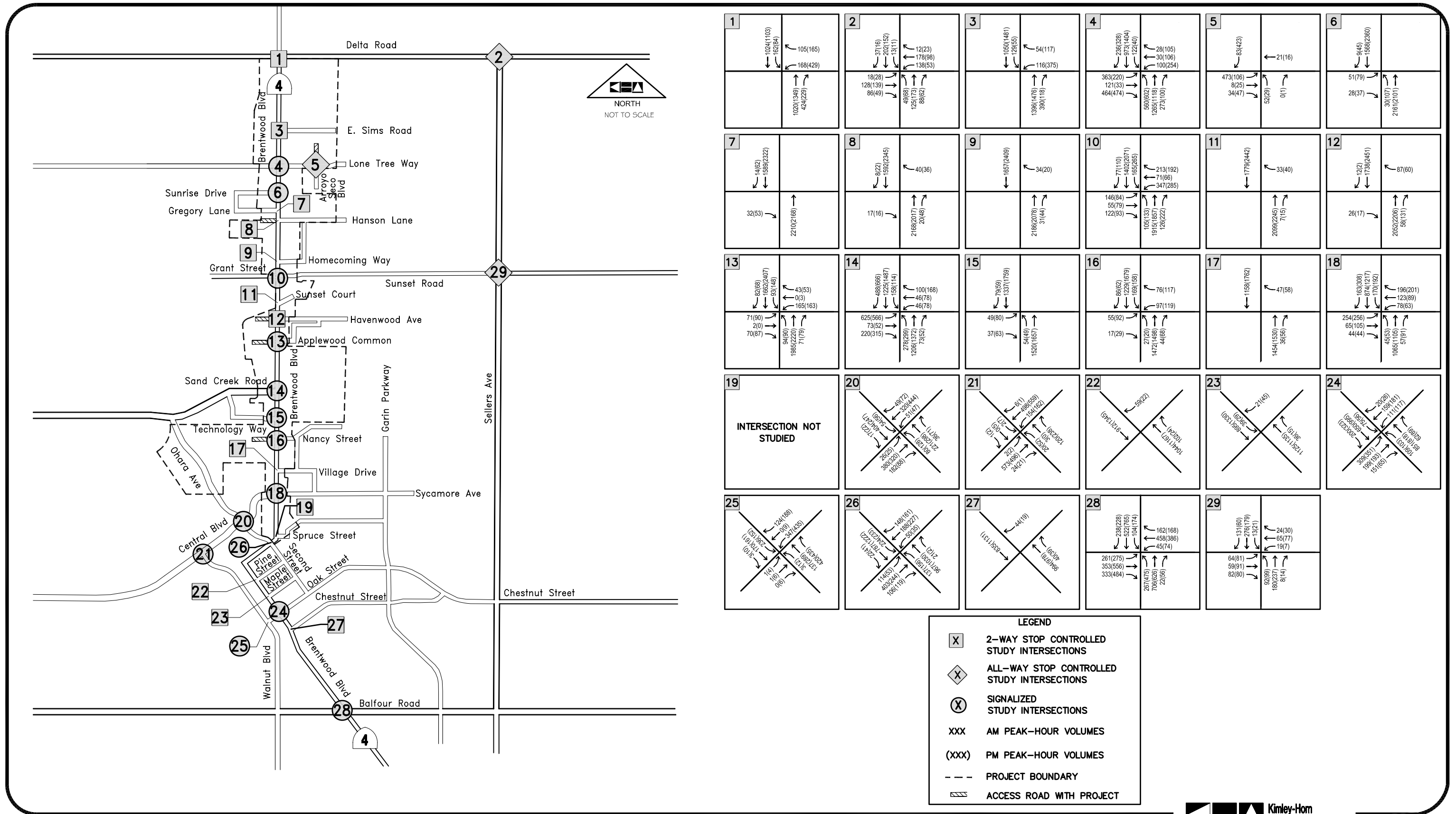


FIGURE 14
 EXISTING PLUS APPROVED PROJECT PLUS PROPOSED PROJECT PEAK-HOUR TRAFFIC VOLUMES WITH MITIGATIONS



CONCLUSIONS

Based upon the analysis documented in this report, the following conclusions are offered:

- The proposed project is expected to generate 31,202 daily trips, including 3,704 AM peak-hour trips and 4,072 PM peak-hour trips.
- The addition of the Proposed Project to the Existing plus Approved Projects Conditions results in a significant impact at fourteen study intersections and one study roadway segment. These impacts can be mitigated to be *less than significant*.
- The addition of the Proposed Project to the Cumulative Conditions does not result in any significant impacts.
- The addition of the Proposed Project to Existing plus Approved Projects and Cumulative Conditions is not expected to result in any significant impacts to bicycle, pedestrian, or transit facilities.

Appendix A:

*Analysis Worksheets for
Existing Conditions*

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 Brentwood Blvd. @ Delta Road

Average Delay (sec/veh): 5.8 Worst Case Level Of Service: E[48.0]

Table with columns for Street Name (Brentwood Blvd., Delta Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach and movement.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach and movement.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach and movement.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

 Level Of Service Computation Report
 2000 HCM 4-Way Stop Method (Base Volume Alternative)

 Intersection #2 Sellers Ave @ Delta

Cycle (sec): 100 Critical Vol./Cap.(X): 0.410
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 11.6
 Optimal Cycle: 0 Level Of Service: B

Street Name: Sellers Ave. Delta Rd.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0

Volume Module:

Base Vol:	44	125	69	13	202	37	18	107	85	110	123	12
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	44	125	69	13	202	37	18	107	85	110	123	12
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	44	125	69	13	202	37	18	107	85	110	123	12
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	44	125	69	13	202	37	18	107	85	110	123	12
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	44	125	69	13	202	37	18	107	85	110	123	12

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.18	0.53	0.29	0.05	0.80	0.15	0.09	0.51	0.40	0.45	0.50	0.05
Final Sat.:	114	324	179	32	493	90	52	311	247	268	300	29

Capacity Analysis Module:

Vol/Sat:	0.39	0.39	0.39	0.41	0.41	0.41	0.34	0.34	0.34	0.41	0.41	0.41
Crit Moves:	****			****			****			****		
Delay/Veh:	11.4	11.4	11.4	11.8	11.8	11.8	10.9	10.9	10.9	12.1	12.1	12.1
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	11.4	11.4	11.4	11.8	11.8	11.8	10.9	10.9	10.9	12.1	12.1	12.1
LOS by Move:	B	B	B	B	B	B	B	B	B	B	B	B
ApproachDel:	11.4			11.8			10.9			12.1		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	11.4			11.8			10.9			12.1		
LOS by Appr:	B			B			B			B		
AllWayAvgQ:	0.5	0.5	0.5	0.6	0.6	0.6	0.4	0.4	0.4	0.6	0.6	0.6

 Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #3 Brentwood Blvd. @ E. Sims Road

Average Delay (sec/veh): 2.3 Worst Case Level Of Service: D[30.7]

Table with columns for Street Name (Brentwood Blvd., E. Sims Road), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 0, 0, 0, 0, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach and movement.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach and movement.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach and movement.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #4 Brentwood Blvd @ Lone Tree Way (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.462
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 42 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Lone Tree Way (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include, Ovl), Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #5 Arroyo Seco Road @ Lone Tree Way

Average Delay (sec/veh): 5.0 Worst Case Level Of Service: A[8.8]

Table with columns for Street Name (Arroyo Seco Road, Lone Tree Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Stop Sign, Uncontrolled), Rights (Include), and Lanes (1 0 0 0 0, etc.).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS, and values for each approach.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #6 Brentwood Blvd. @ Sunrise Drive

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: D[25.3]

Table with columns for Street Name (Brentwood Blvd., Sunrise Dr.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #7 Brentwood Blvd. @ Gregory Lane

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: D[26.1]

Table with columns for Street Name (Brentwood Blvd., Gregory Ln.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1 0 0 1 0, 1 0 0 1 0, 0 0 1! 0 0, 0 0 1! 0 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each of the four approaches.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each of the four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each of the four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS, with values for each of the four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #8 Brentwood Blvd. @ Hanson Lane

Average Delay (sec/veh): 1.0 Worst Case Level Of Service: D[31.3]

Table with columns for Street Name (Brentwood Blvd., Hanson Ln), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 0, 0, 0, 0, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for Brentwood Blvd. and Hanson Ln.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for Brentwood Blvd. and Hanson Ln.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for Brentwood Blvd. and Hanson Ln.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS, and values for Brentwood Blvd. and Hanson Ln.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #9 Brentwood Blvd. @ Homecoming Way

Average Delay (sec/veh): 0.9 Worst Case Level Of Service: D[27.9]

Table with columns for Street Name (Brentwood Blvd., Homecoming Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume. Columns for Brentwood Blvd. (L, T, R) and Homecoming Way (L, T, R).

Critical Gap Module: Critical Gp, FollowUpTim. Columns for Brentwood Blvd. (L, T, R) and Homecoming Way (L, T, R).

Capacity Module: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap. Columns for Brentwood Blvd. (L, T, R) and Homecoming Way (L, T, R).

Level Of Service Module: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS. Columns for Brentwood Blvd. (L, T, R) and Homecoming Way (L, T, R).

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #10 Brentwood Blvd. @ Grant Street / Sunset Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.628
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 61 Level Of Service: B

Street Name: Brentwood Blvd. Grant Street / Sunset Rd.
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Protected Protected Split Phase Split Phase
Rights: Include Include Include Include
Min. Green: 4 10 10 4 10 10 4 4 4 4 4 4 4
Lanes: 1 0 1 0 1 1 0 0 1 0 0 0 1! 0 0 0 1 0 0 1

Volume Module:
Base Vol: 51 628 76 63 551 51 35 53 46 146 65 119
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 51 628 76 63 551 51 35 53 46 146 65 119
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 51 628 76 63 551 51 35 53 46 146 65 119
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 51 628 76 63 551 51 35 53 46 146 65 119
RTOR Reduct: 0 0 76 0 0 0 0 0 0 0 0 0 63
RTOR Vol: 51 628 0 63 551 51 35 53 46 146 65 56
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 51 628 0 63 551 51 35 53 46 146 65 56

Saturation Flow Module:
Sat/Lane: 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 1.00 1.00 1.00 0.92 0.08 0.26 0.40 0.34 0.69 0.31 1.00
Final Sat.: 1650 1650 1650 1650 1510 140 431 653 566 1142 508 1650

Capacity Analysis Module:
Vol/Sat: 0.03 0.38 0.00 0.04 0.36 0.36 0.08 0.08 0.08 0.13 0.13 0.03
Crit Volume: 628 63 134 211
Crit Moves: **** **** **** ****

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #11 Brentwood Blvd. @ Sunset Ct.

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: C[15.4]

Table with columns for Street Name (Brentwood Blvd., Sunset Ct.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS, and values for each approach.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #12 Brentwood Blvd. @ Havenwood Ave.

Average Delay (sec/veh): 0.6 Worst Case Level Of Service: C[23.4]

Table with columns for Street Name (Brentwood Blvd., Havenwood Ave.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 1, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #13 Brentwood Blvd. @ Applewood Common

Cycle (sec): 100 Critical Vol./Cap.(X): 0.484
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 44 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Applewood Common), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green (4, 8, 8), and Lanes (1, 0, 1, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across various movement categories.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across various movement categories.

Level of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #14 Brentwood Blvd. @ Sand Creek Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.361
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 29 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Sand Creek Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green (4, 10, 10), and Lanes (1, 0, 2, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #15 Brentwood Blvd. @ Technology Way

Cycle (sec): 100 Critical Vol./Cap.(X): 0.187
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 23 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Technology Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green (4, 6, 6, 0, 6, 6, 4, 4, 4, 0, 0, 0), and Lanes (1, 0, 2, 0, 0, 0, 0, 1, 1, 0, 1, 0, 0, 0, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume, with values for 12 different movement categories.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat., with values for 12 different movement categories.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves, with values for 12 different movement categories.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #16 Brentwood Blvd @ Nancy Street (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.239
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 24 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Nancy Street (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and FinalVolume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #17 Brentwood Blvd. @ Village Drive

Average Delay (sec/veh): 1.0 Worst Case Level Of Service: C[19.9]

Table with columns for Street Name (Brentwood Blvd., Village Drive), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #18 Brentwood Blvd. @ Central Blvd / Sycamore Ave (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.384
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 32 Level Of Service: A

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Central Blvd / Sycamore Ave (S) with various movement details.

Volume Module table showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across different approaches.

Saturation Flow Module table showing Sat/Lane, Adjustment, Lanes, and Final Sat. values for different approaches.

Capacity Analysis Module table showing Vol/Sat, Crit Volume, and Crit Moves for different approaches.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #20 Central Street @ Second Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.266
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 34 Level Of Service: A

Table with columns for Street Name (Second Street, Central Boulevard), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include, Ignore), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #21 Central Blvd @ Walnut Blvd

Cycle (sec): 100 Critical Vol./Cap.(X): 0.205
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 36 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Central Blvd), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Table for Volume Module showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various approaches.

Table for Saturation Flow Module showing Sat/Lane, Adjustment, Lanes, and Final Sat. values.

Table for Capacity Analysis Module showing Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #22 Brentwood Blvd @ Pine Street

Average Delay (sec/veh): 1.0 Worst Case Level Of Service: B[11.8]

Table with columns for Street Name (Brentwood Blvd, Pine Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume across movements.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim across movements.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap across movements.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS across movements.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #23 Brentwood Blvd @ Maple Street

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: B[10.0]

Table with columns for Street Name (Brentwood Blvd, Maple Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS, and values for each approach.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #24 Brentwood Blvd @ Oak Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.391
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 37 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Oak St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green (4, 10, 10), and Lanes (1, 0, 1, 1, 0).

Table for Volume Module with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Table for Saturation Flow Module with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across various movement categories.

Table for Capacity Analysis Module with columns for Vol/Sat, Crit Volume, and Crit Moves across various movement categories.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #25 Walnut Blvd @ Oak St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.368
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 36 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Oak St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green (5, 7, 7, 0, 7, 7, 5, 5, 5, 5, 5, 5), and Lanes (1, 0, 1, 0, 1, 1, 0, 0, 1, 0, 0, 1, 0, 0, 0, 0, 0, 1, 0, 0, 1).

Volume Module:
Base Vol: 3 128 276 218 154 3 1 1 0 259 0 110
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 3 128 276 218 154 3 1 1 0 259 0 110
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 3 128 276 218 154 3 1 1 0 259 0 110
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 3 128 276 218 154 3 1 1 0 259 0 110
RTOR Reduct: 0 0 259 0 0 0 0 0 0 0 0 0 110
RTOR Vol: 3 128 17 218 154 3 1 1 0 259 0 0
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 3 128 17 218 154 3 1 1 0 259 0 0

Saturation Flow Module:
Sat/Lane: 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 1.00 1.00 1.00 0.98 0.02 0.50 0.50 0.00 1.00 0.00 1.00
Final Sat.: 1650 1650 1650 1650 1618 32 825 825 0 1650 0 1650

Capacity Analysis Module:
Vol/Sat: 0.00 0.08 0.01 0.13 0.10 0.10 0.00 0.00 0.00 0.16 0.00 0.00
Crit Volume: 128 218 2 259
Crit Moves: **** **** **** ****

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #26 Brentwood Blvd. @ Second St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.375
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 30 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Second St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Permitted), Rights (Include), Min. Green (4, 8, 8), and Lanes (1, 0, 1, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across various movement categories.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across various movement categories.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #27 Brentwood Blvd. @ Chestnut St.

Average Delay (sec/veh): 1.3 Worst Case Level Of Service: C[15.9]

Table with columns for Street Name (Brentwood Blvd., Chestnut St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #28 Brentwood Blvd. @ Balfour Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.529
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 48 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Balfour Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Ignore, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

 Level Of Service Computation Report
 2000 HCM 4-Way Stop Method (Base Volume Alternative)

 Intersection #29 Sellers Ave. @ Sunet Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.552
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 11.9
 Optimal Cycle: 0 Level Of Service: B

Street Name: Sellers Ave. Sunset Rd.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0

Volume Module:

Base Vol:	77	175	8	13	275	103	45	59	65	19	65	24
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	77	175	8	13	275	103	45	59	65	19	65	24
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	77	175	8	13	275	103	45	59	65	19	65	24
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	77	175	8	13	275	103	45	59	65	19	65	24
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	77	175	8	13	275	103	45	59	65	19	65	24

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.30	0.67	0.03	0.03	0.71	0.26	0.27	0.35	0.38	0.18	0.60	0.22
Final Sat.:	193	439	20	24	498	187	158	207	228	99	338	125

Capacity Analysis Module:

Vol/Sat:	0.40	0.40	0.40	0.55	0.55	0.55	0.28	0.28	0.28	0.19	0.19	0.19
Crit Moves:	****			****			****			****		
Delay/Veh:	11.4	11.4	11.4	13.4	13.4	13.4	10.4	10.4	10.4	9.9	9.9	9.9
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	11.4	11.4	11.4	13.4	13.4	13.4	10.4	10.4	10.4	9.9	9.9	9.9
LOS by Move:	B	B	B	B	B	B	B	B	B	A	A	A
ApproachDel:	11.4			13.4			10.4			9.9		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	11.4			13.4			10.4			9.9		
LOS by Appr:	B			B			B			A		
AllWayAvgQ:	0.6	0.6	0.6	1.1	1.1	1.1	0.3	0.3	0.3	0.2	0.2	0.2

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 Brentwood Blvd. @ Delta Road

Average Delay (sec/veh): 18.9 Worst Case Level Of Service: F[178.4]

Table with columns for Street Name (Brentwood Blvd., Delta Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

 Level Of Service Computation Report
 2000 HCM 4-Way Stop Method (Base Volume Alternative)

 Intersection #2 Sellers Ave @ Delta

Cycle (sec): 100 Critical Vol./Cap.(X): 0.382
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 9.9
 Optimal Cycle: 0 Level Of Service: A

Street Name: Sellers Ave. Delta Rd.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0

Volume Module:

Base Vol:	67	173	31	11	152	16	28	84	45	32	72	23
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	67	173	31	11	152	16	28	84	45	32	72	23
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	67	173	31	11	152	16	28	84	45	32	72	23
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	67	173	31	11	152	16	28	84	45	32	72	23
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	67	173	31	11	152	16	28	84	45	32	72	23

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.25	0.64	0.11	0.06	0.85	0.09	0.18	0.53	0.29	0.25	0.57	0.18
Final Sat.:	175	453	81	42	582	61	118	353	189	162	364	116

Capacity Analysis Module:

Vol/Sat:	0.38	0.38	0.38	0.26	0.26	0.26	0.24	0.24	0.24	0.20	0.20	0.20
Crit Moves:	****			****			****			****		
Delay/Veh:	10.6	10.6	10.6	9.6	9.6	9.6	9.5	9.5	9.5	9.3	9.3	9.3
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	10.6	10.6	10.6	9.6	9.6	9.6	9.5	9.5	9.5	9.3	9.3	9.3
LOS by Move:	B	B	B	A	A	A	A	A	A	A	A	A
ApproachDel:	10.6			9.6			9.5			9.3		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	10.6			9.6			9.5			9.3		
LOS by Appr:	B			A			A			A		
AllWayAvgQ:	0.5	0.5	0.5	0.3	0.3	0.3	0.3	0.3	0.3	0.2	0.2	0.2

 Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #3 Brentwood Blvd. @ E. Sims Road

Average Delay (sec/veh): 3.4 Worst Case Level Of Service: F[82.2]

Table with columns for Street Name (Brentwood Blvd., E. Sims Road), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 0, 0, 0, 0, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach and movement.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach and movement.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach and movement.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #4 Brentwood Blvd @ Lone Tree Way (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.453
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 42 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Lone Tree Way (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include, Ovl), Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across various movement categories.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across various movement categories.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #5 Arroyo Seco Road @ Lone Tree Way

Average Delay (sec/veh): 3.2 Worst Case Level Of Service: A[8.8]

Table with columns for Street Name (Arroyo Seco Road, Lone Tree Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Stop Sign, Uncontrolled), Rights (Include), and Lanes (0 0 1! 0 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS, and values for each approach.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #6 Brentwood Blvd. @ Sunrise Drive

Average Delay (sec/veh): 1.1 Worst Case Level Of Service: F[72.4]

Street Name: Brentwood Blvd. Sunrise Dr.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	0	0	0	0	0	1	0	0	0

Volume Module:

Base Vol:	26	1012	0	0	814	19	10	0	9	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	26	1012	0	0	814	19	10	0	9	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.85	0.85	0.85	0.87	0.87	0.87	0.68	0.68	0.68	1.00	1.00	1.00
PHF Volume:	31	1189	0	0	935	22	15	0	13	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	31	1189	0	0	935	22	15	0	13	0	0	0

Critical Gap Module:

Critical Gp:	4.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	6.5	6.2	xxxxxx	xxxx	xxxxxx
FollowUpTim:	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	4.0	3.3	xxxxxx	xxxx	xxxxxx

Capacity Module:

Cnflct Vol:	956	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	2196	2196	945	xxxx	xxxx	xxxxxx
Potent Cap.:	688	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	50	45	317	xxxx	xxxx	xxxxxx
Move Cap.:	688	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	48	43	317	xxxx	xxxx	xxxxxx
Volume/Cap:	0.04	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	0.31	0.00	0.04	xxxx	xxxx	xxxxxx

Level Of Service Module:

2Way95thQ:	0.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	10.5	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	B	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	80	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	1.3	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	72.4	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	F	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			72.4			xxxxxx		
ApproachLOS:	*			*			F			*		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #7 Brentwood Blvd. @ Gregory Lane

Average Delay (sec/veh): 3.2 Worst Case Level Of Service: F[108.4]

Table with columns for Street Name (Brentwood Blvd., Gregory Ln.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across various movements.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across various movements.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across various movements.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #8 Brentwood Blvd. @ Hanson Lane

Average Delay (sec/veh): 2.8 Worst Case Level Of Service: F[92.0]

Table with columns for Street Name (Brentwood Blvd., Hanson Ln), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 0, 0, 0, 0, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume across 12 lanes.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim across 12 lanes.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap across 12 lanes.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS across 12 lanes.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #9 Brentwood Blvd. @ Homecoming Way

Average Delay (sec/veh): 0.2 Worst Case Level Of Service: C[20.1]

Table with columns for Street Name (Brentwood Blvd., Homecoming Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across various movement categories.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim, showing values like 4.2, 2.3, and 6.2.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap., showing values like 1128, 591, 250, and 0.05.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS, showing values like 0.0, 11.2, B, C, and 20.1.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #10 Brentwood Blvd. @ Grant Street / Sunset Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.710
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 79 Level Of Service: C

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Grant Street / Sunset Rd. with sub-rows for North, South, East, and West bounds.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume. Rows list various adjustment factors and resulting volumes.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. Rows show saturation flow values and adjustments for different lane configurations.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves. Rows show capacity analysis results for different lane configurations.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #11 Brentwood Blvd. @ Sunset Ct.

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: C[20.8]

Table with columns for Street Name (Brentwood Blvd., Sunset Ct.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #12 Brentwood Blvd. @ Havenwood Ave.

Average Delay (sec/veh): 2.9 Worst Case Level Of Service: F[170.9]

Table with columns for Street Name (Brentwood Blvd., Havenwood Ave.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 1, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #13 Brentwood Blvd. @ Applewood Common

Cycle (sec): 100 Critical Vol./Cap.(X): 0.638
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 63 Level Of Service: B

Table with columns for Street Name (Brentwood Blvd., Applewood Common), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green (4, 8, 8), and Lanes (1, 0, 1, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across various movement categories.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across various movement categories.

Level of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #14 Brentwood Blvd. @ Sand Creek Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.399
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 31 Level Of Service: A

Street Name: Brentwood Blvd. Sand Creek Rd.

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, Crit Moves.

Level of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #15 Brentwood Blvd. @ Technology Way

Cycle (sec): 100 Critical Vol./Cap.(X): 0.218
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 24 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Technology Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green (4, 6, 6, 0, 6, 6, 4, 4, 4, 0, 0, 0), and Lanes (1, 0, 2, 0, 0, 0, 0, 1, 1, 0, 1, 0, 0, 0, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across 12 lanes.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across 12 lanes.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across 12 lanes.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #16 Brentwood Blvd @ Nancy Street (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.277
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 26 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Nancy Street (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and FinalVolume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #17 Brentwood Blvd. @ Village Drive

Average Delay (sec/veh): 1.4 Worst Case Level Of Service: E[36.3]

Table with columns for Street Name (Brentwood Blvd., Village Drive), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #18 Brentwood Blvd. @ Central Blvd / Sycamore Ave (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.462
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 35 Level Of Service: A

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Central Blvd / Sycamore Ave (S) with various movement details.

Volume Module: Table showing traffic volume adjustments including Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table showing Sat/Lane, Adjustment, Lanes, and Final Sat. values for different approaches.

Capacity Analysis Module: Table showing Vol/Sat, Crit Volume, and Crit Moves for different approaches.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #20 Central Street @ Second Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.222
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 34 Level Of Service: A

Table with columns for Street Name (Second Street, Central Boulevard), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include, Ignore), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #21 Central Blvd @ Walnut Blvd

Cycle (sec): 100 Critical Vol./Cap.(X): 0.202
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 36 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Central Blvd), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green (5, 7, 7), and Lanes (1 0 1 0 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #22 Brentwood Blvd @ Pine Street

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: B[13.1]

Table with columns for Street Name (Brentwood Blvd, Pine Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #23 Brentwood Blvd @ Maple Street

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: B[10.3]

Table with columns for Street Name (Brentwood Blvd, Maple Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume, with values for each approach and movement.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim, with values for each approach and movement.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap, with values for each approach and movement.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS, with values for each approach and movement.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #24 Brentwood Blvd @ Oak Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.438
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 41 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Oak St), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green (4, 10, 10), and Lanes (1, 0, 1, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across various movement categories.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across various movement categories.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #25 Walnut Blvd @ Oak St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.420
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 39 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Oak St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #26 Brentwood Blvd. @ Second St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.359
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 29 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Second St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Permitted), Rights (Include), Min. Green (4, 8, 8), and Lanes (1 0 1 1 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across various movement categories.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across various movement categories.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #27 Brentwood Blvd. @ Chestnut St.

Average Delay (sec/veh): 0.9 Worst Case Level Of Service: C[17.7]

Table with columns for Street Name (Brentwood Blvd., Chestnut St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 0, 0, 0, 0, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for Brentwood Blvd. and Chestnut St.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for Brentwood Blvd. and Chestnut St.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap., and values for Brentwood Blvd. and Chestnut St.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS, and values for Brentwood Blvd. and Chestnut St.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #28 Brentwood Blvd. @ Balfour Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.732
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 85 Level Of Service: C

Table with columns for Street Name (Brentwood Blvd., Balfour Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Ignore, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #29 Sellers Ave. @ Sunet Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.492
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 11.3
Optimal Cycle: 0 Level Of Service: B

Street Name: Sellers Ave. Sunset Rd.
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Stop Sign Stop Sign Stop Sign Stop Sign
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0

Volume Module:
Base Vol: 81 236 14 21 175 39 50 91 62 7 77 30
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 81 236 14 21 175 39 50 91 62 7 77 30
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 81 236 14 21 175 39 50 91 62 7 77 30
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 81 236 14 21 175 39 50 91 62 7 77 30
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 81 236 14 21 175 39 50 91 62 7 77 30

Saturation Flow Module:
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.24 0.72 0.04 0.09 0.74 0.17 0.25 0.45 0.30 0.06 0.68 0.26
Final Sat.: 165 479 28 59 491 109 152 276 188 36 395 154

Capacity Analysis Module:
Vol/Sat: 0.49 0.49 0.49 0.36 0.36 0.36 0.33 0.33 0.33 0.19 0.19 0.19
Crit Moves: **** **** ****
Delay/Veh: 12.6 12.6 12.6 10.8 10.8 10.8 10.7 10.7 10.7 9.7 9.7 9.7
Delay Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
AdjDel/Veh: 12.6 12.6 12.6 10.8 10.8 10.8 10.7 10.7 10.7 9.7 9.7 9.7
LOS by Move: B B B B B B B B B A A A
ApproachDel: 12.6 10.8 10.7
Delay Adj: 1.00 1.00 1.00
ApprAdjDel: 12.6 10.8 10.7 9.7
LOS by Appr: B B B A
AllWayAvgQ: 0.8 0.8 0.8 0.5 0.5 0.5 0.4 0.4 0.4 0.2 0.2 0.2

Note: Queue reported is the number of cars per lane.

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	E Sims Rd. and Delta Rd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1184 veh/h Directional split 54 / 46 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1300	
v_p * highest directional split proportion ² (pc/h)		702	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	40 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	40.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		1.9	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		28.0	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1287	
v_p * highest directional split proportion ² (pc/h)		695	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		67.7	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		9.3	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		77.0	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.41	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		97	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	355
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	3.5
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	E Sims Rd. and Delta Rd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The diagram labels: Shoulder width (ft), Lane width (ft), Lane width (ft), Shoulder width (ft), and Segment length, L_i (mi).</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1537 veh/h Directional split 55 / 45 Peak-hour factor, PHF 0.96 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1617	
v_p * highest directional split proportion ² (pc/h)		889	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	40 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	40.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		1.5	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		26.0	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1601	
v_p * highest directional split proportion ² (pc/h)		881	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		75.5	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		6.8	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		82.3	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.51	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		120	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	461
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	4.6
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

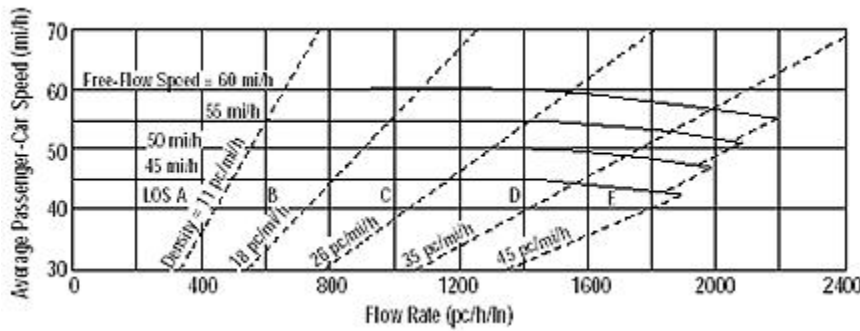
TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	Grant St. to Sunset Ct.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram labels the following dimensions: Shoulder width (ft), Lane width (ft), Lane width (ft), and Shoulder width (ft). Arrows indicate traffic flow in both directions.</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1462 veh/h Directional split 52 / 48 Peak-hour factor, PHF 0.97 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 10	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1522	
v_p * highest directional split proportion ² (pc/h)		791	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		1.6	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		16.6	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1507	
v_p * highest directional split proportion ² (pc/h)		784	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		73.4	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		7.3	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		80.7	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.48	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		38	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	146
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	2.3
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	Grant St. to Sunset Ct.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram labels the following dimensions: Shoulder width (ft), Lane width (ft), Lane width (ft), and Shoulder width (ft). Arrows indicate traffic flow in both directions.</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1822 veh/h Directional split 51 / 49 Peak-hour factor, PHF 0.90 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 10	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		2045	
v_p * highest directional split proportion ² (pc/h)		1043	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		1.1	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		13.0	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		2024	
v_p * highest directional split proportion ² (pc/h)		1032	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		83.1	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		4.3	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		87.4	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.64	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		51	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	182
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	3.9
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Brentwood Blvd.
Agency or Company		From/To	Central Blvd. to Spruce S
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	543	Peak-Hour Factor, PHF	0.90
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

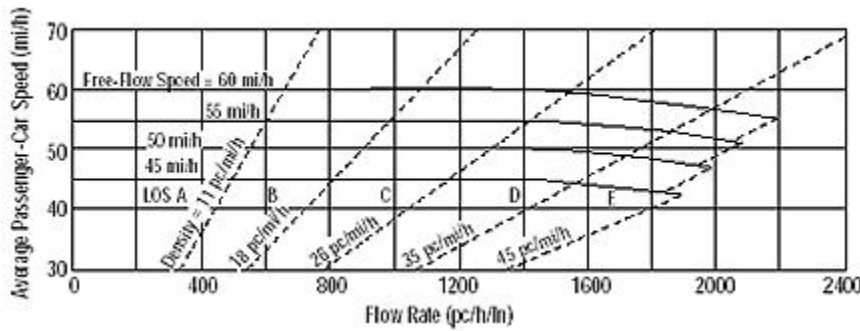
Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	316	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	7.0	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Central Blvd. to Spruce S
 Jurisdiction: City of Brentwood
 Analysis Year: Existing

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	597	Peak-Hour Factor, PHF	0.95
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

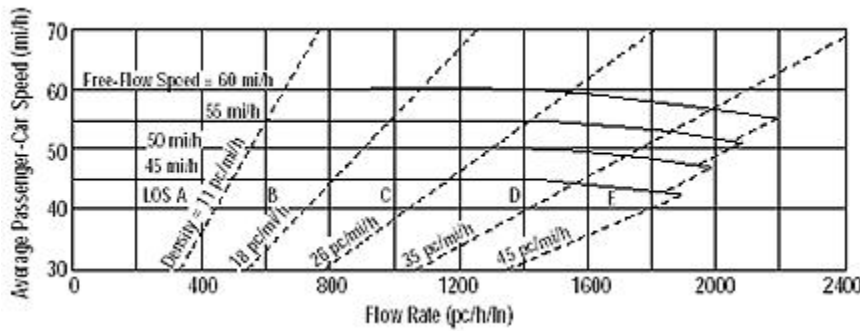
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 329
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 7.3
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Central Blvd. to Spruce S
 Jurisdiction: City of Brentwood
 Analysis Year: Existing

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	634	Peak-Hour Factor, PHF	0.78
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

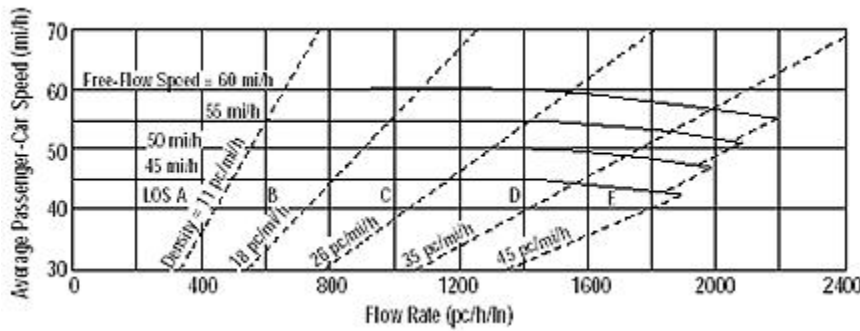
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 426
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 9.5
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v _p	L
Design (N)	FFS, LOS, v _p	N
Design (v _p)	FFS, LOS, N	v _i
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v _p)	FFS, LOS, N	v _i

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Brentwood Blvd.
Agency or Company		From/To	Central Blvd. to Spruce S
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	763	Peak-Hour Factor, PHF	0.99
AADT(veh/h)		%Trucks and Buses, P _T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

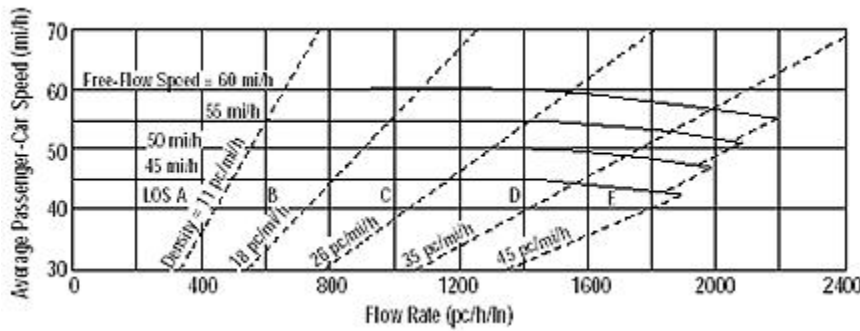
Calculate Flow Adjustments			
f _p	1.00	E _R	1.2
E _T	1.5	f _{HV}	0.952

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f _{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f _{LC} (mi/h)	
Access Points, A (A/mi)	0	f _A (mi/h)	
Median Type, M		f _M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v _p (pc/h/ln)	404	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v _p (pc/h)	
D (pc/mi/ln)	9.0	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Pine Ave. to Oak Ave.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	523	Peak-Hour Factor, PHF	0.90
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

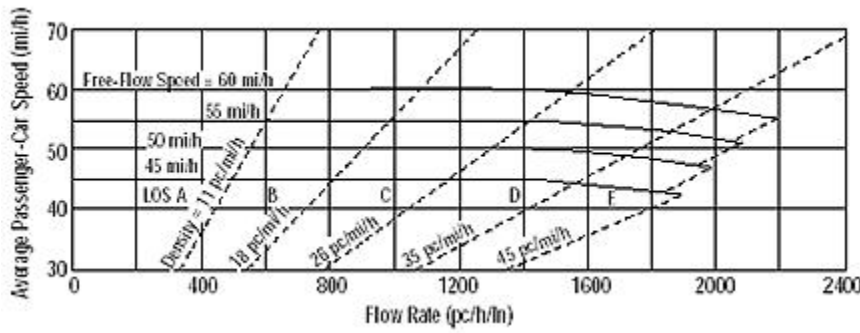
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 305
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 6.8
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Brentwood Blvd.
Agency or Company		From/To	Pine Ave. to Oak Ave.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	457	Peak-Hour Factor, PHF	0.88
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

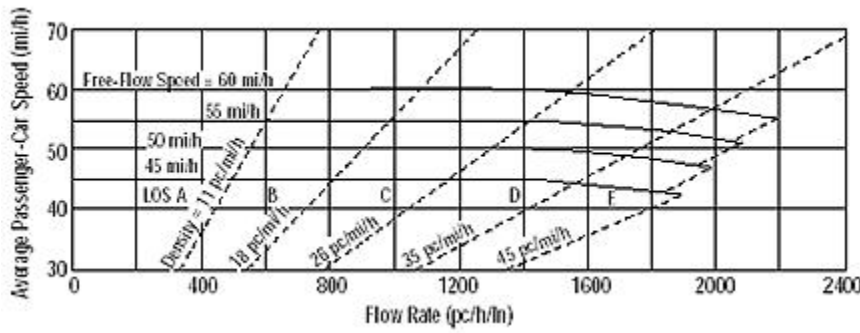
Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	272	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	6.0	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Brentwood Blvd.
Agency or Company		From/To	Pine Ave. to Oak Ave.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	614	Peak-Hour Factor, PHF	0.97
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

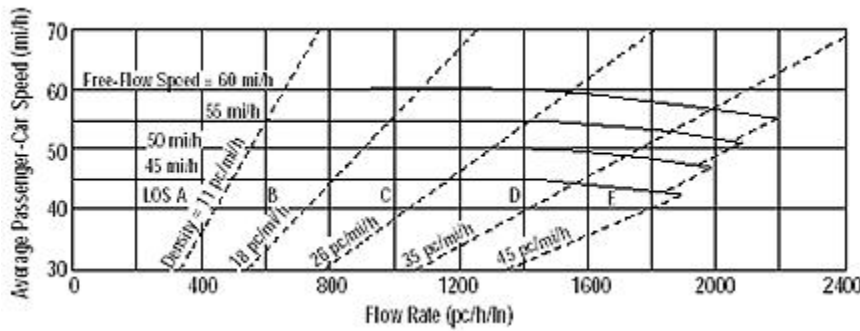
Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	332	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	7.4	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v _p	L
Design (N)	FFS, LOS, v _p	N
Design (v _p)	FFS, LOS, N	v _i
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v _p)	FFS, LOS, N	v _i

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Brentwood Blvd.
Agency or Company		From/To	Pine Ave. to Oak Ave.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	621	Peak-Hour Factor, PHF	0.97
AADT(veh/h)		%Trucks and Buses, P _T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments			
f _p	1.00	E _R	1.2
E _T	1.5	f _{HV}	0.952

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f _{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f _{LC} (mi/h)	
Access Points, A (A/mi)	0	f _A (mi/h)	
Median Type, M		f _M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v _p (pc/h/ln)	336	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v _p (pc/h)	
D (pc/mi/ln)	7.5	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Delta Rd.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 362 veh/h Directional split 58 / 42 Peak-hour factor, PHF 0.76 No-passing zone 0 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 40	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		483	
v_p * highest directional split proportion ² (pc/h)		280	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	50 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	50.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		0.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		46.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		477	
v_p * highest directional split proportion ² (pc/h)		277	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		34.2	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		0.2	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		34.5	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		A	
Volume to capacity ratio, $v/c=V_p/3,200$		0.15	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		119	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	362
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	2.6
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Delta Rd.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 318 veh/h Directional split 51 / 49 Peak-hour factor, PHF 0.91 No-passing zone 0 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 40	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		354	
v_p * highest directional split proportion ² (pc/h)		181	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	50 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	50.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		0.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		47.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		350	
v_p * highest directional split proportion ² (pc/h)		179	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		26.5	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		0.1	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		26.6	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		A	
Volume to capacity ratio, $v/c=V_p/3,200$		0.11	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		87	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	318
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	1.8
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Lone Tree Wy.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram indicates shoulder widths and lane widths for both directions of travel.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 468 veh/h Directional split 64 / 36 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 30	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		516	
v_p * highest directional split proportion ² (pc/h)		330	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	35 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	35.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		4.2	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		26.8	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		510	
v_p * highest directional split proportion ² (pc/h)		326	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		36.1	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		21.6	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		57.7	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.16	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		127	

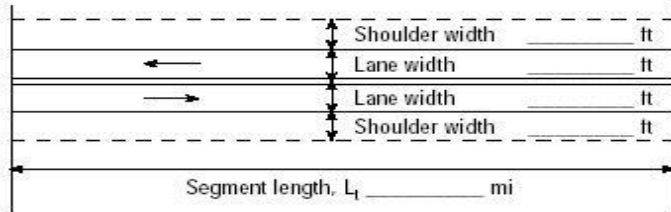
Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	468
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	4.7
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Lone Tree Wy.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The diagram labels: Shoulder width (ft), Lane width (ft), Lane width (ft), Shoulder width (ft), and Segment length, L_i (mi).</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 501 veh/h Directional split 53 / 47 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 30	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		552	
v_p * highest directional split proportion ² (pc/h)		293	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	35 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	35.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		4.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		26.7	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		546	
v_p * highest directional split proportion ² (pc/h)		289	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		38.1	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		21.5	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		59.6	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.17	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		136	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	501
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	5.1
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

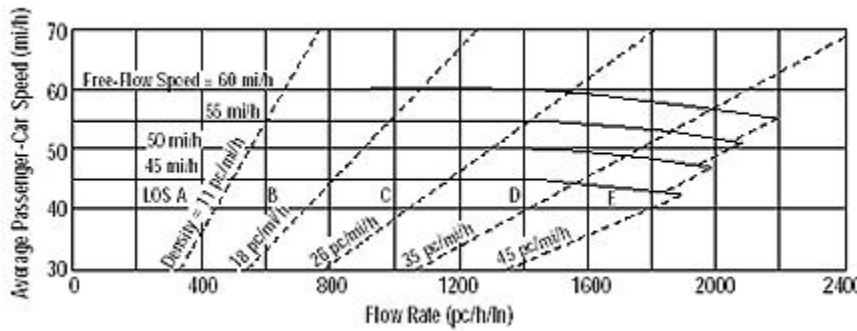
TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sunset Ave
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The diagram labels: Shoulder width (ft), Lane width (ft), Lane width (ft), Shoulder width (ft), and Segment length, L_i (mi).</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 522 veh/h Directional split 63 / 37 Peak-hour factor, PHF 0.88 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		596	
v_p * highest directional split proportion ² (pc/h)		375	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.9	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		21.5	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		594	
v_p * highest directional split proportion ² (pc/h)		374	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		40.7	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		20.8	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		61.5	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.19	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		148	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	522
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	6.9
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sunset Ave
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing
Project Description:			
Input Data			
		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 534 veh/h Directional split 57 / 43 Peak-hour factor, PHF 0.88 No-passing zone 100 % Trucks and Buses, P _T 2% % Recreational vehicles, P _R 4% Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f _G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))		0.996	
Two-way flow rate ¹ , v _p (pc/h)=V/(PHF * f _G * f _{HV})		609	
v _p * highest directional split proportion ² (pc/h)		347	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S _{FM}	30 mi/h	Base free-flow speed, BFFS _{FM}	mi/h
Observed volume, V _f	0 veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS FFS=S _{FM} +0.00776(V _f /f _{HV})	mi/h	Adj. for access points, f _A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A)	30.0 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)		3.9	
Average travel speed, ATS (mi/h) ATS=FFS-0.00776v _p -f _{np}		21.4	
Percent Time-Spent-Following			
Grade Adjustment factor, f _G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))		0.998	
Two-way flow rate ¹ , v _p (pc/h)=V/(PHF * f _G * f _{HV})		608	
v _p * highest directional split proportion ² (pc/h)		347	
Base percent time-spent-following, BPTSF(%)=100(1-e ^{-0.000879v_p})		41.4	
Adj. for directional distribution and no-passing zone, f _{d/np} (%)(Exh. 20-12)		20.4	
Percent time-spent-following, PTSF(%)=BPTSF+f _{d/np}		61.8	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, v/c=V _p /3,200		0.19	
Peak 15-min veh-miles of travel, VMT ₁₅ (veh-mi)=0.25L _i (V/PHF)		152	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	534
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	7.1
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Central Blvd.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	120	Peak-Hour Factor, PHF	0.86
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

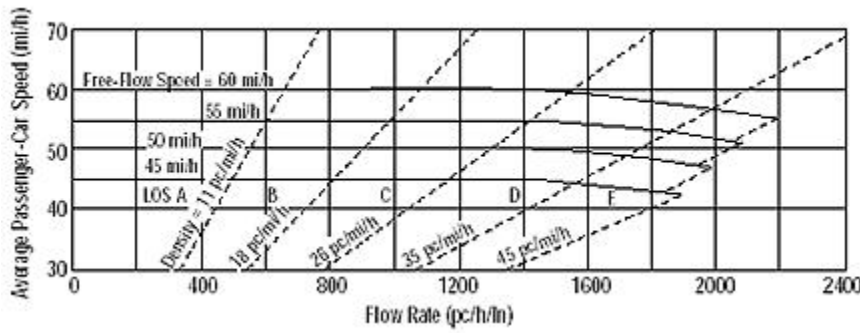
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 70
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 1.6
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v _p	L
Design (N)	FFS, LOS, v _p	N
Design (v _p)	FFS, LOS, N	v _i
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v _p)	FFS, LOS, N	v _i

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Central Blvd.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	279	Peak-Hour Factor, PHF	0.88
AADT(veh/h)		%Trucks and Buses, P _T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f _p	1.00	E _R	1.2
E _T	1.5	f _{HV}	0.990

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

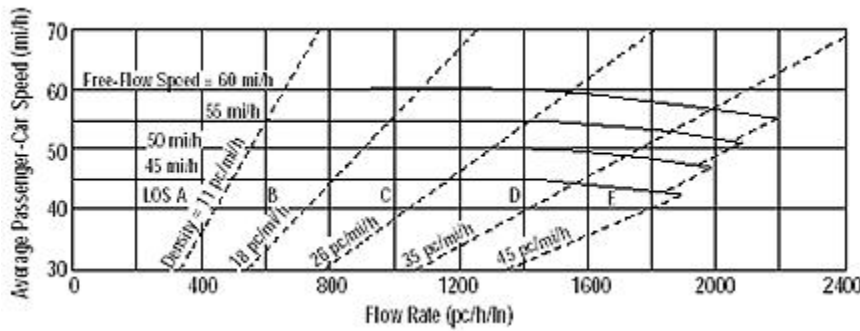
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 160
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 3.6
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Central Blvd.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	215	Peak-Hour Factor, PHF	0.84
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

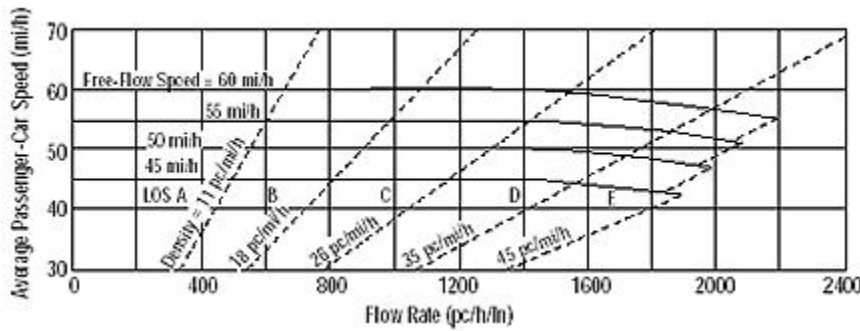
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 129
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 2.9
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Central Blvd.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	290	Peak-Hour Factor, PHF	0.76
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 192
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 4.3
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Grant St.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 301 veh/h Directional split 55 / 45 Peak-hour factor, PHF 0.71 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 7	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		430	
v_p * highest directional split proportion ² (pc/h)		237	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		4.4	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		22.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		425	
v_p * highest directional split proportion ² (pc/h)		234	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		31.2	
Adj. for directional distribution and no-passing zone, $f_{dnp}(\%)(Exh. 20-12)$		23.1	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{dnp}$		54.3	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		B	
Volume to capacity ratio, $v/c=V_p/3,200$		0.13	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		106	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	301
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	4.8
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Grant St.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On each side of the lanes are shoulders. The diagram labels: Shoulder width (ft), Lane width (ft), Lane width (ft), Shoulder width (ft), and Segment length, L_i (mi).</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 244 veh/h Directional split 55 / 45 Peak-hour factor, PHF 0.87 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 7	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		284	
v_p * highest directional split proportion ² (pc/h)		156	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.9	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		23.9	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		281	
v_p * highest directional split proportion ² (pc/h)		155	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		21.9	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		23.1	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		44.9	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		B	
Volume to capacity ratio, $v/c=V_p/3,200$		0.09	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		70	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	244
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	2.9
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sycamore Ave.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 358 veh/h Directional split 60 / 40 Peak-hour factor, PHF 0.87 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 35	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		417	
v_p * highest directional split proportion ² (pc/h)		250	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		4.4	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		22.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		412	
v_p * highest directional split proportion ² (pc/h)		247	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		30.4	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		22.1	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		52.5	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		B	
Volume to capacity ratio, $v/c=V_p/3,200$		0.13	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		51	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	179
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	2.3
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sycamore Ave.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i mi. The diagram includes two lanes, each with a width of _____ ft, and two shoulders, each with a width of _____ ft. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 339 veh/h Directional split 53 / 47 Peak-hour factor, PHF 0.82 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 35	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		419	
v_p * highest directional split proportion ² (pc/h)		222	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		4.4	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		22.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		414	
v_p * highest directional split proportion ² (pc/h)		219	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		30.5	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		23.8	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		54.3	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		B	
Volume to capacity ratio, $v/c=V_p/3,200$		0.13	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		52	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	170
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	2.3
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

Appendix B:

*Analysis Worksheets for
Existing plus Approved Projects Conditions*

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 Brentwood Blvd. @ Delta Road

Average Delay (sec/veh): 10.6 Worst Case Level Of Service: F[105.2]

Street Name:	Brentwood Blvd.					Delta Rd.						
Approach:	North Bound		South Bound			East Bound			West Bound			
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled		Uncontrolled			Stop Sign			Stop Sign			
Rights:	Include		Include			Include			Include			
Lanes:	0	0	0	1	0	1	0	0	1	0	0	0

Volume Module:

Base Vol:	0	596	39	68	499	0	0	0	0	63	0	89
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	596	39	68	499	0	0	0	0	63	0	89
Added Vol:	0	146	0	0	65	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	742	39	68	564	0	0	0	0	63	0	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.90	0.90	0.90	1.00	1.00	1.00	0.91	0.91	0.91
PHF Volume:	0	823	43	76	629	0	0	0	0	70	0	98
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	823	43	76	629	0	0	0	0	70	0	98

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.2	xxxx	xxxxxx	7.1	6.5	6.2	6.4	6.5	6.2
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	3.5	4.0	3.3	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	866	xxxx	xxxxxx	1674	1646	629	1625	1625	844
Potent Cap.:	xxxx	xxxx	xxxxxx	745	xxxx	xxxxxx	76	99	482	113	102	363
Move Cap.:	xxxx	xxxx	xxxxxx	745	xxxx	xxxxxx	51	89	482	104	92	363
Volume/Cap:	xxxx	xxxx	xxxx	0.10	xxxx	xxxx	0.00	0.00	0.00	0.67	0.00	0.27

Level of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.3	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	xxxxxx	xxxx	xxxxxx	10.4	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	*	*	*	B	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	0	xxxxxx	xxxx	178	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	7.3	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	105	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	F	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			105.2		
ApproachLOS:	*			*			*			F		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 2000 HCM 4-Way Stop Method (Future Volume Alternative)

 Intersection #2 Sellers Ave @ Delta

Cycle (sec): 100 Critical Vol./Cap.(X): 0.410
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 11.6
 Optimal Cycle: 0 Level Of Service: B

Street Name: Sellers Ave. Delta Rd.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0

Volume Module:

Base Vol:	44	125	69	13	202	37	18	107	85	110	123	12
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	44	125	69	13	202	37	18	107	85	110	123	12
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	44	125	69	13	202	37	18	107	85	110	123	12
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	44	125	69	13	202	37	18	107	85	110	123	12
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	44	125	69	13	202	37	18	107	85	110	123	12
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	44	125	69	13	202	37	18	107	85	110	123	12

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.18	0.53	0.29	0.05	0.80	0.15	0.09	0.51	0.40	0.45	0.50	0.05
Final Sat.:	114	324	179	32	493	90	52	311	247	268	300	29

Capacity Analysis Module:

Vol/Sat:	0.39	0.39	0.39	0.41	0.41	0.41	0.34	0.34	0.34	0.41	0.41	0.41
Crit Moves:	****			****			****			****		
Delay/Veh:	11.4	11.4	11.4	11.8	11.8	11.8	10.9	10.9	10.9	12.1	12.1	12.1
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	11.4	11.4	11.4	11.8	11.8	11.8	10.9	10.9	10.9	12.1	12.1	12.1
LOS by Move:	B	B	B	B	B	B	B	B	B	B	B	B
ApproachDel:	11.4			11.8			10.9			12.1		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	11.4			11.8			10.9			12.1		
LOS by Appr:	B			B			B			B		
AllWayAvgQ:	0.5	0.5	0.5	0.6	0.6	0.6	0.4	0.4	0.4	0.6	0.6	0.6

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #3 Brentwood Blvd. @ E. Sims Road

Average Delay (sec/veh): 4.2 Worst Case Level Of Service: F[60.3]

Table with columns for Street Name, Approach, Movement, Control, Rights, and Lanes. Rows include Brentwood Blvd. and E. Sims Road with various movement and lane configurations.

Volume Module: Table showing traffic volume metrics such as Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across different approaches.

Critical Gap Module: Table showing critical gap and follow-up time values for different approaches.

Capacity Module: Table showing capacity-related metrics like Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap. for various approaches.

Level of Service Module: Table showing level of service (LOS) and delay values for different movements and approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #4 Brentwood Blvd @ Lone Tree Way (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.530
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 49 Level Of Service: A

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd and Lone Tree Way (S) with various movement and control details.

Volume Module: Table showing traffic volume data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table showing saturation flow data including Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table showing capacity analysis data including Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #5 Arroyo Seco Road @ Lone Tree Way

Average Delay (sec/veh): 4.0 Worst Case Level Of Service: A[8.9]

Table with columns for Street Name, Approach, Movement, Control, Rights, and Lanes. Rows include Arroyo Seco Road and Lone Tree Way with various movement and control details.

Volume Module: Table showing traffic volume calculations including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table showing Critical Gap and FollowUpTim values for different movements.

Capacity Module: Table showing Capacity calculations including Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table showing Level of Service calculations including 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #6 Brentwood Blvd. @ Sunrise Drive

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: E[39.8]

Table with columns for Street Name, Approach, Movement, Control, Rights, and Lanes. Rows include Brentwood Blvd. and Sunrise Dr. with various movement types (L, T, R) and lane configurations.

Volume Module: Table showing traffic volume data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across different movements.

Critical Gap Module: Table showing Critical Gap and FollowUpTim values for different movements.

Capacity Module: Table showing Capacity-related metrics like Cnflict Vol, Potent Cap., Move Cap., and Volume/Cap. for various movements.

Level Of Service Module: Table showing Level of Service (LOS) and ApproachDel values for different movements and approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #7 Brentwood Blvd. @ Gregory Lane

Average Delay (sec/veh): 0.6 Worst Case Level Of Service: D[34.4]

Table with columns for Street Name (Brentwood Blvd., Gregory Ln.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1 0 0 1 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #8 Brentwood Blvd. @ Hanson Lane

Average Delay (sec/veh): 2.5 Worst Case Level Of Service: F[88.0]

Table with columns for Street Name (Brentwood Blvd., Hanson Ln), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 0, 0, 0, 0, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across 12 lanes.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across 12 lanes.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap. across 12 lanes.

Level of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across 12 lanes.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #9 Brentwood Blvd. @ Homecoming Way

Average Delay (sec/veh): 2.4 Worst Case Level Of Service: F[85.1]

Table with columns for Street Name (Brentwood Blvd., Homecoming Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table showing traffic volume data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across different movements.

Critical Gap Module: Table showing Critical Gap and FollowUpTim values for different movements.

Capacity Module: Table showing Capacity-related metrics such as Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level of Service Module: Table showing Level of Service (LOS) by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #10 Brentwood Blvd. @ Grant Street / Sunset Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.784
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 105 Level Of Service: C

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Grant Street / Sunset Rd. with sub-rows for North, South, East, and West bounds.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, etc.) and rows for different street directions.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat., and rows for different street directions.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves, and rows for different street directions.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #11 Brentwood Blvd. @ Sunset Ct.

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: C[19.0]

Table with columns for Street Name (Brentwood Blvd., Sunset Ct.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across various movements.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across various movements.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across various movements.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across various movements.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #12 Brentwood Blvd. @ Havenwood Ave.

Average Delay (sec/veh): 2.0 Worst Case Level Of Service: F[89.4]

Table with columns for Street Name (Brentwood Blvd., Havenwood Ave.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1 0 1 0 1, 1 0 0 1 0, 0 0 0 0 1, 0 0 1! 0 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across four approaches.

Critical Gap Module: Table with columns for Critical Gap and FollowUpTime across four approaches.

Capacity Module: Table with columns for Conflict Vol, Potent Cap., Move Cap., and Volume/Cap. across four approaches.

Level of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

 Intersection #13 Brentwood Blvd. @ Applewood Common

Cycle (sec): 100 Critical Vol./Cap.(X): 0.620
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 60 Level Of Service: B

Street Name:	Brentwood Blvd.						Applewood Common					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	4	8	8	4	8	8	4	4	4	4	4	4
Lanes:	1	0	1	0	1	0	1	0	0	1	0	0

Volume Module:

Base Vol:	13	723	38	18	623	7	16	2	18	37	0	23
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	13	723	38	18	623	7	16	2	18	37	0	23
Added Vol:	4	202	4	0	205	3	9	0	10	13	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	17	925	42	18	828	10	25	2	28	50	0	23
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	17	925	42	18	828	10	25	2	28	50	0	23
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	17	925	42	18	828	10	25	2	28	50	0	23
RTOR Reduct:	0	0	42	0	0	0	0	0	0	0	0	0
RTOR Vol:	17	925	0	18	828	10	25	2	28	50	0	23
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	17	925	0	18	828	10	25	2	28	50	0	23

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	1.00	1.00	0.99	0.01	1.00	0.07	0.93	1.00	0.00	1.00
Final Sat.:	1650	1650	1650	1650	1630	20	1650	110	1540	1650	0	1650

Capacity Analysis Module:

Vol/Sat:	0.01	0.56	0.00	0.01	0.51	0.51	0.02	0.02	0.02	0.03	0.00	0.01
Crit Volume:		925		18				30		50		
Crit Moves:		****		****				****		****		

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #14 Brentwood Blvd. @ Sand Creek Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.481
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 36 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Sand Creek Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #15 Brentwood Blvd. @ Technology Way

Cycle (sec): 100 Critical Vol./Cap.(X): 0.256
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 25 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Technology Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #16 Brentwood Blvd @ Nancy Street (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.328
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 28 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Nancy Street (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #17 Brentwood Blvd. @ Village Drive

Average Delay (sec/veh): 1.3 Worst Case Level Of Service: E[39.7]

Table with columns for Street Name (Brentwood Blvd., Village Drive), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table showing traffic volume components like Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across different movements.

Critical Gap Module: Table showing Critical Gap and FollowUpTim values for different movements.

Capacity Module: Table showing Capacity-related metrics like Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap. for different movements.

Level Of Service Module: Table showing Level of Service (LOS) and other metrics like 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #18 Brentwood Blvd. @ Central Blvd / Sycamore Ave (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.537
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 40 Level Of Service: A

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Central Blvd / Sycamore Ave (S) with various movement details.

Volume Module: Table showing traffic volume metrics such as Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table showing saturation flow metrics like Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table showing capacity analysis metrics such as Vol/Sat, Crit Volume, and Crit Moves.

Level of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

 Intersection #20 Central Street @ Second Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.331
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 34 Level Of Service: A

Street Name:	Second Street						Central Boulevard								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected			Protected			Protected					
Rights:	Include			Include			Include			Ignore					
Min. Green:	5	5	5	5	5	5	5	7	7	5	7	7			
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0	2	0	1

Volume Module:	Second Street NB			Second Street SB			Central Blvd EB			Central Blvd WB		
Base Vol:	57	195	36	54	388	4	6	138	133	51	179	49
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	57	195	36	54	388	4	6	138	133	51	179	49
Added Vol:	23	0	0	0	0	0	0	119	49	0	72	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	80	195	36	54	388	4	6	257	182	51	251	49
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	80	195	36	54	388	4	6	257	182	51	251	49
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	80	195	36	54	388	4	6	257	182	51	251	49
RTOR Reduct:	0	0	0	0	0	0	0	0	0	0	0	0
RTOR Vol:	80	195	36	54	388	4	6	257	182	51	251	49
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	80	195	36	54	388	4	6	257	182	51	251	49

Saturation Flow Module:	Second Street NB			Second Street SB			Central Blvd EB			Central Blvd WB		
Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.69	0.31	1.00	1.98	0.02	1.00	1.17	0.83	1.00	2.00	1.00
Final Sat.:	1650	2786	514	1650	3266	34	1650	1932	1368	1650	3300	1650

Capacity Analysis Module:	Second Street NB			Second Street SB			Central Blvd EB			Central Blvd WB		
Vol/Sat:	0.05	0.07	0.07	0.03	0.12	0.12	0.00	0.13	0.13	0.03	0.08	0.03
Crit Volume:	80			196			220			51		
Crit Moves:	****			****			****			****		

Level of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #21 Central Blvd @ Walnut Blvd

Cycle (sec): 100 Critical Vol./Cap.(X): 0.243
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 36 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Central Blvd), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #22 Brentwood Blvd @ Pine Street

Average Delay (sec/veh): 0.9 Worst Case Level Of Service: B[13.5]

Street Name:	Brentwood Blvd			Pine Street										
Approach:	North Bound		South Bound		East Bound		West Bound							
Movement:	L	T	R	L	T	R	L	T	R					
Control:	Uncontrolled			Uncontrolled			Stop Sign		Stop Sign					
Rights:	Include			Include			Include		Include					
Lanes:	0	0	1	1	0	0	0	0	0	0	0	1	0	0

Volume Module:

Base Vol:	0	427	96	21	475	0	0	0	0	14	0	59
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	427	96	21	475	0	0	0	0	14	0	59
Added Vol:	0	120	6	0	181	0	0	0	0	1	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	547	102	21	656	0	0	0	0	15	0	59
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	547	102	21	656	0	0	0	0	15	0	59
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	547	102	21	656	0	0	0	0	15	0	59

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.8	6.5	6.9
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	649	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	968	1296	325
Potent Cap.:	xxxx	xxxx	xxxxxx	881	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	251	161	671
Move Cap.:	xxxx	xxxx	xxxxxx	881	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	247	157	671
Volume/Cap:	xxxx	xxxx	xxxx	0.02	xxxx	xxxx	xxxx	xxxx	xxxx	0.06	0.00	0.09

Level of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.1	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	xxxxxx	xxxx	xxxxxx	9.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	*	*	*	A	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	498	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	0.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	0.5	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	9.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	13.5	xxxxxx
Shared LOS:	*	*	*	A	*	*	*	*	*	*	B	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx				13.5	
ApproachLOS:	*			*			*				B	

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #23 Brentwood Blvd @ Maple Street

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: B[10.6]

Street Name:	Brentwood Blvd					Maple Street														
Approach:	North Bound		South Bound			East Bound			West Bound											
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Uncontrolled					Uncontrolled			Stop Sign			Stop Sign								
Rights:	Include					Include			Include			Include								
Lanes:	0	1	0	1	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	1

Volume Module:

Base Vol:	0	511	31	18	477	0	0	0	0	0	0	0	13
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	511	31	18	477	0	0	0	0	0	0	0	13
Added Vol:	0	117	5	16	166	0	0	0	0	0	0	0	8
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	628	36	34	643	0	0	0	0	0	0	0	21
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	628	36	34	643	0	0	0	0	0	0	0	21
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	628	36	34	643	0	0	0	0	0	0	0	21

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	6.9
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	664	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	332
Potent Cap.:	xxxx	xxxx	xxxxxx	869	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	664
Move Cap.:	xxxx	xxxx	xxxxxx	869	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	664
Volume/Cap:	xxxx	xxxx	xxxx	0.04	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.03

Level of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.1	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	0.1
Control Del:	xxxxxx	xxxx	xxxxxx	9.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	10.6
LOS by Move:	*	*	*	A	*	*	*	*	*	*	*	B
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	0.0	xxxx	xxxxxx	0.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	9.0	xxxx	xxxxxx	9.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	A	*	*	A	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx					10.6
ApproachLOS:	*			*			*					B

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #24 Brentwood Blvd @ Oak Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.460
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 42 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Oak St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and rows for each.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat., and rows for each.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves, and rows for each.

Level of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #25 Walnut Blvd @ Oak St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.394
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 38 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Oak St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with columns for various volume and adjustment factors like Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

 Intersection #26 Brentwood Blvd. @ Second St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.429
 Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 33 Level Of Service: A

Street Name:	Brentwood Blvd.						Second St.								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected			Permitted			Permitted					
Rights:	Include			Include			Include			Include					
Min. Green:	4	8	8	4	8	8	4	4	4	4	4	4			
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0	2	0	1

Volume Module:

Base Vol:	100	387	21	183	393	21	111	383	64	35	176	129
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	100	387	21	183	393	21	111	383	64	35	176	129
Added Vol:	11	109	0	15	155	1	3	20	26	0	12	19
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	111	496	21	198	548	22	114	403	90	35	188	148
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	111	496	21	198	548	22	114	403	90	35	188	148
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	111	496	21	198	548	22	114	403	90	35	188	148
RTOR Reduct:	0	0	0	0	0	0	0	0	0	0	0	148
RTOR Vol:	111	496	21	198	548	22	114	403	90	35	188	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	111	496	21	198	548	22	114	403	90	35	188	0

Saturation Flow Module:

Sat/Lane:	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.92	0.08	1.00	1.92	0.08	1.00	1.63	0.37	1.00	2.00	1.00
Final Sat.:	1720	3300	140	1720	3307	133	1720	2812	628	1720	3440	1720

Capacity Analysis Module:

Vol/Sat:	0.06	0.15	0.15	0.12	0.17	0.17	0.07	0.14	0.14	0.02	0.05	0.00
Crit Volume:	259		198		247		35					
Crit Moves:	****		****		****		****					

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #27 Brentwood Blvd. @ Chestnut St.

Average Delay (sec/veh): 1.6 Worst Case Level Of Service: D[25.5]

Table with columns for Street Name, Approach, Movement, Control, Rights, and Lanes. Rows include Brentwood Blvd. and Chestnut St. with various movement types like L-T-R and control types like Uncontrolled and Stop Sign.

Volume Module: Table showing traffic volume data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across different approaches.

Critical Gap Module: Table showing critical gap and follow-up time data for different approaches, with values like 4.2, 6.4, 6.5, 2.3, 3.5, 4.0, 3.3.

Capacity Module: Table showing capacity-related data including Conflict Vol, Potent Cap., Move Cap., and Volume/Cap. for different approaches.

Level of Service Module: Table showing level of service data including 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #28 Brentwood Blvd. @ Balfour Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.605
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 58 Level Of Service: B

Table with columns for Street Name (Brentwood Blvd., Balfour Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights (Ignore, Include), Min. Green (8, 10, 10, 6, 10, 10, 5, 5, 5, 4, 5, 5), and Lanes (1 0 2 0 1, 1 0 2 0 1, 1 0 1 0 1, 1 0 1 1 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across 12 lanes.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across 12 lanes.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across 12 lanes.

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #29 Sellers Ave. @ Sunet Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.557
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 12.0
Optimal Cycle: 0 Level Of Service: B

Street Name: Sellers Ave. Sunset Rd.

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module: Table with columns for Adjustment, Lanes, Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Moves, Delay/Veh, Delay Adj, AdjDel/Veh, LOS by Move, ApproachDel, Delay Adj, ApprAdjDel, LOS by Appr, AllWayAvgQ.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 Brentwood Blvd. @ Delta Road

Average Delay (sec/veh): 39.4 Worst Case Level Of Service: F[443.2]

Table with columns for Street Name (Brentwood Blvd., Delta Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume for each approach and movement.

Critical Gap Module: Table showing Critical Gap and FollowUpTime for each approach and movement.

Capacity Module: Table showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap. for each approach and movement.

Level of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS for each approach and movement.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #2 Sellers Ave @ Delta

Cycle (sec): 100 Critical Vol./Cap.(X): 0.382
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 9.9
Optimal Cycle: 0 Level Of Service: A

Street Name: Sellers Ave. Delta Rd.
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Stop Sign Stop Sign Stop Sign Stop Sign
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0

Volume Module:
Base Vol: 67 173 31 11 152 16 28 84 45 32 72 23
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 67 173 31 11 152 16 28 84 45 32 72 23
Added Vol: 0 0 0 0 0 0 0 0 0 0 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 67 173 31 11 152 16 28 84 45 32 72 23
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 67 173 31 11 152 16 28 84 45 32 72 23
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 67 173 31 11 152 16 28 84 45 32 72 23
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 67 173 31 11 152 16 28 84 45 32 72 23

Saturation Flow Module:
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.25 0.64 0.11 0.06 0.85 0.09 0.18 0.53 0.29 0.25 0.57 0.18
Final Sat.: 175 453 81 42 582 61 118 353 189 162 364 116

Capacity Analysis Module:
Vol/Sat: 0.38 0.38 0.38 0.26 0.26 0.26 0.24 0.24 0.24 0.20 0.20 0.20
Crit Moves: ****
Delay/Veh: 10.6 10.6 10.6 9.6 9.6 9.6 9.5 9.5 9.5 9.3 9.3 9.3
Delay Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
AdjDel/Veh: 10.6 10.6 10.6 9.6 9.6 9.6 9.5 9.5 9.5 9.3 9.3 9.3
LOS by Move: B B B A A A A A A A A A
ApproachDel: 10.6 9.6 9.5 9.3
Delay Adj: 1.00 1.00 1.00
ApprAdjDel: 10.6 9.6 9.5 9.3
LOS by Appr: B A A
AllWayAvgQ: 0.5 0.5 0.5 0.3 0.3 0.3 0.3 0.3 0.3 0.2 0.2 0.2

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #3 Brentwood Blvd. @ E. Sims Road

Average Delay (sec/veh): 11.1 Worst Case Level Of Service: F[293.1]

Street Name: Brentwood Blvd. E. Sims Road

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	0	0	1	0	1	0	0	0	0	0	0	1

Volume Module:

Base Vol:	0	837	23	31	664	0	0	0	0	44	0	21
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	837	23	31	664	0	0	0	0	44	0	21
Added Vol:	0	106	14	0	199	0	0	0	0	9	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	943	37	31	863	0	0	0	0	53	0	21
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.88	0.88	0.88	0.88	0.88	0.88	1.00	1.00	1.00	0.90	0.90	0.90
PHF Volume:	0	1070	42	35	978	0	0	0	0	59	0	23
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	1070	42	35	978	0	0	0	0	59	0	23

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	6.5	6.2
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	1112	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	2140	2140	1091
Potent Cap.:	xxxx	xxxx	xxxxxx	599	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	54	49	261
Move Cap.:	xxxx	xxxx	xxxxxx	599	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	51	46	261
Volume/Cap:	xxxx	xxxx	xxxx	0.06	xxxx	xxxx	xxxx	xxxx	xxxx	1.14	0.00	0.09

Level of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.2	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	xxxxxx	xxxx	xxxxxx	11.4	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	*	*	*	B	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	66	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	0.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	6.6	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	11.4	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	293	xxxxxx
Shared LOS:	*	*	*	B	*	*	*	*	*	*	F	*
ApproachDel:	xxxxxx		xxxxxx		xxxxxx		xxxxxx		293.1			
ApproachLOS:	*		*		*		*		F			

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #4 Brentwood Blvd @ Lone Tree Way (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.535
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 49 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Lone Tree Way (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include, Ovl, Include), Min. Green (4, 8, 8, 4, 8, 8, 4, 4, 4, 4, 4, 4), and Lanes (1, 0, 1, 0, 1, 1, 0, 1, 1, 0, 1, 0, 1, 0, 0, 1, 0, 1, 0, 0, 1).

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and rows for different approaches (North, South, East, West).

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across different approaches.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across different approaches.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #5 Arroyo Seco Road @ Lone Tree Way

Average Delay (sec/veh): 2.3 Worst Case Level Of Service: A[8.9]

Street Name:	Arroyo Seco Road						Lone Tree Way									
Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled						
Rights:	Include			Include			Include			Include						
Lanes:	0	0	1!0	0	0	0	0	0	0	1	0	0	0	1	0	0

Volume Module:

Base Vol:	29	0	1	0	0	0	0	3	47	0	2	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	29	0	1	0	0	0	0	3	47	0	2	0
Added Vol:	0	0	0	0	0	0	0	22	0	0	14	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	29	0	1	0	0	0	0	25	47	0	16	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	29	0	1	0	0	0	0	25	47	0	16	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	29	0	1	0	0	0	0	25	47	0	16	0

Critical Gap Module:

Critical Gp:	6.4	6.5	6.2	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	3.5	4.0	3.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	65	65	49	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	941	826	1020	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	941	826	1020	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	0.03	0.00	0.00	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	944	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	0.1	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	8.9	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	A	*	*	*	*	*	*	*	*	*	*
ApproachDel:	8.9			xxxxxxx			xxxxxxx			xxxxxxx		
ApproachLOS:	A			*			*			*		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #6 Brentwood Blvd. @ Sunrise Drive

Average Delay (sec/veh): 8.7 Worst Case Level Of Service: F[457.4]

Table with columns for Street Name (Brentwood Blvd., Sunrise Dr.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #7 Brentwood Blvd. @ Gregory Lane

Average Delay (sec/veh): 17.8 Worst Case Level Of Service: F[703.9]

Table with columns for Street Name (Brentwood Blvd., Gregory Ln.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #8 Brentwood Blvd. @ Hanson Lane

Average Delay (sec/veh): 18.4 Worst Case Level Of Service: F[711.1]

Table with columns for Street Name (Brentwood Blvd., Hanson Ln), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 0, 0, 0, 0, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across 12 lanes.

Critical Gap Module: Table with columns for Critical Gap (4.2, 6.4, 6.5, 6.2) and FollowUpTime (2.3, 3.5, 4.0, 3.3).

Capacity Module: Table with columns for Conflict Vol, Potent Cap., Move Cap., and Volume/Cap. across 12 lanes.

Level of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #9 Brentwood Blvd. @ Homecoming Way

Average Delay (sec/veh): 3.6 Worst Case Level Of Service: F[326.3]

Street Name:	Brentwood Blvd.						Homecoming Way					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	0	0	0	1	0	1	0	0	0	0	0	1!

Volume Module:

Base Vol:	0	918	9	8	835	0	0	0	0	0	0	5
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	918	9	8	835	0	0	0	0	0	0	5
Added Vol:	0	233	11	1	359	0	0	0	0	7	0	1
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1151	20	9	1194	0	0	0	0	7	0	6
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.82	0.82	0.82	0.86	0.86	0.86	1.00	1.00	1.00	0.42	0.42	0.42
PHF Volume:	0	1400	24	11	1393	0	0	0	0	17	0	14
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	1400	24	11	1393	0	0	0	0	17	0	14

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	6.5	6.2
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	1425	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	2827	2827	1412
Potent Cap.:	xxxx	xxxx	xxxxxx	454	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	20	18	169
Move Cap.:	xxxx	xxxx	xxxxxx	454	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	19	17	169
Volume/Cap:	xxxx	xxxx	xxxx	0.02	xxxx	xxxx	xxxx	xxxx	xxxx	0.88	0.00	0.09

Level of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.1	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	
Control Del:	xxxxxx	xxxx	xxxxxx	13.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	
LOS by Move:	*	*	*	B	*	*	*	*	*	*	*	*	
Movement:	LT	-	LTR	-	RT	LT	-	LTR	-	RT	LT	-	LTR
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	32	xxxxxx	
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	3.3	xxxxxx	
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	326	xxxxxx	
Shared LOS:	*	*	*	*	*	*	*	*	*	*	F	*	
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			326.3			
ApproachLOS:	*			*			*			F			

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #10 Brentwood Blvd. @ Grant Street / Sunset Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.909
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: E

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Grant Street / Sunset Rd. with sub-rows for North, South, East, and West bounds.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and rows for different street directions.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat., showing saturation flow values for different street directions.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves, showing capacity analysis results for different street directions.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #11 Brentwood Blvd. @ Sunset Ct.

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: D[32.4]

Street Name: Brentwood Blvd. Sunset Ct.

Approach:	North Bound					South Bound					East Bound					West Bound				
Movement:	L - T - R					L - T - R					L - T - R					L - T - R				
Control:	Uncontrolled					Uncontrolled					Stop Sign					Stop Sign				
Rights:	Include					Include					Include					Include				
Lanes:	0	0	0	1	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	1

Volume Module:

Base Vol:	0	943	15	6	919	0	0	0	0	0	0	0	25
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	943	15	6	919	0	0	0	0	0	0	0	25
Added Vol:	0	291	0	0	349	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1234	15	6	1268	0	0	0	0	0	0	0	25
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.89	0.89	0.89	0.94	0.94	0.94	1.00	1.00	1.00	1.00	0.57	0.57	0.57
PHF Volume:	0	1380	17	6	1355	0	0	0	0	0	0	0	44
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	1380	17	6	1355	0	0	0	0	0	0	0	44

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	6.2
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	1397	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	1389
Potent Cap.:	xxxx	xxxx	xxxxxx	465	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	175
Move Cap.:	xxxx	xxxx	xxxxxx	465	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	175
Volume/Cap:	xxxx	xxxx	xxxx	0.01	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.25

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.0	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	1.0
Control Del:	xxxxxx	xxxx	xxxxxx	12.9	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	32.4
LOS by Move:	*	*	*	B	*	*	*	*	*	*	*	D
Movement:	LT - LTR - RT			LT - LTR - RT			LT - LTR - RT			LT - LTR - RT		
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			32.4		
ApproachLOS:	*			*			*			D		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #12 Brentwood Blvd. @ Havenwood Ave.

Average Delay (sec/veh): 27.1 Worst Case Level Of Service: F[1481.8]

Table with columns for Street Name (Brentwood Blvd., Havenwood Ave.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1 0 1 0 1, 1 0 0 1 0, 0 0 0 0 1, 0 0 1! 0 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflict Vol, Potent Cap., Move Cap., and Volume/Cap. across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #13 Brentwood Blvd. @ Applewood Common

Cycle (sec): 100 Critical Vol./Cap.(X): 0.866
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 170 Level Of Service: D

Table with columns for Street Name (Brentwood Blvd., Applewood Common), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #14 Brentwood Blvd. @ Sand Creek Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.560
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 42 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Sand Creek Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #15 Brentwood Blvd. @ Technology Way

Cycle (sec): 100 Critical Vol./Cap.(X): 0.330
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 28 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Technology Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #16 Brentwood Blvd @ Nancy Street (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.401
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 31 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Nancy Street (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #17 Brentwood Blvd. @ Village Drive

Average Delay (sec/veh): 5.5 Worst Case Level Of Service: F[218.8]

Table with columns for Street Name (Brentwood Blvd., Village Drive), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table showing traffic volume data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume for each approach and movement.

Critical Gap Module: Table showing critical gap and follow-up time values for each approach and movement.

Capacity Module: Table showing capacity-related metrics such as Conflict Vol, Potent Cap., Move Cap., and Volume/Cap. for each approach and movement.

Level of Service Module: Table showing level of service (LOS) and delay values for each approach and movement, including Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

 Intersection #18 Brentwood Blvd. @ Central Blvd / Sycamore Ave (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.702
 Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 62 Level Of Service: C

Street Name:	Brentwood Blvd.					Central Blvd / Sycamore Ave (S)									
Approach:	North Bound			South Bound		East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected		Permitted			Permitted						
Rights:	Include			Include		Include			Include						
Min. Green:	4	15	15	4	10	10	4	4	4	4	4	4			
Lanes:	1	0	1	0	1	1	0	1	1	0	1	0	1	0	1

Volume Module:

Base Vol:	43	550	41	72	515	70	109	68	38	23	64	71
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	43	550	41	72	515	70	109	68	38	23	64	71
Added Vol:	7	254	40	54	191	107	86	33	4	24	19	32
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	50	804	81	126	706	177	195	101	42	47	83	103
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	50	804	81	126	706	177	195	101	42	47	83	103
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	50	804	81	126	706	177	195	101	42	47	83	103
RTOR Reduct:	0	0	47	0	0	0	0	0	0	0	0	103
RTOR Vol:	50	804	34	126	706	177	195	101	42	47	83	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	50	804	34	126	706	177	195	101	42	47	83	0

Saturation Flow Module:

Sat/Lane:	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	1.00	1.00	1.60	0.40	1.00	1.41	0.59	1.00	1.00	1.00
Final Sat.:	1720	1720	1720	1720	2750	690	1720	2430	1010	1720	1720	1720

Capacity Analysis Module:

Vol/Sat:	0.03	0.47	0.02	0.07	0.26	0.26	0.11	0.04	0.04	0.03	0.05	0.00
Crit Volume:	804			126		195			83			
Crit Moves:	****			****		****			****			

Level of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

Intersection #20 Central Street @ Second Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.272
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 34 Level Of Service: A

Street Name:	Second Street					Central Boulevard									
Approach:	North Bound		South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected		Protected			Protected			Protected						
Rights:	Include		Include			Include			Ignore						
Min. Green:	5	5	5	5	5	5	5	7	7	5	7	7			
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0	2	0	1

Volume Module:

Base Vol:	71	279	71	56	219	1	11	129	49	47	171	72
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	71	279	71	56	219	1	11	129	49	47	171	72
Added Vol:	57	0	0	0	0	0	0	123	39	0	134	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	128	279	71	56	219	1	11	252	88	47	305	72
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	128	279	71	56	219	1	11	252	88	47	305	72
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	128	279	71	56	219	1	11	252	88	47	305	72
RTOR Reduct:	0	0	0	0	0	0	0	0	0	0	0	0
RTOR Vol:	128	279	71	56	219	1	11	252	88	47	305	72
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	128	279	71	56	219	1	11	252	88	47	305	72

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.59	0.41	1.00	1.99	0.01	1.00	1.48	0.52	1.00	2.00	1.00
Final Sat.:	1650	2631	669	1650	3285	15	1650	2446	854	1650	3300	1650

Capacity Analysis Module:

Vol/Sat:	0.08	0.11	0.11	0.03	0.07	0.07	0.01	0.10	0.10	0.03	0.09	0.04
Crit Volume:			175	56			170			47		
Crit Moves:			****	****			****			****		

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #21 Central Blvd @ Walnut Blvd

Cycle (sec): 100 Critical Vol./Cap.(X): 0.280
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 36 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Central Blvd), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #22 Brentwood Blvd @ Pine Street

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: C[20.6]

Table with columns for Street Name (Brentwood Blvd, Pine Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, and Lanes.

Volume Module: Table showing traffic volume data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table showing Critical Gap and FollowUpTim values for different approaches.

Capacity Module: Table showing Capacity-related metrics like Cnflict Vol, Potent Cap., Move Cap., and Volume/Cap.

Level of Service Module: Table showing Level of Service (LOS) by Move, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #23 Brentwood Blvd @ Maple Street

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: B[11.9]

Street Name:	Brentwood Blvd					Maple Street														
Approach:	North Bound		South Bound			East Bound			West Bound											
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Uncontrolled					Uncontrolled					Stop Sign									
Rights:	Include					Include					Include									
Lanes:	0	1	0	1	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	1

Volume Module:

Base Vol:	0	587	12	16	603	0	0	0	0	0	0	0	17
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	587	12	16	603	0	0	0	0	0	0	0	17
Added Vol:	0	266	3	11	206	0	0	0	0	0	0	0	28
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	853	15	27	809	0	0	0	0	0	0	0	45
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	853	15	27	809	0	0	0	0	0	0	0	45
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	853	15	27	809	0	0	0	0	0	0	0	45

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	6.9
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	868	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	434
Potent Cap.:	xxxx	xxxx	xxxxxx	723	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	570
Move Cap.:	xxxx	xxxx	xxxxxx	723	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	570
Volume/Cap:	xxxx	xxxx	xxxx	0.04	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.08

Level of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.1	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	0.3
Control Del:	xxxxxx	xxxx	xxxxxx	10.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	11.9
LOS by Move:	*	*	*	B	*	*	*	*	*	*	*	B
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	0.0	xxxx	xxxxxx	0.1	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	9.0	xxxx	xxxxxx	10.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	A	*	*	B	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx					11.9
ApproachLOS:	*			*			*					B

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #24 Brentwood Blvd @ Oak Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.532
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 49 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Oak St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green, and Lanes.

Table for Volume Module showing Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Table for Saturation Flow Module showing Sat/Lane, Adjustment, Lanes, and Final Sat. values for different movements.

Table for Capacity Analysis Module showing Vol/Sat, Crit Volume, and Crit Moves for various movements.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #25 Walnut Blvd @ Oak St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.460
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 42 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Oak St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green (5, 7, 7, 0, 7, 7, 5, 5, 5, 5, 5, 5), and Lanes (1 0 1 0 1, 1 0 0 1 0, 0 0 1! 0 0, 0 1 0 0 1).

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and values for 12 different movement categories.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. values for 12 movement categories.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves for 12 movement categories.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #26 Brentwood Blvd. @ Second St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.457
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 34 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Second St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Permitted), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and 12 columns of values.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat., with 12 columns of values.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves, with 12 columns of values.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #27 Brentwood Blvd. @ Chestnut St.

Average Delay (sec/veh): 1.5 Worst Case Level Of Service: E[40.9]

Table with columns for Street Name (Brentwood Blvd., Chestnut St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #28 Brentwood Blvd. @ Balfour Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.793
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 110 Level Of Service: C

Table with columns for Street Name (Brentwood Blvd., Balfour Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights (Ignore, Include), Min. Green (8, 10, 10, 6, 10, 10, 5, 5, 5, 4, 5, 5), and Lanes (1, 0, 2, 0, 1, 1, 0, 2, 0, 1, 1, 0, 1, 0, 1, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across various movement categories.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across various movement categories.

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #29 Sellers Ave. @ Sunet Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.514

Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 11.6

Optimal Cycle: 0 Level Of Service: B

Street Name: Sellers Ave. Sunset Rd.

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

-----|-----|-----|-----|-----|

Control: Stop Sign Stop Sign Stop Sign Stop Sign

Rights: Include Include Include Include

Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0

Lanes: 0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0

-----|-----|-----|-----|-----|

Volume Module:

Base Vol: 81 236 14 21 175 39 50 91 62 7 77 30

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 81 236 14 21 175 39 50 91 62 7 77 30

Added Vol: 13 0 0 0 0 0 0 0 0 7 0 0 0

PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0

Initial Fut: 94 236 14 21 175 39 50 91 69 7 77 30

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Volume: 94 236 14 21 175 39 50 91 69 7 77 30

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0

Reduced Vol: 94 236 14 21 175 39 50 91 69 7 77 30

PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

FinalVolume: 94 236 14 21 175 39 50 91 69 7 77 30

-----|-----|-----|-----|-----|

Saturation Flow Module:

Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Lanes: 0.27 0.69 0.04 0.09 0.74 0.17 0.24 0.43 0.33 0.06 0.68 0.26

Final Sat.: 183 459 27 58 485 108 146 266 202 35 389 152

-----|-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat: 0.51 0.51 0.51 0.36 0.36 0.36 0.34 0.34 0.34 0.20 0.20 0.20

Crit Moves: **** **** ****

Delay/Veh: 13.1 13.1 13.1 10.9 10.9 10.9 10.9 10.9 10.9 9.8 9.8 9.8

Delay Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

AdjDel/Veh: 13.1 13.1 13.1 10.9 10.9 10.9 10.9 10.9 10.9 9.8 9.8 9.8

LOS by Move: B B B B B B B B B A A A

ApproachDel: 13.1 10.9 10.9 9.8

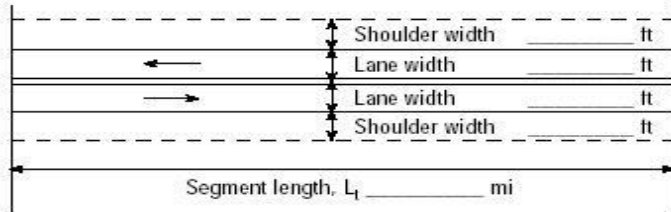
Delay Adj: 1.00 1.00 1.00

ApprAdjDel: 13.1 10.9 10.9 9.8

LOS by Appr: B B B A

AllWayAvgQ: 0.9 0.9 0.9 0.5 0.5 0.5 0.4 0.4 0.4 0.2 0.2 0.2

Note: Queue reported is the number of cars per lane.

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	E Sims Rd. and Delta Rd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP
Project Description:			
Input Data			
		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1423 veh/h Directional split 57 / 43 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P _T 10 % % Recreational vehicles, P _R 0 % Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f _G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))		0.990	
Two-way flow rate ¹ , v _p (pc/h)=V/(PHF * f _G * f _{HV})		1562	
v _p * highest directional split proportion ² (pc/h)		890	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S _{FM}	40 mi/h	Base free-flow speed, BFFS _{FM}	mi/h
Observed volume, V _f	0 veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS FFS=S _{FM} +0.00776(V _f /f _{HV})	mi/h	Adj. for access points, f _A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A)	40.0 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)		1.5	
Average travel speed, ATS (mi/h) ATS=FFS-0.00776v _p -f _{np}		26.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f _G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))		1.000	
Two-way flow rate ¹ , v _p (pc/h)=V/(PHF * f _G * f _{HV})		1547	
v _p * highest directional split proportion ² (pc/h)		882	
Base percent time-spent-following, BPTSF(%)=100(1-e ^{-0.000879v_p})		74.3	
Adj. for directional distribution and no-passing zone, f _{d/np} (%)(Exh. 20-12)		7.1	
Percent time-spent-following, PTSF(%)=BPTSF+f _{d/np}		81.5	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, v/c=V _p /3,200		0.49	
Peak 15-min veh-miles of travel, VMT ₁₅ (veh-mi)=0.25L _i (V/PHF)		116	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	427
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	4.4
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	E Sims Rd. and Delta Rd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The diagram labels: Shoulder width (ft), Lane width (ft), Lane width (ft), Shoulder width (ft), and Segment length, L_i (mi).</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1860 veh/h Directional split 52 / 48 Peak-hour factor, PHF 0.96 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1957	
v_p * highest directional split proportion ² (pc/h)		1018	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	40 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	40.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		1.1	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		23.7	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1938	
v_p * highest directional split proportion ² (pc/h)		1008	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		81.8	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		4.7	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		86.5	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.61	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		145	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	558
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	6.1
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

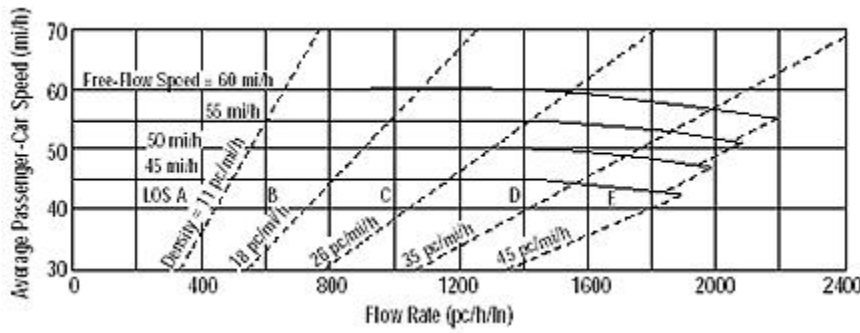
TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	Grant St. to Sunset Ct.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1927 veh/h Directional split 52 / 48 Peak-hour factor, PHF 0.97 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 10	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		2006	
v_p * highest directional split proportion ² (pc/h)		1043	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		1.1	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		13.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1987	
v_p * highest directional split proportion ² (pc/h)		1033	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		82.6	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(\text{Exh. 20-12})$		4.5	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		87.0	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.63	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		50	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	193
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	3.7
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	Grant St. to Sunset Ct.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 2503 veh/h Directional split 51 / 49 Peak-hour factor, PHF 0.90 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 10	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		2809	
v_p * highest directional split proportion ² (pc/h)		1433	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		0.9	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		7.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		2781	
v_p * highest directional split proportion ² (pc/h)		1418	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		91.3	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		2.1	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		93.4	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.88	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		70	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	250
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	9.6
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Central Blvd. to Spruce S
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	672	Peak-Hour Factor, PHF	0.90
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

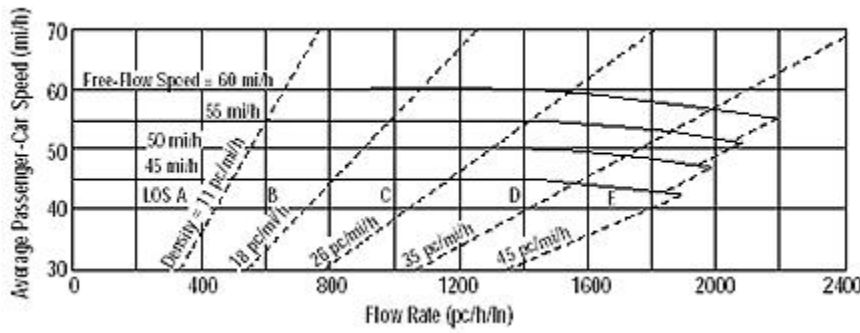
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 392
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 8.7
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_i
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_i

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Brentwood Blvd.
Agency or Company		From/To	Central Blvd. to Spruce S
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	789	Peak-Hour Factor, PHF	0.95
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

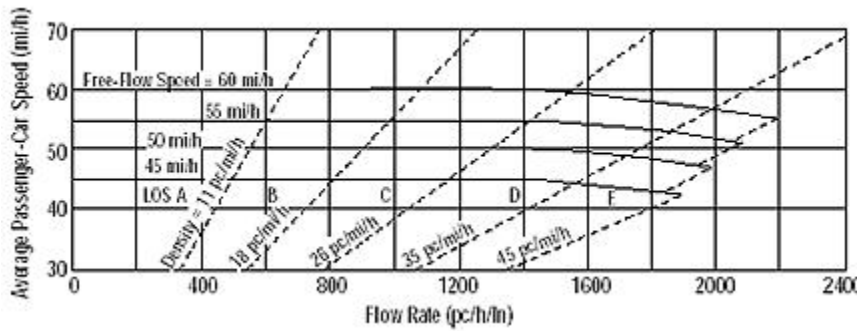
Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	436	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	9.7	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v _p	L
Design (N)	FFS, LOS, v _p	N
Design (v _p)	FFS, LOS, N	v _i
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v _p)	FFS, LOS, N	v _i

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Central Blvd. to Spruce S
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	935	Peak-Hour Factor, PHF	0.78
AADT(veh/h)		%Trucks and Buses, P _T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f _p	1.00	E _R	1.2
E _T	1.5	f _{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

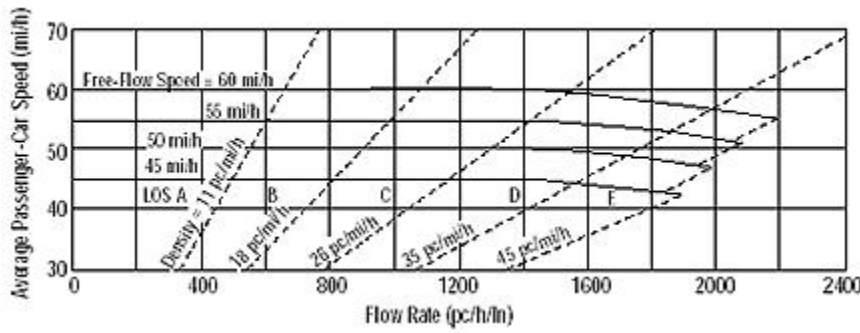
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 629
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 14.0
 LOS: B

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v _p	L
Design (N)	FFS, LOS, v _p	N
Design (v _p)	FFS, LOS, N	v _i
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v _p)	FFS, LOS, N	v _i

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Central Blvd. to Spruce S
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	995	Peak-Hour Factor, PHF	0.99
AADT(veh/h)		%Trucks and Buses, P _T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f _p	1.00	E _R	1.2
E _T	1.5	f _{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

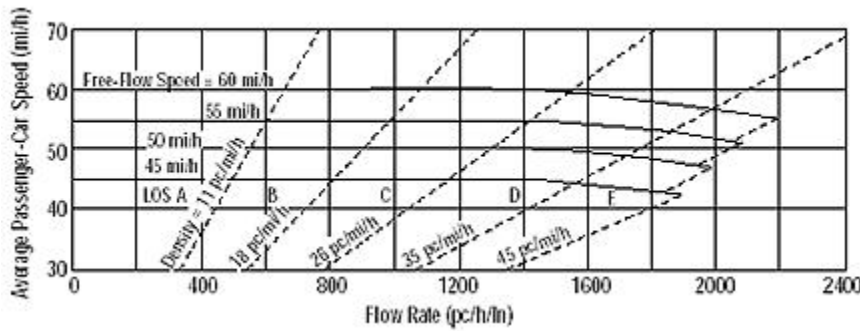
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 527
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 11.7
 LOS: B

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Pine Ave. to Oak Ave.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	647	Peak-Hour Factor, PHF	0.90
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

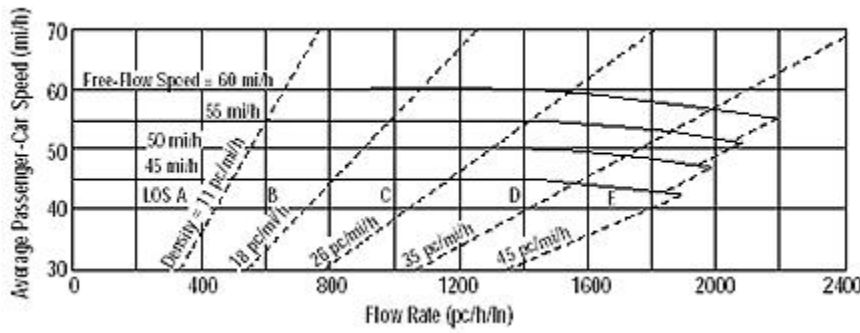
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 377
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 8.4
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Pine Ave. to Oak Ave.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	636	Peak-Hour Factor, PHF	0.88
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

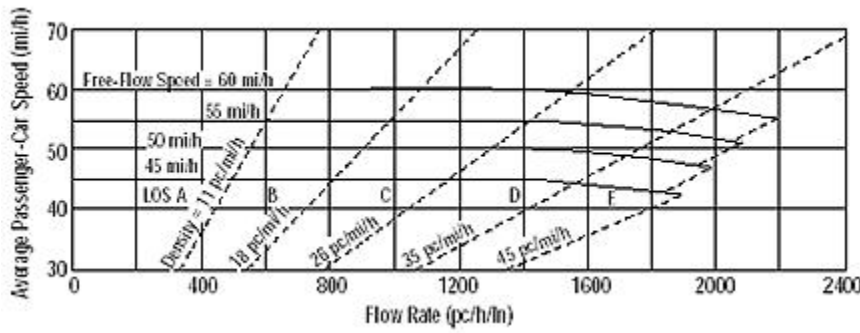
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 379
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 8.4
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Brentwood Blvd.
Agency or Company		From/To	Pine Ave. to Oak Ave.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	904	Peak-Hour Factor, PHF	0.97
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

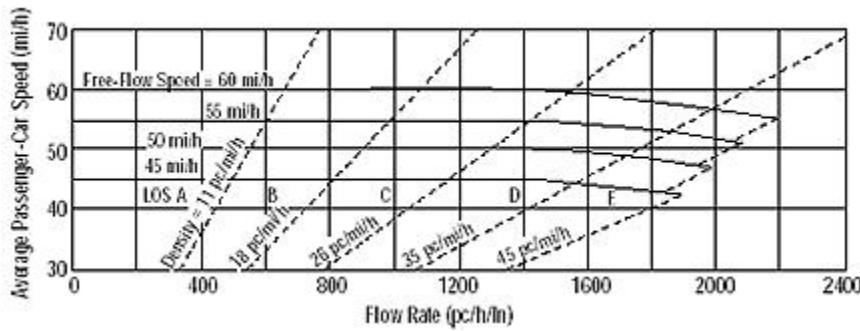
Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	489	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	10.9	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Brentwood Blvd.
Agency or Company		From/To	Pine Ave. to Oak Ave.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	841	Peak-Hour Factor, PHF	0.97
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	455	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	10.1	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Delta Rd.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 362 veh/h Directional split 58 / 42 Peak-hour factor, PHF 0.76 No-passing zone 0 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 40	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		483	
v_p * highest directional split proportion ² (pc/h)		280	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	50 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	50.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		0.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		46.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		477	
v_p * highest directional split proportion ² (pc/h)		277	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		34.2	
Adj. for directional distribution and no-passing zone, $f_{dnp}(\%)(Exh. 20-12)$		0.2	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{dnp}$		34.5	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		A	
Volume to capacity ratio, $v/c=V_p/3,200$		0.15	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		119	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	362
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	2.6
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Delta Rd.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 318 veh/h Directional split 51 / 49 Peak-hour factor, PHF 0.91 No-passing zone 0 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 40	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		354	
v_p * highest directional split proportion ² (pc/h)		181	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	50 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	50.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		0.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		47.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		350	
v_p * highest directional split proportion ² (pc/h)		179	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		26.5	
Adj. for directional distribution and no-passing zone, $f_{dnp}(\%)(Exh. 20-12)$		0.1	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{dnp}$		26.6	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		A	
Volume to capacity ratio, $v/c=V_p/3,200$		0.11	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		87	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	318
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	1.8
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Lone Tree Wy.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 789 veh/h Directional split 64 / 36 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 30	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		861	
v_p * highest directional split proportion ² (pc/h)		551	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	35 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	35.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		2.9	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		25.4	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		859	
v_p * highest directional split proportion ² (pc/h)		550	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		53.0	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(\text{Exh. 20-12})$		13.9	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		66.9	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.27	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		214	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	789
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	8.4
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Lone Tree Wy.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 702 veh/h Directional split 50 / 50 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 30	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		766	
v_p * highest directional split proportion ² (pc/h)		383	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	35 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	35.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.2	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		25.9	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		765	
v_p * highest directional split proportion ² (pc/h)		383	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		49.0	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		16.3	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		65.2	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.24	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		191	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	702
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	7.4
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

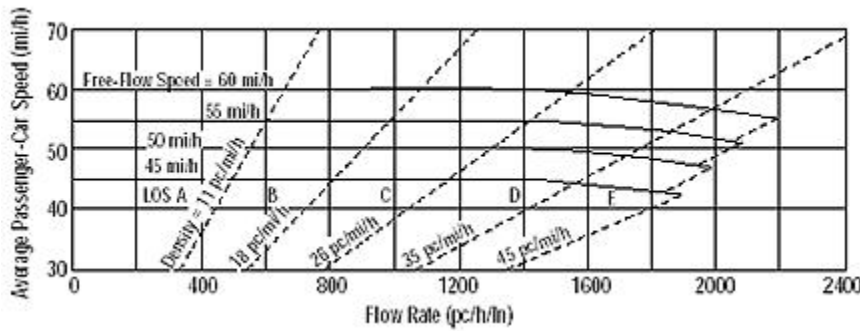
TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sunset Ave
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 705 veh/h Directional split 66 / 34 Peak-hour factor, PHF 0.88 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		804	
v_p * highest directional split proportion ² (pc/h)		531	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		20.8	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		803	
v_p * highest directional split proportion ² (pc/h)		530	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		50.6	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		14.5	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		65.1	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.25	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		200	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	705
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	9.6
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sunset Ave
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i mi. The diagram includes two lanes, each with a width of Lane width ft, and two shoulders, each with a width of Shoulder width ft. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 794 veh/h Directional split 51 / 49 Peak-hour factor, PHF 0.88 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		906	
v_p * highest directional split proportion ² (pc/h)		462	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		2.8	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		20.2	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		904	
v_p * highest directional split proportion ² (pc/h)		461	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		54.8	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(\text{Exh. 20-12})$		14.0	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		68.8	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.28	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		226	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	794
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	11.2
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Central Blvd.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	240	Peak-Hour Factor, PHF	0.86
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

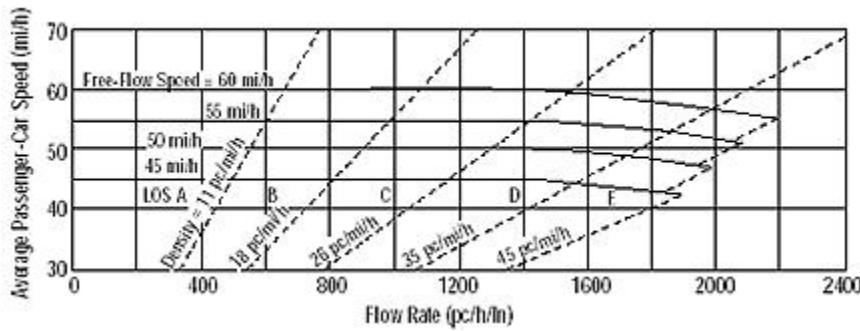
Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	140	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	3.1	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Central Blvd.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	363	Peak-Hour Factor, PHF	0.88
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

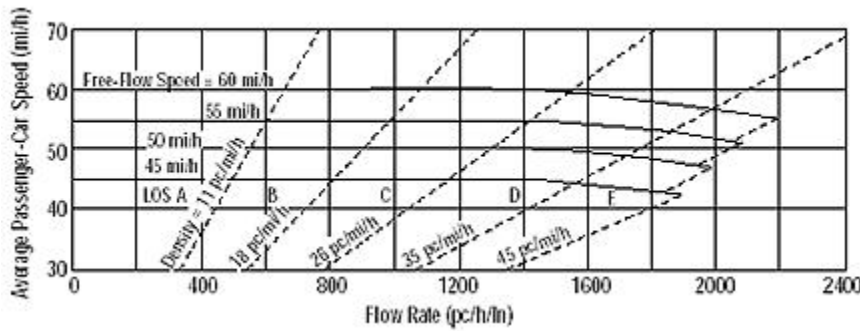
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 208
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 4.6
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Central Blvd.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	338	Peak-Hour Factor, PHF	0.84
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

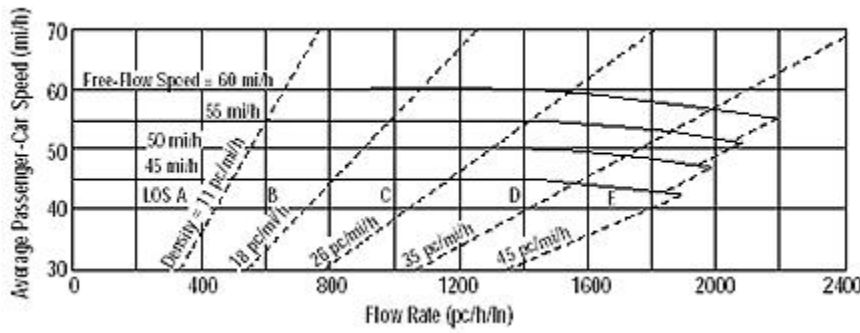
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 203
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 4.5
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Central Blvd.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	437	Peak-Hour Factor, PHF	0.76
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	290	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	6.4	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Grant St.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 301 veh/h Directional split 55 / 45 Peak-hour factor, PHF 0.71 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 7	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		430	
v_p * highest directional split proportion ² (pc/h)		237	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		4.4	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		22.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		425	
v_p * highest directional split proportion ² (pc/h)		234	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		31.2	
Adj. for directional distribution and no-passing zone, $f_{dnp}(\%)(Exh. 20-12)$		23.1	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{dnp}$		54.3	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		B	
Volume to capacity ratio, $v/c=V_p/3,200$		0.13	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		106	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	301
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	4.8
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Grant St.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 244 veh/h Directional split 55 / 45 Peak-hour factor, PHF 0.87 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 7	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		284	
v_p * highest directional split proportion ² (pc/h)		156	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.9	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		23.9	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		281	
v_p * highest directional split proportion ² (pc/h)		155	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		21.9	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(\text{Exh. 20-12})$		23.1	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		44.9	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		B	
Volume to capacity ratio, $v/c=V_p/3,200$		0.09	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		70	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	244
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	2.9
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sycamore Ave.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 504 veh/h Directional split 64 / 36 Peak-hour factor, PHF 0.87 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 35	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		587	
v_p * highest directional split proportion ² (pc/h)		376	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.9	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		21.5	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		580	
v_p * highest directional split proportion ² (pc/h)		371	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		39.9	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		21.0	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		60.9	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.18	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		72	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	252
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	3.3
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sycamore Ave.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 541 veh/h Directional split 57 / 43 Peak-hour factor, PHF 0.82 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 35	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		662	
v_p * highest directional split proportion ² (pc/h)		377	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.6	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		21.2	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		661	
v_p * highest directional split proportion ² (pc/h)		377	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		44.1	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		18.8	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		62.9	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.21	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		82	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	271
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	3.9
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

Appendix C:

*Analysis Worksheets for
Existing plus Approved Projects plus Proposed Project Conditions*

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 Brentwood Blvd. @ Delta Road

Average Delay (sec/veh): 697.0 Worst Case Level Of Service: F[7437.3]

Street Name:	Brentwood Blvd.					Delta Rd.						
Approach:	North Bound		South Bound		East Bound		West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled		Uncontrolled		Stop Sign		Stop Sign					
Rights:	Include		Include		Include		Include					
Lanes:	0	0	0	1	0	0	0	1	0	0	0	0

Volume Module:

Base Vol:	0	770	39	68	564	0	0	0	0	63	0	89
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	770	39	68	564	0	0	0	0	63	0	89
Added Vol:	0	250	385	94	460	0	0	0	0	105	0	16
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1020	424	162	1024	0	0	0	0	168	0	105
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.90	0.90	0.90	1.00	1.00	1.00	0.91	0.91	0.91
PHF Volume:	0	1131	470	181	1142	0	0	0	0	186	0	116
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	1131	470	181	1142	0	0	0	0	186	0	116

Critical Gap Module:

Critical Gp:xxxxxx	xxxx	xxxx	xxxxxx	4.2	xxxx	xxxxxx	7.1	6.5	6.2	6.4	6.5	6.2
FollowUpTim:xxxxxx	xxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	3.5	4.0	3.3	3.5	4.0	3.3

Capacity Module:

Cnflict Vol:xxxxxx	xxxx	xxxx	xxxxxx	1601	xxxx	xxxxxx	2927	3104	1142	2869	2869	1366
Potent Cap.:xxxxxx	xxxx	xxxx	xxxxxx	387	xxxx	xxxxxx	10	12	244	18	17	180
Move Cap.:xxxxxx	xxxx	xxxx	xxxxxx	387	xxxx	xxxxxx	2	6	244	12	9	180
Volume/Cap:xxxxxx	xxxx	xxxx	xxxxxx	0.47	xxxx	xxxxxx	0.00	0.00	0.00	15.98	0.00	0.64

Level of Service Module:

2Way95thQ:xxxxxx	xxxx	xxxx	xxxxxx	2.4	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:xxxxxx	xxxx	xxxx	xxxxxx	22.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	*	*	*	C	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	0	xxxxxx	xxxx	18	xxxxxx
SharedQueue:xxxxxx	xxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	38.4	xxxxxx
Shrd ConDel:xxxxxx	xxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	7437	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	F	*
ApproachDel:xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	xxxxxx	7437.3		
ApproachLOS:	*	*	*	*	*	*	*	*	*	F		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #2 Sellers Ave @ Delta

Cycle (sec): 100 Critical Vol./Cap.(X): 0.565
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 13.5
 Optimal Cycle: 0 Level Of Service: B

Street Name: Sellers Ave. Delta Rd.

Approach: North Bound South Bound East Bound West Bound
 Movement: L - T - R L - T - R L - T - R L - T - R

Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0

Volume Module:

Base Vol:	44	125	69	13	202	37	18	107	85	110	123	12
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	44	125	69	13	202	37	18	107	85	110	123	12
Added Vol:	5	0	19	0	0	0	0	21	1	28	55	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	49	125	88	13	202	37	18	128	86	138	178	12
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	49	125	88	13	202	37	18	128	86	138	178	12
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	49	125	88	13	202	37	18	128	86	138	178	12
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	49	125	88	13	202	37	18	128	86	138	178	12

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.19	0.48	0.33	0.05	0.80	0.15	0.08	0.55	0.37	0.42	0.54	0.04
Final Sat.:	107	272	192	29	449	82	44	314	211	244	315	21

Capacity Analysis Module:

Vol/Sat:	0.46	0.46	0.46	0.45	0.45	0.45	0.41	0.41	0.41	0.56	0.56	0.56
Crit Moves:	****			****			****			****		
Delay/Veh:	13.0	13.0	13.0	13.0	13.0	13.0	12.2	12.2	12.2	15.4	15.4	15.4
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	13.0	13.0	13.0	13.0	13.0	13.0	12.2	12.2	12.2	15.4	15.4	15.4
LOS by Move:	B	B	B	B	B	B	B	B	B	C	C	C
ApproachDel:	13.0			13.0			12.2			15.4		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	13.0			13.0			12.2			15.4		
LOS by Appr:	B			B			B			C		
AllWayAvgQ:	0.7	0.7	0.7	0.6	0.6	0.6	0.5	0.5	0.5	1.0	1.0	1.0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #3 Brentwood Blvd. @ E. Sims Road

Average Delay (sec/veh): 676.8 Worst Case Level Of Service: F[10875.4]

Street Name: Brentwood Blvd. E. Sims Road

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign										
Rights:	Include			Include			Include			Include										
Lanes:	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	0	0

Volume Module:

Base Vol:	0	780	29	21	593	0	0	0	0	0	53	0	35
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	780	29	21	593	0	0	0	0	0	53	0	35
Added Vol:	0	616	361	108	457	0	0	0	0	0	63	0	19
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1396	390	129	1050	0	0	0	0	0	116	0	54
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.94	0.94	0.94	0.87	0.87	0.87	1.00	1.00	1.00	1.00	0.79	0.79	0.79
PHF Volume:	0	1485	415	148	1201	0	0	0	0	0	147	0	68
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	1485	415	148	1201	0	0	0	0	0	147	0	68

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	6.5	6.2
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	1900	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	3189	3189	1693
Potent Cap.:	xxxx	xxxx	xxxxxx	295	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	11	10	115
Move Cap.:	xxxx	xxxx	xxxxxx	295	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	6	5	115
Volume/Cap:	xxxx	xxxx	xxxx	0.50	xxxx	xxxx	xxxx	xxxx	xxxx	22.82	0.00	0.59

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	2.6	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	xxxxxx	xxxx	xxxxxx	28.8	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	*	*	*	D	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	9	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	2.6	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	28.6	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	28.8	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	D	*	*	*	*	*	*	F	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	*			*			*			F		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

 Intersection #4 Brentwood Blvd @ Lone Tree Way (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 1.213
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 180 Level Of Service: F

Street Name:	Brentwood Blvd						Lone Tree Way (S)					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Ovl			Include		
Min. Green:	4	8	8	4	8	8	4	4	4	4	4	4
Lanes:	1	0	1	0	1	1	0	1	0	0	1	0

Volume Module:

Base Vol:	300	536	23	11	604	196	135	8	140	56	10	9
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	300	536	23	11	604	196	135	8	140	56	10	9
Added Vol:	260	729	250	111	369	40	228	113	324	44	20	19
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	560	1265	273	122	973	236	363	121	464	100	30	28
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	560	1265	273	122	973	236	363	121	464	100	30	28
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	560	1265	273	122	973	236	363	121	464	100	30	28
RTOR Reduct:	0	0	100	0	0	0	0	0	464	0	0	28
RTOR Vol:	560	1265	173	122	973	236	363	121	0	100	30	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	560	1265	173	122	973	236	363	121	0	100	30	0

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	1.00	1.00	1.61	0.39	0.75	0.25	1.00	0.77	0.23	1.00
Final Sat.:	1650	1650	1650	1650	2656	644	1238	413	1650	1269	381	1650

Capacity Analysis Module:

Vol/Sat:	0.34	0.77	0.10	0.07	0.37	0.37	0.29	0.29	0.00	0.08	0.08	0.00
Crit Volume:	1265			122			484			130		
Crit Moves:	****			****			****			****		

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #5 Arroyo Seco Road @ Lone Tree Way

Average Delay (sec/veh): 10.1 Worst Case Level Of Service: E[41.4]

Street Name:	Arroyo Seco Road						Lone Tree Way					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	0	1	0	0	1	0	0

Volume Module:

Base Vol:	52	0	0	0	0	0	0	8	34	0	21	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	52	0	0	0	0	0	0	8	34	0	21	0
Added Vol:	0	0	0	0	0	83	473	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	52	0	0	0	0	83	473	8	34	0	21	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	52	0	0	0	0	83	473	8	34	0	21	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	52	0	0	0	0	83	473	8	34	0	21	0

Critical Gap Module:

Critical Gp:	7.1	6.5	6.2	7.1	6.5	6.2	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx
FollowUpTim:	3.5	4.0	3.3	3.5	4.0	3.3	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflct Vol:	1034	992	25	992	1009	21	21	xxxx	xxxxx	xxxx	xxxx	xxxxx
Potent Cap.:	211	246	1051	225	240	1056	1595	xxxx	xxxxx	xxxx	xxxx	xxxxx
Move Cap.:	149	173	1051	173	169	1056	1595	xxxx	xxxxx	xxxx	xxxx	xxxxx
Volume/Cap:	0.35	0.00	0.00	0.00	0.00	0.08	0.30	xxxx	xxxx	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	1.4	xxxx	xxxxx	xxxx	xxxx	xxxxx	1.3	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	41.4	xxxx	xxxxx	xxxxx	xxxx	xxxxx	8.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	E	*	*	*	*	*	A	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	0	xxxx	xxxx	1056	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	0.3	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	8.7	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	A	*	*	*	*	*	*
ApproachDel:	41.4			8.7			xxxxxxx			xxxxxxx		
ApproachLOS:	E			A			*			*		

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #6 Brentwood Blvd. @ Sunrise Drive

Average Delay (sec/veh): 143.9 Worst Case Level Of Service: F[8609.2]

Table with columns for Street Name, Approach, Movement, Control, Rights, and Lanes. Rows include North Bound, South Bound, East Bound, and West Bound movements.

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume for each approach.

Critical Gap Module: Table showing Critical Gap and FollowUpTim for each approach.

Capacity Module: Table showing Cnflict Vol, Potent Cap., Move Cap., and Volume/Cap. for each approach.

Level of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS for each approach.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #7 Brentwood Blvd. @ Gregory Lane

Average Delay (sec/veh): 178.2 Worst Case Level Of Service: F[8924.1]

Street Name: Brentwood Blvd. Gregory Ln.

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #8 Brentwood Blvd. @ Hanson Lane

Average Delay (sec/veh): 3570.6 Worst Case Level Of Service: F[373766.9]

Table with columns for Street Name (Brentwood Blvd., Hanson Ln), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1 0 0 1 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflict Vol, Potent Cap., Move Cap., and Volume/Cap.

Level of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #9 Brentwood Blvd. @ Homecoming Way

Average Delay (sec/veh): 444.1 Worst Case Level Of Service: F[18048.4]

Table with columns for Street Name (Brentwood Blvd., Homecoming Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table showing traffic volume data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across different movements.

Critical Gap Module: Table showing Critical Gap and FollowUpTim values for different movements.

Capacity Module: Table showing Capacity-related metrics like Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap. for various movements.

Level of Service Module: Table showing Level of Service (LOS) and other metrics like 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #10 Brentwood Blvd. @ Grant Street / Sunset Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 1.601
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Grant Street / Sunset Rd. with sub-rows for North, South, East, and West bounds.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, etc.) and rows for different traffic scenarios.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat., showing saturation flow values for different approaches.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves, providing capacity analysis for each approach.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #11 Brentwood Blvd. @ Sunset Ct.

Average Delay (sec/veh): 3.2 Worst Case Level Of Service: F[297.1]

Street Name: Brentwood Blvd. Sunset Ct.

Approach:	North Bound					South Bound					East Bound					West Bound				
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Control:	Uncontrolled					Uncontrolled					Stop Sign					Stop Sign				
Rights:	Include					Include					Include					Include				
Lanes:	0	0	0	1	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	1

Volume Module:

Base Vol:	0	973	7	13	911	0	0	0	0	0	0	0	17
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	973	7	13	911	0	0	0	0	0	0	0	17
Added Vol:	0	1122	0	7	850	0	0	0	0	0	0	0	16
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	2095	7	20	1761	0	0	0	0	0	0	0	33
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.88	0.88	0.88	0.91	0.91	0.91	1.00	1.00	1.00	1.00	0.73	0.73	0.73
PHF Volume:	0	2383	8	22	1933	0	0	0	0	0	0	0	46
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	2383	8	22	1933	0	0	0	0	0	0	0	46

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	6.2
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	2391	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	2387
Potent Cap.:	xxxx	xxxx	xxxxxx	188	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	43
Move Cap.:	xxxx	xxxx	xxxxxx	188	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	43
Volume/Cap:	xxxx	xxxx	xxxx	0.12	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	1.05

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.4	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	4.3			
Control Del:	xxxxxx	xxxx	xxxxxx	26.7	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	297.1			
LOS by Move:	*	*	*	D	*	*	*	*	*	*	*	F			
Movement:	LT	-	LTR	-	RT	LT	-	LTR	-	RT	LT	-	LTR	-	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx			
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx			
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx			
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*			
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			297.1					
ApproachLOS:	*			*			*			F					

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #12 Brentwood Blvd. @ Havenwood Ave.

Average Delay (sec/veh): 4210.6 Worst Case Level Of Service: F[75498.1]

Table with columns for Street Name (Brentwood Blvd., Havenwood Ave.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1 0 1 0 1, 1 0 0 1 0, 0 0 0 0 1, 0 0 1! 0 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume, and values for Brentwood Blvd. and Havenwood Ave.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for Brentwood Blvd. and Havenwood Ave.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap., and values for Brentwood Blvd. and Havenwood Ave.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS, and values for Brentwood Blvd. and Havenwood Ave.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

Intersection #13 Brentwood Blvd. @ Applewood Common

Cycle (sec): 100 Critical Vol./Cap.(X): 1.323
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 180 Level Of Service: F

Street Name: Brentwood Blvd. Applewood Common

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

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Control: Protected Protected Split Phase Split Phase

Rights: Include Include Include Include

Min. Green: 4 8 8 4 8 8 4 4 4 4 4 4

Lanes: 1 0 1 0 1 1 0 0 1 0 1 0 0 1 0

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Volume Module:

Base Vol:	17	925	42	18	889	10	25	2	28	50	0	23
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	17	925	42	18	889	10	25	2	28	50	0	23
Added Vol:	66	1067	29	32	792	72	46	0	42	19	0	20
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	83	1992	71	50	1681	82	71	2	70	69	0	43
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	83	1992	71	50	1681	82	71	2	70	69	0	43
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	83	1992	71	50	1681	82	71	2	70	69	0	43
RTOR Reduct:	0	0	69	0	0	0	0	0	0	0	0	0
RTOR Vol:	83	1992	2	50	1681	82	71	2	70	69	0	43
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	83	1992	2	50	1681	82	71	2	70	69	0	43

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Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	1.00	1.00	0.95	0.05	1.00	0.03	0.97	1.00	0.00	1.00
Final Sat.:	1650	1650	1650	1650	1573	77	1650	46	1604	1650	0	1650

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Capacity Analysis Module:

Vol/Sat:	0.05	1.21	0.00	0.03	1.07	1.07	0.04	0.04	0.04	0.04	0.00	0.03
Crit Volume:		1992		50				72		69		
Crit Moves:		****		****				****		****		

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

 Intersection #14 Brentwood Blvd. @ Sand Creek Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 1.127
 Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 180 Level Of Service: F

Street Name: Brentwood Blvd. Sand Creek Rd.

Approach:	North Bound					South Bound					East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Control:	Protected					Protected					Protected			Protected						
Rights:	Include					Include					Include			Include						
Min. Green:	4	10	10			4	10	10			4	4	4			0	0	0		
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0	1	1	0	1	0	1	1	0

Volume Module:

Base Vol:	203	624	0	0	784	235	146	0	116	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	203	624	0	0	784	235	146	0	116	0	0	0
Added Vol:	75	582	73	158	441	253	479	73	104	46	46	100
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	278	1206	73	158	1225	488	625	73	220	46	46	100
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	278	1206	73	158	1225	488	625	73	220	46	46	100
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	278	1206	73	158	1225	488	625	73	220	46	46	100
RTOR Reduct:	0	0	0	0	0	0	0	0	0	0	0	0
RTOR Vol:	278	1206	73	158	1225	488	625	73	220	46	46	100
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	278	1206	73	158	1225	488	625	73	220	46	46	100

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.89	0.11	1.00	1.43	0.57	1.00	1.00	1.00	1.00	1.00	1.00
Final Sat.:	1650	3112	188	1650	2360	940	1650	1650	1650	1650	1650	1650

Capacity Analysis Module:

Vol/Sat:	0.17	0.39	0.39	0.10	0.52	0.52	0.38	0.04	0.13	0.03	0.03	0.06
Crit Volume:	278				857		625					100
Crit Moves:	****				****		****					****

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #15 Brentwood Blvd. @ Technology Way

Cycle (sec): 100 Critical Vol./Cap.(X): 0.472
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 35 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Technology Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #16 Brentwood Blvd @ Nancy Street (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.602
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 47 Level Of Service: B

Table with columns for Street Name (Brentwood Blvd, Nancy Street (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #17 Brentwood Blvd. @ Village Drive

Average Delay (sec/veh): 55.9 Worst Case Level Of Service: F[1545.0]

Table with columns for Street Name (Brentwood Blvd., Village Drive), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table showing traffic volume calculations including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume.

Critical Gap Module: Table showing critical gap and follow-up time values for different movements.

Capacity Module: Table showing conflict volume, potential capacity, move capacity, and volume/capacity ratios.

Level Of Service Module: Table showing level of service (LOS) by movement, approach delay, and approach LOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

 Intersection #18 Brentwood Blvd. @ Central Blvd / Sycamore Ave (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.937
 Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 180 Level Of Service: E

Street Name:	Brentwood Blvd.					Central Blvd / Sycamore Ave (S)									
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected			Permitted			Permitted					
Rights:	Include			Include			Include			Include					
Min. Green:	4	15	15	4	10	10	4	4	4	4	4	4			
Lanes:	1	0	1	0	1	1	0	1	1	0	1	0	1	0	1

Volume Module:

Base Vol:	43	587	42	78	615	112	140	59	41	69	119	137
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	43	587	42	78	615	112	140	59	41	69	119	137
Added Vol:	2	478	15	92	259	51	114	6	3	9	4	59
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	45	1065	57	170	874	163	254	65	44	78	123	196
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	45	1065	57	170	874	163	254	65	44	78	123	196
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	45	1065	57	170	874	163	254	65	44	78	123	196
RTOR Reduct:	0	0	57	0	0	0	0	0	0	0	0	170
RTOR Vol:	45	1065	0	170	874	163	254	65	44	78	123	26
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	45	1065	0	170	874	163	254	65	44	78	123	26

Saturation Flow Module:

Sat/Lane:	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	1.00	1.00	1.69	0.31	1.00	1.19	0.81	1.00	1.00	1.00
Final Sat.:	1720	1720	1720	1720	2899	541	1720	2051	1389	1720	1720	1720

Capacity Analysis Module:

Vol/Sat:	0.03	0.62	0.00	0.10	0.30	0.30	0.15	0.03	0.03	0.05	0.07	0.02
Crit Volume:		1065		170			254			123		
Crit Moves:		****		****			****			****		

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

Intersection #20 Central Street @ Second Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.377
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 37 Level Of Service: A

Street Name:	Second Street						Central Boulevard								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected			Protected			Protected					
Rights:	Include			Include			Include			Ignore					
Min. Green:	5	5	5	5	5	5	5	7	7	5	7	7			
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0	2	0	1

Volume Module:

Base Vol:	80	195	36	54	388	4	6	257	182	51	263	49
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	80	195	36	54	388	4	6	257	182	51	263	49
Added Vol:	0	26	0	0	16	13	20	123	0	0	57	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	80	221	36	54	404	17	26	380	182	51	320	49
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	80	221	36	54	404	17	26	380	182	51	320	49
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	80	221	36	54	404	17	26	380	182	51	320	49
RTOR Reduct:	0	0	0	0	0	0	0	0	0	0	0	0
RTOR Vol:	80	221	36	54	404	17	26	380	182	51	320	49
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	80	221	36	54	404	17	26	380	182	51	320	49

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.72	0.28	1.00	1.92	0.08	1.00	1.35	0.65	1.00	2.00	1.00
Final Sat.:	1650	2838	462	1650	3167	133	1650	2231	1069	1650	3300	1650

Capacity Analysis Module:

Vol/Sat:	0.05	0.08	0.08	0.03	0.13	0.13	0.02	0.17	0.17	0.03	0.10	0.03
Crit Volume:	80			211			281			51		
Crit Moves:	****			****			****			****		

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #21 Central Blvd @ Walnut Blvd

Cycle (sec): 100 Critical Vol./Cap.(X): 0.287
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 36 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Central Blvd), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green (5, 7, 7), and Lanes (1 0 1 0 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #22 Brentwood Blvd @ Pine Street

Average Delay (sec/veh): 1.0 Worst Case Level Of Service: C[24.3]

Table with columns for Street Name (Brentwood Blvd, Pine Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, and Lanes.

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume for each approach.

Critical Gap Module: Table showing Critical Gap and FollowUpTime for each approach.

Capacity Module: Table showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap. for each approach.

Level Of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS for each approach.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #23 Brentwood Blvd @ Maple Street

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: B[13.3]

Table with columns for Street Name (Brentwood Blvd, Maple Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #24 Brentwood Blvd @ Oak Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.567
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 53 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Oak St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green (4, 10, 10), and Lanes (1, 0, 1, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #25 Walnut Blvd @ Oak St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.438
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 41 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Oak St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with columns for various volume adjustment factors (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and their values for each approach.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. values for each approach.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves for each approach.

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

 Intersection #26 Brentwood Blvd. @ Second St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.574
 Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 44 Level Of Service: A

Street Name: Brentwood Blvd. Second St.

Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected			Permitted			Permitted					
Rights:	Include			Include			Include			Include					
Min. Green:	4	8	8	4	8	8	4	4	4	4	4	4			
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0	2	0	1

Volume Module:

Base Vol:	111	496	21	203	564	22	114	403	90	35	188	148
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	111	496	21	203	564	22	114	403	90	35	188	148
Added Vol:	26	471	0	0	223	0	0	0	16	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	137	967	21	203	787	22	114	403	106	35	188	148
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	137	967	21	203	787	22	114	403	106	35	188	148
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	137	967	21	203	787	22	114	403	106	35	188	148
RTOR Reduct:	0	0	0	0	0	0	0	0	0	0	0	148
RTOR Vol:	137	967	21	203	787	22	114	403	106	35	188	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	137	967	21	203	787	22	114	403	106	35	188	0

Saturation Flow Module:

Sat/Lane:	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.96	0.04	1.00	1.95	0.05	1.00	1.58	0.42	1.00	2.00	1.00
Final Sat.:	1720	3367	73	1720	3346	94	1720	2724	716	1720	3440	1720

Capacity Analysis Module:

Vol/Sat:	0.08	0.29	0.29	0.12	0.24	0.24	0.07	0.15	0.15	0.02	0.05	0.00
Crit Volume:		494		203				255		35		
Crit Moves:		****		****				****		****		

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #27 Brentwood Blvd. @ Chestnut St.

Average Delay (sec/veh): 2.9 Worst Case Level Of Service: F[71.3]

Table with columns for Street Name (Brentwood Blvd., Chestnut St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume, with values for each of the 12 movements.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim, with values for each of the 12 movements.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap., with values for each of the 12 movements.

Level of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS, with values for each of the 12 movements.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #28 Brentwood Blvd. @ Balfour Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.666
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 68 Level Of Service: B

Table with columns for Street Name (Brentwood Blvd., Balfour Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights (Ignore, Include), Min. Green (8, 10, 10, 6, 10, 10, 5, 5, 5, 4, 5, 5), and Lanes (1 0 2 0 1, 1 0 2 0 1, 1 0 1 0 1, 1 0 1 1 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across 12 lanes.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across 12 lanes.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across 12 lanes.

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #29 Sellers Ave. @ Sunet Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.611
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 13.1
Optimal Cycle: 0 Level Of Service: B

Street Name: Sellers Ave. Sunset Rd.

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Approach, Movement, Control, Rights, Min. Green, and Lanes.

Volume Module: Table with 12 columns representing different traffic movements and 13 rows of volume-related metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module: Table with 12 columns and 3 rows showing adjustment factors, lane saturation, and final saturation values.

Capacity Analysis Module: Table with 12 columns and 13 rows showing delay, LOS, and other performance metrics for each movement.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 Brentwood Blvd. @ Delta Road

Average Delay (sec/veh): 3259.1 Worst Case Level Of Service: F[17141.7]

Street Name:	Brentwood Blvd.					Delta Rd.						
Approach:	North Bound		South Bound		East Bound		West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled		Uncontrolled		Stop Sign		Stop Sign					
Rights:	Include		Include		Include		Include					
Lanes:	0	0	0	1	0	0	1	0	0	0	0	0

Volume Module:

Base Vol:	0	862	104	63	810	0	0	0	0	80	0	81
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	862	104	63	810	0	0	0	0	80	0	81
Added Vol:	0	487	125	21	293	0	0	0	0	349	0	84
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1349	229	84	1103	0	0	0	0	429	0	165
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.93	0.93	0.93	0.94	0.94	0.94	1.00	1.00	1.00	0.86	0.86	0.86
PHF Volume:	0	1449	246	89	1172	0	0	0	0	501	0	193
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	1449	246	89	1172	0	0	0	0	501	0	193

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.2	xxxx	xxxxxx	7.1	6.5	6.2	6.4	6.5	6.2
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	3.5	4.0	3.3	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	1695	xxxx	xxxxxx	3019	3046	1172	2923	2923	1572
Potent Cap.:	xxxx	xxxx	xxxxxx	356	xxxx	xxxxxx	8	13	234	17	15	136
Move Cap.:	xxxx	xxxx	xxxxxx	356	xxxx	xxxxxx	0	9	234	14	11	136
Volume/Cap:	xxxx	xxxx	xxxx	0.25	xxxx	xxxx	xxxx	0.00	0.00	36.78	0.00	1.42

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	1.0	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	xxxxxx	xxxx	xxxxxx	18.5	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	*	*	*	C	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	0	xxxxxx	xxxx	18	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	87.4	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	F	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	*			*			*			F		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #2 Sellers Ave @ Delta

Cycle (sec): 100 Critical Vol./Cap.(X): 0.457
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 11.1
Optimal Cycle: 0 Level Of Service: B

Street Name: Sellers Ave. Delta Rd.

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 13 columns representing different traffic movements and 13 rows of volume-related metrics such as Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 13 columns and 3 rows showing saturation flow metrics like Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 13 columns and 13 rows showing capacity analysis metrics like Vol/Sat, Crit Moves, Delay/Veh, etc.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #3 Brentwood Blvd. @ E. Sims Road

Average Delay (sec/veh): 4630.4 Worst Case Level Of Service: F[34804.9]

Street Name:	Brentwood Blvd.			E. Sims Road						
Approach:	North Bound		South Bound		East Bound		West Bound			
Movement:	L	T	R	L	T	R	L	T	R	
Control:	Uncontrolled			Uncontrolled			Stop Sign		Stop Sign	
Rights:	Include			Include			Include		Include	
Lanes:	0	0	1	0	1	0	0	0	0	0

Volume Module:

Base Vol:	0	961	37	31	863	0	0	0	0	53	0	21
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	961	37	31	863	0	0	0	0	53	0	21
Added Vol:	0	515	81	24	618	0	0	0	0	322	0	96
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1476	118	55	1481	0	0	0	0	375	0	117
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.88	0.88	0.88	0.88	0.88	0.88	1.00	1.00	1.00	0.90	0.90	0.90
PHF Volume:	0	1675	134	62	1679	0	0	0	0	415	0	130
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	1675	134	62	1679	0	0	0	0	415	0	130

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	6.5	6.2
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	1809	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	3546	3546	1742
Potent Cap.:	xxxx	xxxx	xxxxxx	321	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	7	6	107
Move Cap.:	xxxx	xxxx	xxxxxx	321	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	6	5	107
Volume/Cap:	xxxx	xxxx	xxxx	0.19	xxxx	xxxx	xxxx	xxxx	xxxx	74.89	0.00	1.21

Level of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.7	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	xxxxxx	xxxx	xxxxxx	18.9	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	*	*	*	C	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	7	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	0.7	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	70.1	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	18.9	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	C	*	*	*	*	*	*	F	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			xxxxxx		
ApproachLOS:	*			*			*			F		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #4 Brentwood Blvd @ Lone Tree Way (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 1.261
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with columns for Street Name (Brentwood Blvd, Lone Tree Way (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include, Ovl, Include), Min. Green (4, 8, 8, 4, 8, 8, 4, 4, 4, 4, 4, 4), and Lanes (1, 0, 1, 0, 1, 1, 0, 1, 1, 0, 1, 0, 1, 0, 0, 1, 0, 1, 0, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across 12 lanes.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across 12 lanes.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across 12 lanes.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #5 Arroyo Seco Road @ Lone Tree Way

Average Delay (sec/veh): 9.0 Worst Case Level Of Service: C[19.0]

Table with columns for Street Name, Approach, Movement, Control, Rights, and Lanes. Rows include Arroyo Seco Road and Lone Tree Way with various movement details.

Volume Module: Table showing traffic volume data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table showing critical gap and follow-up time values for different movements.

Capacity Module: Table showing capacity-related metrics such as Cnfflict Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table showing level of service metrics including 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #6 Brentwood Blvd. @ Sunrise Drive

Average Delay (sec/veh): 2033.5 Worst Case Level Of Service: F[108414.7]

Table with columns for Street Name (Brentwood Blvd., Sunrise Dr.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #7 Brentwood Blvd. @ Gregory Lane

Average Delay (sec/veh): 2355.5 Worst Case Level Of Service: F[119289.2]

Street Name: Brentwood Blvd. Gregory Ln.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	0	0	0	1	0	0	1

Volume Module:

Base Vol:	28	1251	0	0	1116	40	30	0	37	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	28	1251	0	0	1116	40	30	0	37	0	0	0
Added Vol:	20	869	0	0	1206	22	17	0	16	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	48	2120	0	0	2322	62	47	0	53	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.89	0.89	0.89	0.81	0.81	0.81	0.92	0.92	0.92	1.00	1.00	1.00
PHF Volume:	54	2395	0	0	2860	76	51	0	57	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	54	2395	0	0	2860	76	51	0	57	0	0	0

Critical Gap Module:

Critical Gp:	4.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx	6.4	6.5	6.2	7.1	6.5	6.2
FollowUpTim:	2.3	xxxx	xxxxx	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	2936	xxxx	xxxxx	xxxx	xxxx	xxxxx	5402	5402	2898	5430	5440	2395
Potent Cap.:	113	xxxx	xxxxx	xxxx	xxxx	xxxxx	0	0	21	0	0	43
Move Cap.:	113	xxxx	xxxxx	xxxx	xxxx	xxxxx	0	0	21	0	0	43
Volume/Cap:	0.48	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.00	2.76	xxxx	0.00	0.00

Level Of Service Module:

2Way95thQ:	2.1	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx			
Control Del:	63.3	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx			
LOS by Move:	F	*	*	*	*	*	*	*	*	*	*	*			
Movement:	LT	-	LTR	-	RT	LT	-	LTR	-	RT	LT	-	LTR	-	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	0	xxxxx	xxxx	0	xxxxx			
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	16.0	xxxxx	xxxxx	xxxx	xxxxx			
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx			
Shared LOS:	*	*	*	*	*	*	*	F	*	*	*	*			
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			xxxxxx					
ApproachLOS:	*			*			F			*					

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #8 Brentwood Blvd. @ Hanson Lane

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Table with columns for Street Name (Brentwood Blvd., Hanson Ln), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1 0 0 1 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #9 Brentwood Blvd. @ Homecoming Way

Average Delay (sec/veh): 2051.6 Worst Case Level Of Service: F[104874.5]

Table with columns for Street Name (Brentwood Blvd., Homecoming Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table showing traffic volume data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume for each movement.

Critical Gap Module: Table showing critical gap and follow-up time values for each movement.

Capacity Module: Table showing capacity-related metrics such as Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap for each movement.

Level of Service Module: Table showing level of service (LOS) and delay (ApproachDel, ApproachLOS) for each movement.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

 Intersection #10 Brentwood Blvd. @ Grant Street / Sunset Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 1.730
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 180 Level Of Service: F

Street Name:	Brentwood Blvd.					Grant Street / Sunset Rd.						
Approach:	North Bound		South Bound			East Bound			West Bound			
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	4	10	10	4	10	10	4	4	4	4	4	4
Lanes:	1	0	1	0	1	1	0	0	1	0	0	1

Volume Module:

Base Vol:	30	1028	171	143	1026	19	26	75	34	175	60	170
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	30	1028	171	143	1026	19	26	75	34	175	60	170
Added Vol:	86	853	32	27	1096	94	44	2	61	24	2	16
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	116	1881	203	170	2122	113	70	77	95	199	62	186
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	116	1881	203	170	2122	113	70	77	95	199	62	186
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	116	1881	203	170	2122	113	70	77	95	199	62	186
RTOR Reduct:	0	0	199	0	0	0	0	0	0	0	0	170
RTOR Vol:	116	1881	4	170	2122	113	70	77	95	199	62	16
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	116	1881	4	170	2122	113	70	77	95	199	62	16

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	1.00	1.00	0.95	0.05	0.29	0.32	0.39	0.76	0.24	1.00
Final Sat.:	1650	1650	1650	1650	1567	83	477	525	648	1258	392	1650

Capacity Analysis Module:

Vol/Sat:	0.07	1.14	0.00	0.10	1.35	1.35	0.15	0.15	0.15	0.16	0.16	0.01
Crit Volume:	116				2235				242		261	
Crit Moves:	****				****				****		****	

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #11 Brentwood Blvd. @ Sunset Ct.

Average Delay (sec/veh): 9.2 Worst Case Level Of Service: F[668.4]

Table with columns for Street Name (Brentwood Blvd., Sunset Ct.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume, with values for each of the 12 movements.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim, with values for each of the 12 movements.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap, with values for each of the 12 movements.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS, with values for each of the 12 movements.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #12 Brentwood Blvd. @ Havenwood Ave.

Average Delay (sec/veh): 129314.5 Worst Case Level Of Service: F[xxxxx]

Table with columns for Street Name (Brentwood Blvd., Havenwood Ave.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 1, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #13 Brentwood Blvd. @ Applewood Common

Cycle (sec): 100 Critical Vol./Cap.(X): 1.653
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with columns for Street Name (Brentwood Blvd., Applewood Common), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #14 Brentwood Blvd. @ Sand Creek Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 1.278
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with columns for Street Name (Brentwood Blvd., Sand Creek Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #15 Brentwood Blvd. @ Technology Way

Cycle (sec): 100 Critical Vol./Cap.(X): 0.603
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 47 Level Of Service: B

Table with columns for Street Name (Brentwood Blvd., Technology Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #16 Brentwood Blvd @ Nancy Street (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.672
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 57 Level Of Service: B

Table with columns for Street Name (Brentwood Blvd, Nancy Street (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #17 Brentwood Blvd. @ Village Drive

Average Delay (sec/veh): 274.2 Worst Case Level Of Service: F[6239.4]

Table with columns for Street Name (Brentwood Blvd., Village Drive), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume for each approach and movement.

Critical Gap Module: Table showing Critical Gap and FollowUpTime for each approach and movement.

Capacity Module: Table showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap. for each approach and movement.

Level Of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS for each approach and movement.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #18 Brentwood Blvd. @ Central Blvd / Sycamore Ave (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.954
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: E

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Central Blvd / Sycamore Ave (S) with various movement details.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and rows for different approaches.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across different approaches.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across different approaches.

Level of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #20 Central Street @ Second Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.295
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 34 Level Of Service: A

Table with columns for Street Name (Second Street, Central Boulevard), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include, Ignore), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and rows for each.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat., and rows for each.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves, and rows for each.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #21 Central Blvd @ Walnut Blvd

Cycle (sec): 100 Critical Vol./Cap.(X): 0.305
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 36 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Central Blvd), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, Min. Green, and Lanes.

Volume Module: Table showing various volume metrics like Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table showing Sat/Lane, Adjustment, Lanes, and Final Sat. values.

Capacity Analysis Module: Table showing Vol/Sat, Crit Volume, and Crit Moves.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #22 Brentwood Blvd @ Pine Street

Average Delay (sec/veh): 0.6 Worst Case Level Of Service: E[41.3]

Table with columns for Street Name (Brentwood Blvd, Pine Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 0, 0, 0, 0, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across various movements.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across various movements.

Capacity Module: Table with columns for Cnflict Vol, Potent Cap., Move Cap., and Volume/Cap across various movements.

Level of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across various movements.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #23 Brentwood Blvd @ Maple Street

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: B[13.6]

Table with columns for Street Name (Brentwood Blvd, Maple Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, and Lanes.

Volume Module: Table showing traffic volume metrics such as Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table showing Critical Gap and FollowUpTim values for different approaches.

Capacity Module: Table showing Capacity metrics such as Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table showing Level of Service metrics such as 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

Intersection #24 Brentwood Blvd @ Oak Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.707
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 78 Level Of Service: C

Street Name: Brentwood Blvd. Oak St.

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

-----|-----|-----|-----|-----|

Control: Protected Protected Split Phase Split Phase

Rights: Include Include Include Include

Min. Green: 4 10 10 4 10 10 4 4 4 4 4 4 4

Lanes: 1 0 1 1 0 1 0 1 1 0 1 0 1 0 1

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Volume Module:

Base Vol:	103	611	89	30	620	185	276	193	65	82	181	26
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	611	89	30	620	185	276	193	65	82	181	26
Added Vol:	0	207	0	0	375	138	75	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	103	818	89	30	995	323	351	193	65	82	181	26
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	103	818	89	30	995	323	351	193	65	82	181	26
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	103	818	89	30	995	323	351	193	65	82	181	26
RTOR Reduct:	0	0	0	0	0	0	0	0	0	0	0	26
RTOR Vol:	103	818	89	30	995	323	351	193	65	82	181	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	103	818	89	30	995	323	351	193	65	82	181	0

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Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.80	0.20	1.00	1.51	0.49	1.73	0.95	0.32	1.00	1.00	1.00
Final Sat.:	1650	2976	324	1650	2491	809	2599	1564	526	1650	1650	1650

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Capacity Analysis Module:

Vol/Sat:	0.06	0.27	0.27	0.02	0.40	0.40	0.14	0.12	0.12	0.05	0.11	0.00
Crit Volume:	103					659	203				181	
Crit Moves:	****					****	****				****	

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

 Intersection #25 Walnut Blvd @ Oak St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.546
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 50 Level Of Service: A

Street Name: Walnut Blvd Oak St.

Approach:	North Bound					South Bound					East Bound			West Bound						
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Control:	Protected					Protected					Split Phase			Split Phase						
Rights:	Include					Include					Include			Include						
Min. Green:	5		7		7	0		7		7	5		5		5	5		5		5
Lanes:	1	0	1	0	1	1	0	0	1	0	0	0	1	0	0	0	1	0	0	1

Volume Module:

Base Vol:	12	286	360	152	156	10	4	6	6	297	9	188
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	12	286	360	152	156	10	4	6	6	297	9	188
Added Vol:	0	3	75	0	5	0	0	0	0	138	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	12	289	435	152	161	10	4	6	6	435	9	188
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	12	289	435	152	161	10	4	6	6	435	9	188
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	12	289	435	152	161	10	4	6	6	435	9	188
RTOR Reduct:	0	0	435	0	0	0	0	0	0	0	0	152
RTOR Vol:	12	289	0	152	161	10	4	6	6	435	9	36
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	12	289	0	152	161	10	4	6	6	435	9	36

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	1.00	1.00	0.94	0.06	0.25	0.37	0.38	0.98	0.02	1.00
Final Sat.:	1650	1650	1650	1650	1554	96	413	619	619	1617	33	1650

Capacity Analysis Module:

Vol/Sat:	0.01	0.18	0.00	0.09	0.10	0.10	0.01	0.01	0.01	0.27	0.27	0.02
Crit Volume:	289			152			16			444		
Crit Moves:	****			****			****			****		

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

 Intersection #26 Brentwood Blvd. @ Second St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.576
 Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 44 Level Of Service: A

Street Name: Brentwood Blvd. Second St.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	4	8	8	4	8	8	4	4	4	4	4	4
Lanes:	1	0	1	1	0	1	1	0	1	1	0	2

Volume Module:

Base Vol:	137	766	2	217	737	41	53	244	91	22	227	161
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	137	766	2	217	737	41	53	244	91	22	227	161
Added Vol:	19	263	0	0	485	0	0	0	28	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	156	1029	2	217	1222	41	53	244	119	22	227	161
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	156	1029	2	217	1222	41	53	244	119	22	227	161
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	156	1029	2	217	1222	41	53	244	119	22	227	161
RTOR Reduct:	0	0	0	0	0	0	0	0	0	0	0	161
RTOR Vol:	156	1029	2	217	1222	41	53	244	119	22	227	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	156	1029	2	217	1222	41	53	244	119	22	227	0

Saturation Flow Module:

Sat/Lane:	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.99	0.01	1.00	1.94	0.06	1.00	1.34	0.66	1.00	2.00	1.00
Final Sat.:	1720	3433	7	1720	3328	112	1720	2312	1128	1720	3440	1720

Capacity Analysis Module:

Vol/Sat:	0.09	0.30	0.30	0.13	0.37	0.37	0.03	0.11	0.11	0.01	0.07	0.00
Crit Volume:	156			632			182			22		
Crit Moves:	****			****			****			****		

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #27 Brentwood Blvd. @ Chestnut St.

Average Delay (sec/veh): 3.8 Worst Case Level Of Service: F[150.3]

Table with columns for Street Name (Brentwood Blvd., Chestnut St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across various movements.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across various movements.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across various movements.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across various movements.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #28 Brentwood Blvd. @ Balfour Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.902
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: E

Table with columns for Street Name (Brentwood Blvd., Balfour Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights (Ignore, Include), Min. Green (8, 10, 10, 6, 10, 10, 5, 5, 5, 4, 5, 5), and Lanes (1, 0, 2, 0, 1, 1, 0, 2, 0, 1, 1, 0, 1, 0, 1, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across 12 lanes.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across 12 lanes.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across 12 lanes.

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #29 Sellers Ave. @ Sunet Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.547

Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 12.5

Optimal Cycle: 0 Level Of Service: B

Street Name: Sellers Ave. Sunset Rd.

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

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Control: Stop Sign Stop Sign Stop Sign Stop Sign

Rights: Include Include Include Include

Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0

Lanes: 0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0

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Volume Module:

Base Vol: 94 236 14 21 175 39 50 91 69 7 77 30

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 94 236 14 21 175 39 50 91 69 7 77 30

Added Vol: 6 1 0 0 4 21 31 0 11 0 0 0

PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0

Initial Fut: 100 237 14 21 179 60 81 91 80 7 77 30

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Volume: 100 237 14 21 179 60 81 91 80 7 77 30

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0

Reduced Vol: 100 237 14 21 179 60 81 91 80 7 77 30

PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

FinalVolume: 100 237 14 21 179 60 81 91 80 7 77 30

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Saturation Flow Module:

Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Lanes: 0.28 0.68 0.04 0.08 0.69 0.23 0.32 0.36 0.32 0.06 0.68 0.26

Final Sat.: 183 434 26 51 435 146 193 216 190 34 369 144

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Capacity Analysis Module:

Vol/Sat: 0.55 0.55 0.55 0.41 0.41 0.41 0.42 0.42 0.42 0.21 0.21 0.21

Crit Moves: **** **** **** ****

Delay/Veh: 14.1 14.1 14.1 11.7 11.7 11.7 12.1 12.1 12.1 10.1 10.1 10.1

Delay Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

AdjDel/Veh: 14.1 14.1 14.1 11.7 11.7 11.7 12.1 12.1 12.1 10.1 10.1 10.1

LOS by Move: B B B B B B B B B B B B

ApproachDel: 14.1 11.7 12.1 10.1

Delay Adj: 1.00 1.00 1.00 1.00

ApprAdjDel: 14.1 11.7 12.1 10.1

LOS by Appr: B B B B

AllWayAvgQ: 1.0 1.0 1.0 0.6 0.6 0.6 0.6 0.6 0.6 0.2 0.2 0.2

Note: Queue reported is the number of cars per lane.

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	E Sims Rd. and Delta Rd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The diagram labels: Shoulder width (ft), Lane width (ft), Lane width (ft), Shoulder width (ft), and Segment length, L_i (mi).</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 2623 veh/h Directional split 55 / 45 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		2880	
v_p * highest directional split proportion ² (pc/h)		1584	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	40 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	40.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		0.9	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		16.8	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		2851	
v_p * highest directional split proportion ² (pc/h)		1568	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		91.8	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		2.1	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		93.9	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.90	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		214	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	787
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	12.7
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	E Sims Rd. and Delta Rd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP+PP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 3144 veh/h Directional split 51 / 49 Peak-hour factor, PHF 0.96 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		3308	
v_p * highest directional split proportion ² (pc/h)		1687	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	40 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	40.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)			
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$			
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		3275	
v_p * highest directional split proportion ² (pc/h)		1670	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		94.4	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		1.5	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		95.9	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)			
Volume to capacity ratio, $v/c=V_p/3,200$		1.03	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		246	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	943
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

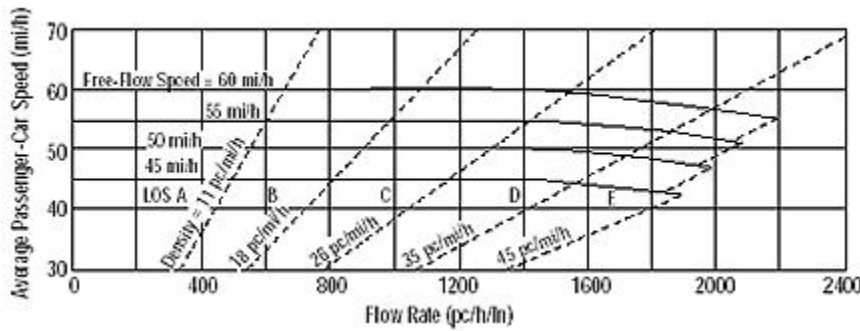
TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	Grant St. to Sunset Ct.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 3930 veh/h Directional split 55 / 45 Peak-hour factor, PHF 0.97 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 10	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		4092	
v_p * highest directional split proportion ² (pc/h)		2251	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)			
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$			
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		4052	
v_p * highest directional split proportion ² (pc/h)		2229	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		97.2	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		1.8	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		99.0	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)			
Volume to capacity ratio, $v/c=V_p/3,200$		1.28	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		101	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	393
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	Grant St. to Sunset Ct.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 4666 veh/h Directional split 53 / 47 Peak-hour factor, PHF 0.90 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 10	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		5236	
v_p * highest directional split proportion ² (pc/h)		2775	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)			
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$			
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		5184	
v_p * highest directional split proportion ² (pc/h)		2748	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		99.0	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		1.6	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		100.6	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)			
Volume to capacity ratio, $v/c=V_p/3,200$		1.64	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		130	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	467
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Central Blvd. to Spruce S
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	1167	Peak-Hour Factor, PHF	0.90
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

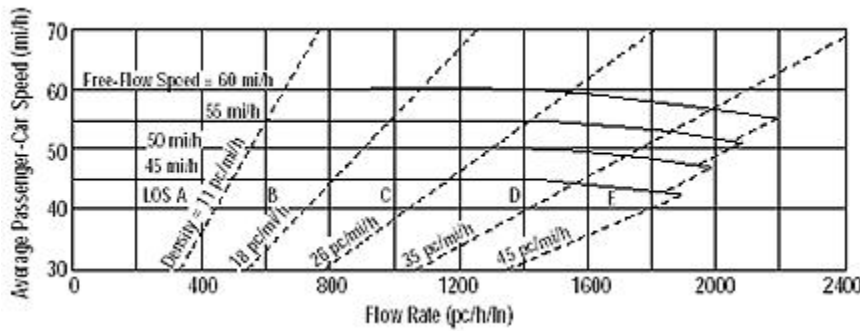
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 680
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 15.1
 LOS: B

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Brentwood Blvd.
Agency or Company		From/To	Central Blvd. to Spruce S
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP+PP

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	1012	Peak-Hour Factor, PHF	0.95
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

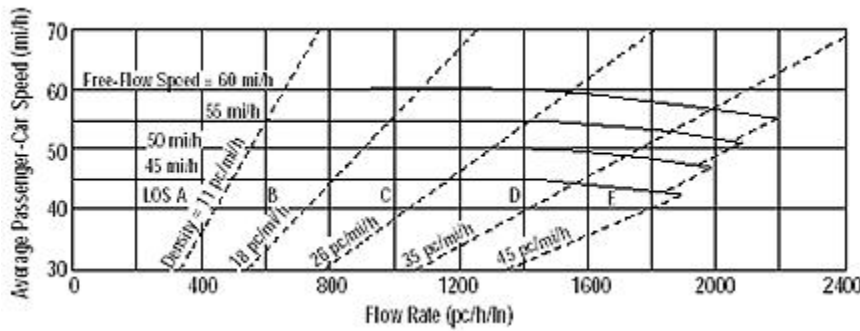
Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	559	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	12.4	Max Service Flow Rate (pc/h/ln)	
LOS	B	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Central Blvd. to Spruce S
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	1248	Peak-Hour Factor, PHF	0.78
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

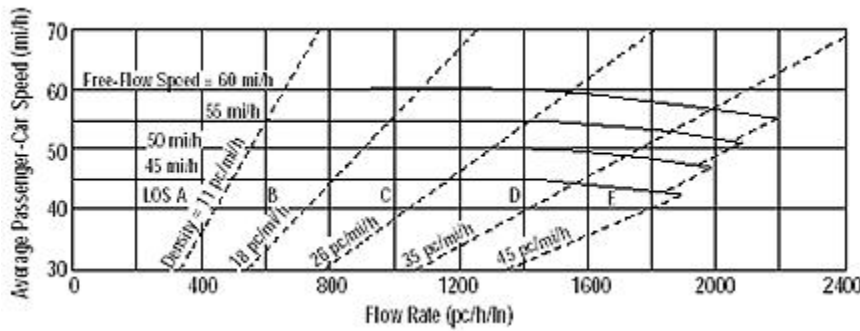
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 840
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 18.7
 LOS: C

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Central Blvd. to Spruce S
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	1480	Peak-Hour Factor, PHF	0.99
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

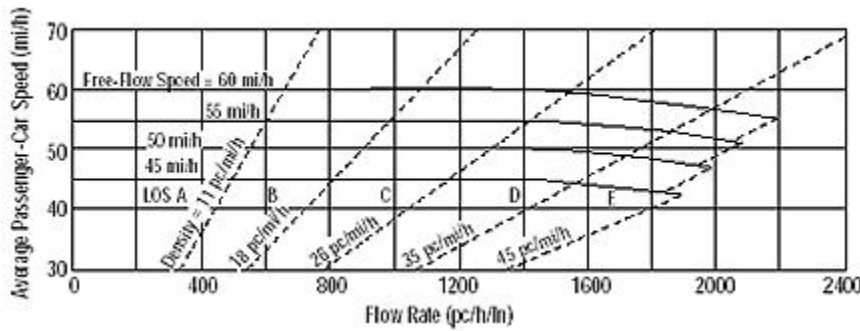
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 784
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 17.4
 LOS: B

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Pine Ave. to Oak Ave.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	1144	Peak-Hour Factor, PHF	0.90
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

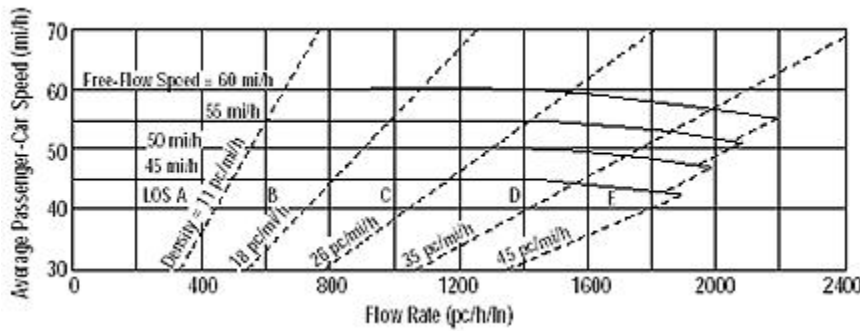
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 667
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 14.8
 LOS: B

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Pine Ave. to Oak Ave.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	876	Peak-Hour Factor, PHF	0.88
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

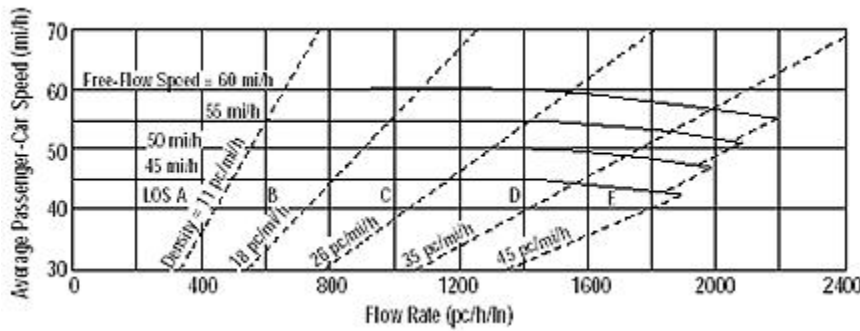
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 522
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 11.6
 LOS: B

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Pine Ave. to Oak Ave.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	1186	Peak-Hour Factor, PHF	0.97
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

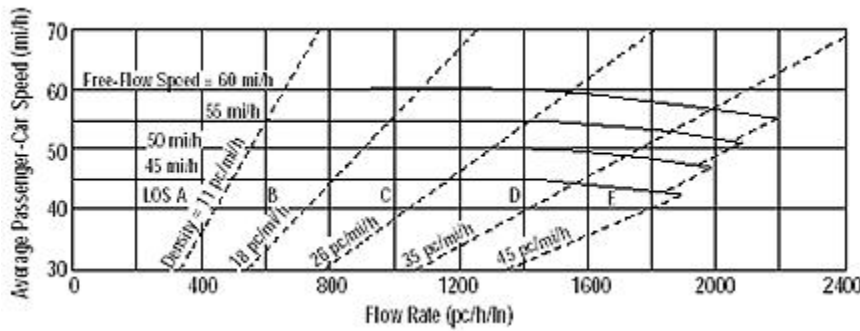
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 641
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 14.2
 LOS: B

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Brentwood Blvd.
Agency or Company		From/To	Pine Ave. to Oak Ave.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP+PP

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	1353	Peak-Hour Factor, PHF	0.97
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	732	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	16.3	Max Service Flow Rate (pc/h/ln)	
LOS	B	Design LOS	

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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Delta Rd.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP+PP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 505 veh/h Directional split 54 / 46 Peak-hour factor, PHF 0.76 No-passing zone 0 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 40	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		667	
v_p * highest directional split proportion ² (pc/h)		360	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	50 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	50.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		0.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		44.8	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		666	
v_p * highest directional split proportion ² (pc/h)		360	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		44.3	
Adj. for directional distribution and no-passing zone, $f_{dnp}(\%)(Exh. 20-12)$		0.0	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{dnp}$		44.3	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		B	
Volume to capacity ratio, $v/c=V_p/3,200$		0.21	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		166	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	505
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	3.7
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Delta Rd.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP+PP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 810 veh/h Directional split 73 / 27 Peak-hour factor, PHF 0.91 No-passing zone 0 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 40	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		894	
v_p * highest directional split proportion ² (pc/h)		653	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	50 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	50.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		0.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		43.1	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		892	
v_p * highest directional split proportion ² (pc/h)		651	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		54.3	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		0.0	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		54.3	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		B	
Volume to capacity ratio, $v/c=V_p/3,200$		0.28	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		223	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	810
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	5.2
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Lone Tree Wy.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1774 veh/h Directional split 53 / 47 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 30	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1932	
v_p * highest directional split proportion ² (pc/h)		1024	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	35 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	35.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		1.2	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		18.8	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1928	
v_p * highest directional split proportion ² (pc/h)		1022	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		81.6	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(\text{Exh. 20-12})$		4.8	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		86.4	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.60	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		482	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	1774
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	25.6
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Lone Tree Wy.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP+PP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1763 veh/h Directional split 59 / 41 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 30	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1920	
v_p * highest directional split proportion ² (pc/h)		1133	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	35 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	35.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		1.2	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		18.9	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1916	
v_p * highest directional split proportion ² (pc/h)		1130	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		81.4	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		4.8	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		86.3	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.60	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		479	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	1763
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	25.3
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

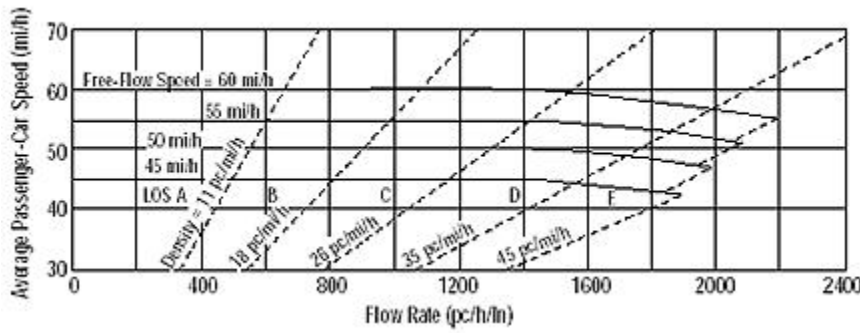
TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sunset Ave
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 778 veh/h Directional split 67 / 33 Peak-hour factor, PHF 0.88 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		888	
v_p * highest directional split proportion ² (pc/h)		595	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		2.8	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		20.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		886	
v_p * highest directional split proportion ² (pc/h)		594	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		54.1	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(\text{Exh. 20-12})$		13.6	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		67.7	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.28	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		221	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	778
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	10.9
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sunset Ave
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 889 veh/h Directional split 50 / 50 Peak-hour factor, PHF 0.88 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1014	
v_p * highest directional split proportion ² (pc/h)		507	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		2.6	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		19.6	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1012	
v_p * highest directional split proportion ² (pc/h)		506	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		58.9	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(\text{Exh. 20-12})$		12.8	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		71.7	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		D	
Volume to capacity ratio, $v/c=V_p/3,200$		0.32	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		253	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	889
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	12.9
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v _p	L
Design (N)	FFS, LOS, v _p	N
Design (v _p)	FFS, LOS, N	v _i
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v _p)	FFS, LOS, N	v _i

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Central Blvd.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP+PP

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	363	Peak-Hour Factor, PHF	0.86
AADT(veh/h)		%Trucks and Buses, P _T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

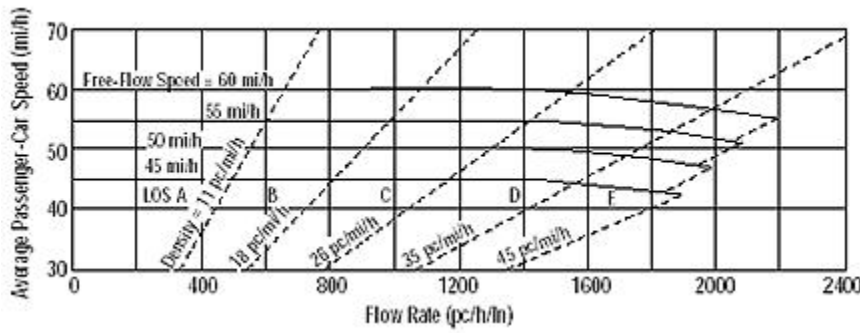
Calculate Flow Adjustments			
f _p	1.00	E _R	1.2
E _T	1.5	f _{HV}	0.990

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f _{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f _{LC} (mi/h)	
Access Points, A (A/mi)	0	f _A (mi/h)	
Median Type, M		f _M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v _p (pc/h/ln)	213	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v _p (pc/h)	
D (pc/mi/ln)	4.7	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v _p	L
Design (N)	FFS, LOS, v _p	N
Design (v _p)	FFS, LOS, N	v _i
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v _p)	FFS, LOS, N	v _i

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Central Blvd.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	420	Peak-Hour Factor, PHF	0.88
AADT(veh/h)		%Trucks and Buses, P _T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f _p	1.00	E _R	1.2
E _T	1.5	f _{HV}	0.990

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

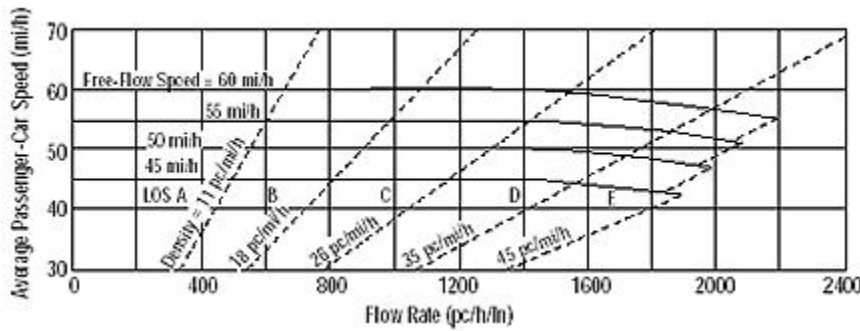
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 241
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 5.4
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Central Blvd.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	405	Peak-Hour Factor, PHF	0.84
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

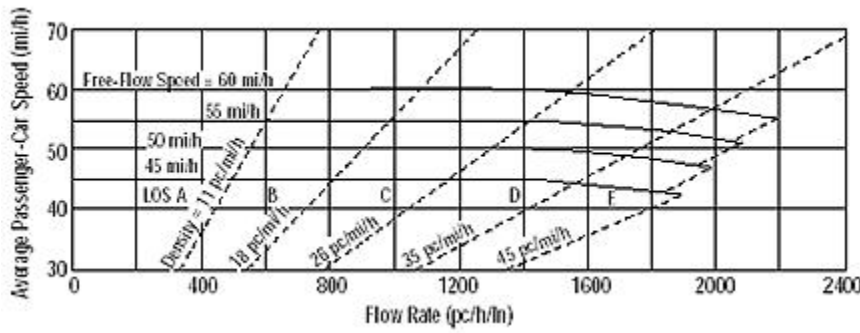
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 243
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 5.4
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Central Blvd.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP+PP

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	563	Peak-Hour Factor, PHF	0.76
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	374	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	8.3	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Grant St.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP+PP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram labels the following dimensions: Shoulder width (ft), Lane width (ft), Lane width (ft), and Shoulder width (ft). Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 547 veh/h Directional split 55 / 45 Peak-hour factor, PHF 0.71 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 7	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		774	
v_p * highest directional split proportion ² (pc/h)		426	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.1	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		20.9	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		772	
v_p * highest directional split proportion ² (pc/h)		425	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		49.3	
Adj. for directional distribution and no-passing zone, $f_{dnp}(\%)(Exh. 20-12)$		15.7	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{dnp}$		65.0	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.24	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		193	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	547
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	9.2
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Grant St.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP+PP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 525 veh/h Directional split 55 / 45 Peak-hour factor, PHF 0.87 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 7	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		606	
v_p * highest directional split proportion ² (pc/h)		333	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.9	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		21.4	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		605	
v_p * highest directional split proportion ² (pc/h)		333	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		41.2	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		20.5	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		61.7	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.19	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		151	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	525
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	7.0
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sycamore Ave.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 689 veh/h Directional split 58 / 42 Peak-hour factor, PHF 0.87 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 35	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		795	
v_p * highest directional split proportion ² (pc/h)		461	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		20.8	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		794	
v_p * highest directional split proportion ² (pc/h)		461	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		50.2	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		14.8	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		65.0	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.25	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		99	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	345
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	4.8
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sycamore Ave.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Existing+AP+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 741 veh/h Directional split 52 / 48 Peak-hour factor, PHF 0.82 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 35	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		907	
v_p * highest directional split proportion ² (pc/h)		472	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		2.8	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		20.2	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		905	
v_p * highest directional split proportion ² (pc/h)		471	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		54.9	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(\text{Exh. 20-12})$		13.9	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		68.8	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.28	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		113	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	371
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	5.6
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

Appendix D:

*Analysis Worksheets for
Cumulative Conditions*

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 Brentwood Blvd. @ Delta Road

Average Delay (sec/veh): 260.7 Worst Case Level Of Service: F[1014.1]

Table with columns for Street Name (Brentwood Blvd., Delta Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across 12 lanes.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across 12 lanes.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across 12 lanes.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across 12 lanes.

Note: Queue reported is the number of cars per lane.

 Level Of Service Computation Report
 2000 HCM 4-Way Stop Method (Base Volume Alternative)

 Intersection #2 Sellers Ave @ Delta

Cycle (sec): 100 Critical Vol./Cap.(X): 0.506
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 12.0
 Optimal Cycle: 0 Level Of Service: B

Street Name: Sellers Ave. Delta Rd.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0

Volume Module:

Base Vol:	45	127	70	6	99	18	22	129	102	145	162	16
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	45	127	70	6	99	18	22	129	102	145	162	16
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	45	127	70	6	99	18	22	129	102	145	162	16
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	45	127	70	6	99	18	22	129	102	145	162	16
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	45	127	70	6	99	18	22	129	102	145	162	16

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.19	0.52	0.29	0.05	0.80	0.15	0.09	0.51	0.40	0.45	0.50	0.05
Final Sat.:	111	313	172	27	441	80	56	330	261	287	320	32

Capacity Analysis Module:

Vol/Sat:	0.41	0.41	0.41	0.22	0.22	0.22	0.39	0.39	0.39	0.51	0.51	0.51
Crit Moves:	****			****			****			****		
Delay/Veh:	12.0	12.0	12.0	10.4	10.4	10.4	11.2	11.2	11.2	13.3	13.3	13.3
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	12.0	12.0	12.0	10.4	10.4	10.4	11.2	11.2	11.2	13.3	13.3	13.3
LOS by Move:	B	B	B	B	B	B	B	B	B	B	B	B
ApproachDel:	12.0			10.4			11.2			13.3		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	12.0			10.4			11.2			13.3		
LOS by Appr:	B			B			B			B		
AllWayAvgQ:	0.6	0.6	0.6	0.2	0.2	0.2	0.5	0.5	0.5	0.9	0.9	0.9

 Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #3 Brentwood Blvd. @ E. Sims Road

Average Delay (sec/veh): 1.4 Worst Case Level Of Service: E[35.4]

Table with columns for Street Name (Brentwood Blvd., E. Sims Road), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 0, 0, 0, 0, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #4 Brentwood Blvd @ Lone Tree Way (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.854
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 156 Level Of Service: D

Table with columns for Street Name (Brentwood Blvd, Lone Tree Way (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include, Ovl), Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #5 Arroyo Seco Road @ Lone Tree Way

Average Delay (sec/veh): 3.1 Worst Case Level Of Service: F[3605.0]

Table with columns for Street Name (Arroyo Seco Road, Lone Tree Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Stop Sign, Uncontrolled), Rights (Include), and Lanes (1 0 0 1 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS, and values for each approach.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #6 Brentwood Blvd. @ Sunrise Drive

Average Delay (sec/veh): 2.2 Worst Case Level Of Service: F[114.8]

Table with columns for Street Name (Brentwood Blvd., Sunrise Dr.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #7 Brentwood Blvd. @ Gregory Lane

Average Delay (sec/veh): 2.0 Worst Case Level Of Service: F[107.4]

Table with columns for Street Name (Brentwood Blvd., Gregory Ln.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach and movement.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach and movement.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach and movement.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #8 Brentwood Blvd. @ Hanson Lane

Average Delay (sec/veh): 3.9 Worst Case Level Of Service: F[139.1]

Table with columns for Street Name (Brentwood Blvd., Hanson Ln), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for Brentwood Blvd. and Hanson Ln.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for Brentwood Blvd. and Hanson Ln.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for Brentwood Blvd. and Hanson Ln.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS, and values for Brentwood Blvd. and Hanson Ln.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #9 Brentwood Blvd. @ Homecoming Way

Average Delay (sec/veh): 1.9 Worst Case Level Of Service: F[95.8]

Table with columns for Street Name (Brentwood Blvd., Homecoming Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for Brentwood Blvd. and Homecoming Way.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for Brentwood Blvd. and Homecoming Way.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for Brentwood Blvd. and Homecoming Way.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS, and values for Brentwood Blvd. and Homecoming Way.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #10 Brentwood Blvd. @ Grant Street / Sunset Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.853
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 155 Level Of Service: D

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Grant Street / Sunset Rd. with sub-rows for North, South, East, and West bounds.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume. Rows list various adjustment factors and resulting volumes.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. Rows show saturation flow values and adjustments for different lane configurations.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves. Rows show capacity analysis results for different lane configurations.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #11 Brentwood Blvd. @ Sunset Ct.

Average Delay (sec/veh): 1.1 Worst Case Level Of Service: C[21.3]

Street Name: Brentwood Blvd. Sunset Ct.

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume. Rows include various volume and adjustment factors.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim. Rows include gap and follow-up time values.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap. Rows include capacity and volume-related metrics.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS. Rows include level of service and delay metrics.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #12 Brentwood Blvd. @ Havenwood Ave.

Average Delay (sec/veh): 1.1 Worst Case Level Of Service: E[45.7]

Table with columns for Street Name (Brentwood Blvd., Havenwood Ave.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 1, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across various approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across various approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across various approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across various approaches.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #13 Brentwood Blvd. @ Applewood Common

Cycle (sec): 100 Critical Vol./Cap.(X): 0.735
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 86 Level Of Service: C

Table with columns for Street Name (Brentwood Blvd., Applewood Common), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green (4, 8, 8), and Lanes (1, 0, 1, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across various movement categories.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across various movement categories.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #14 Brentwood Blvd. @ Sand Creek Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.504
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 38 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Sand Creek Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green (4, 10, 10), and Lanes (1, 0, 1, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across 12 lanes.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across 12 lanes.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across 12 lanes.

Level of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #15 Brentwood Blvd. @ Technology Way

Cycle (sec): 100 Critical Vol./Cap.(X): 0.285
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 26 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Technology Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green (4, 6, 6, 0, 6, 6, 4, 4, 4, 0, 0, 0), and Lanes (1, 0, 2, 0, 0, 0, 0, 1, 1, 0, 1, 0, 0, 0, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across 12 lanes.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across 12 lanes.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across 12 lanes.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #16 Brentwood Blvd @ Nancy Street (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.387
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 30 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Nancy Street (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #17 Brentwood Blvd. @ Village Drive

Average Delay (sec/veh): 1.3 Worst Case Level Of Service: E[39.3]

Table with columns for Street Name (Brentwood Blvd., Village Drive), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume for each approach.

Critical Gap Module: Table showing Critical Gp and FollowUpTim for each approach.

Capacity Module: Table showing Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap for each approach.

Level Of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS for each approach.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #18 Brentwood Blvd. @ Central Blvd / Sycamore Ave (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.348
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 32 Level Of Service: A

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Central Blvd / Sycamore Ave (S) with various movement details.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #20 Central Street @ Second Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.508
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 46 Level Of Service: A

Table with columns for Street Name (Second Street, Central Boulevard), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include, Ignore), Rights, Min. Green, and Lanes.

Volume Module: Table showing traffic volume adjustments including Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table showing Sat/Lane, Adjustment, Lanes, and Final Sat values for different movements.

Capacity Analysis Module: Table showing Vol/Sat, Crit Volume, and Crit Moves for different movements.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #21 Central Blvd @ Walnut Blvd

Cycle (sec): 100 Critical Vol./Cap.(X): 0.471
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 43 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Central Blvd), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Table for Volume Module showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various approaches.

Table for Saturation Flow Module showing Sat/Lane, Adjustment, Lanes, and Final Sat. across various approaches.

Table for Capacity Analysis Module showing Vol/Sat, Crit Volume, and Crit Moves across various approaches.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #22 Brentwood Blvd @ Pine Street

Average Delay (sec/veh): 0.2 Worst Case Level Of Service: A[9.6]

Table with columns for Street Name (Brentwood Blvd, Pine Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, and Lanes.

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume for each approach.

Critical Gap Module: Table showing Critical Gap and FollowUp Time for each approach.

Capacity Module: Table showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap for each approach.

Level Of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, Approach Del, and Approach LOS for each approach.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #23 Brentwood Blvd @ Maple Street

Average Delay (sec/veh): 5.5 Worst Case Level Of Service: B[12.0]

Table with columns for Street Name (Brentwood Blvd, Maple Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across movements.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across movements.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across movements.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across movements.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #24 Brentwood Blvd @ Oak Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.171
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 34 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Oak St), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
 CCTALOS Method (Base Volume Alternative)

 Intersection #25 Walnut Blvd @ Oak St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.521
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 48 Level Of Service: A

Street Name: Walnut Blvd						Oak St.					
Approach: North Bound			South Bound			East Bound			West Bound		
Movement: L - T - R			L - T - R			L - T - R			L - T - R		
Control: Protected			Protected			Split Phase			Split Phase		
Rights: Include			Include			Include			Include		
Min. Green:	5	7	7	0	7	7	5	5	5	5	5
Lanes:	1	0	1	0	1	0	0	1	0	0	1

Volume Module:

Base Vol:	6	268	578	277	195	4	1	1	0	107	2	46
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	6	268	578	277	195	4	1	1	0	107	2	46
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	6	268	578	277	195	4	1	1	0	107	2	46
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	6	268	578	277	195	4	1	1	0	107	2	46
RTOR Reduct:	0	0	107	0	0	0	0	0	0	0	0	46
RTOR Vol:	6	268	471	277	195	4	1	1	0	107	2	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	6	268	471	277	195	4	1	1	0	107	2	0

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	1.00	1.00	0.98	0.02	0.50	0.50	0.00	0.98	0.02	1.00
Final Sat.:	1650	1650	1650	1650	1617	33	825	825	0	1620	30	1650

Capacity Analysis Module:

Vol/Sat:	0.00	0.16	0.29	0.17	0.12	0.12	0.00	0.00	0.00	0.07	0.07	0.00
Crit Volume:			471	277				2			109	
Crit Moves:			****	****				****			****	

 Level Of Service Computation Report
 CCTALOS Method (Base Volume Alternative)

 Intersection #26 Brentwood Blvd. @ Second St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.217
 Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 25 Level Of Service: A

Street Name:	Brentwood Blvd.						Second St.								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected			Permitted			Permitted					
Rights:	Include			Include			Include			Include					
Min. Green:	4	8	8	4	8	8	4	4	4	4	4	4			
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0	2	0	1

Volume Module:

Base Vol:	57	221	12	107	230	12	33	115	19	47	235	172
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	57	221	12	107	230	12	33	115	19	47	235	172
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	57	221	12	107	230	12	33	115	19	47	235	172
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	57	221	12	107	230	12	33	115	19	47	235	172
RTOR Reduct:	0	0	0	0	0	0	0	0	0	0	0	107
RTOR Vol:	57	221	12	107	230	12	33	115	19	47	235	65
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	57	221	12	107	230	12	33	115	19	47	235	65

Saturation Flow Module:

Sat/Lane:	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.90	0.10	1.00	1.90	0.10	1.00	1.72	0.28	1.00	2.00	1.00
Final Sat.:	1720	3263	177	1720	3269	171	1720	2952	488	1720	3440	1720

Capacity Analysis Module:

Vol/Sat:	0.03	0.07	0.07	0.06	0.07	0.07	0.02	0.04	0.04	0.03	0.07	0.04
Crit Volume:	116			107			33			118		
Crit Moves:	****			****			****			****		

Level of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #27 Brentwood Blvd. @ Chestnut St.

Average Delay (sec/veh): 0.6 Worst Case Level Of Service: B[10.3]

Street Name:	Brentwood Blvd.						Chestnut St.																	
Approach:	North Bound			South Bound			East Bound			West Bound														
Movement:	L	T	R	L	T	R	L	T	R	L	T	R												
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign														
Rights:	Include			Include			Include			Include														
Lanes:	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0

Volume Module:

Base Vol:	0	273	19	12	108	0	0	0	0	0	5	0	11
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	273	19	12	108	0	0	0	0	0	5	0	11
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	273	19	12	108	0	0	0	0	0	5	0	11
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	273	19	12	108	0	0	0	0	0	5	0	11

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	6.5	6.2
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	292	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	415	415	283
Potent Cap.:	xxxx	xxxx	xxxxxx	1225	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	594	528	756
Move Cap.:	xxxx	xxxx	xxxxxx	1225	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	590	523	756
Volume/Cap:	xxxx	xxxx	xxxx	0.01	xxxx	xxxx	xxxx	xxxx	xxxx	0.01	0.00	0.01

Level of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.0	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	xxxxxx	xxxx	xxxxxx	8.0	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	*	*	*	A	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	695	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	0.0	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	0.1	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	8.0	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	10.3	xxxxxx
Shared LOS:	*	*	*	A	*	*	*	*	*	*	B	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			10.3		
ApproachLOS:	*			*			*			B		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 CCTALOS Method (Base Volume Alternative)

 Intersection #28 Brentwood Blvd. @ Balfour Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.343
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 40 Level Of Service: A

Street Name:	Brentwood Blvd.						Balfour Rd.								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected			Protected			Protected					
Rights:	Ignore			Include			Include			Include					
Min. Green:	8	10	10	6	10	10	5	5	5	4	5	5			
Lanes:	1	0	2	0	1	1	0	2	0	1	1	0	1	1	0

Volume Module:

Base Vol:	217	298	13	7	58	45	124	170	161	30	343	49
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	217	298	13	7	58	45	124	170	161	30	343	49
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	217	298	13	7	58	45	124	170	161	30	343	49
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	217	298	13	7	58	45	124	170	161	30	343	49
RTOR Reduct:	0	0	0	0	0	45	0	0	161	0	0	0
RTOR Vol:	217	298	13	7	58	0	124	170	0	30	343	49
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	217	298	13	7	58	0	124	170	0	30	343	49

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	1.00	1.00	1.00	1.75	0.25
Final Sat.:	1650	3300	1650	1650	3300	1650	1650	1650	1650	1650	2888	413

Capacity Analysis Module:

Vol/Sat:	0.13	0.09	0.01	0.00	0.02	0.00	0.08	0.10	0.00	0.02	0.12	0.12
Crit Volume:	217				29		124			196		
Crit Moves:	****				****		****			****		

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #29 Sellers Ave. @ Sunet Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.731

Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 15.7

Optimal Cycle: 0 Level Of Service: C

Street Name: Sellers Ave. Sunset Rd.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0

Volume Module:

Base Vol:	145	328	15	10	217	81	49	65	71	18	61	23
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	145	328	15	10	217	81	49	65	71	18	61	23
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	145	328	15	10	217	81	49	65	71	18	61	23
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	145	328	15	10	217	81	49	65	71	18	61	23
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	145	328	15	10	217	81	49	65	71	18	61	23

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.30	0.67	0.03	0.03	0.71	0.26	0.26	0.35	0.39	0.18	0.60	0.22
Final Sat.:	198	449	21	21	453	169	145	192	210	88	300	113

Capacity Analysis Module:

Vol/Sat:	0.73	0.73	0.73	0.48	0.48	0.48	0.34	0.34	0.34	0.20	0.20	0.20
Crit Moves:	****			****			****			****		
Delay/Veh:	20.3	20.3	20.3	12.7	12.7	12.7	11.5	11.5	11.5	10.5	10.5	10.5
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	20.3	20.3	20.3	12.7	12.7	12.7	11.5	11.5	11.5	10.5	10.5	10.5
LOS by Move:	C	C	C	B	B	B	B	B	B	B	B	B
ApproachDel:	20.3			12.7			11.5			10.5		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	20.3			12.7			11.5			10.5		
LOS by Appr:	C			B			B			B		
AllWayAvgQ:	2.2	2.2	2.2	0.8	0.8	0.8	0.4	0.4	0.4	0.2	0.2	0.2

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 Brentwood Blvd. @ Delta Road

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Table with columns for Street Name (Brentwood Blvd., Delta Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #2 Sellers Ave @ Delta

Cycle (sec): 100 Critical Vol./Cap.(X): 0.872
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 26.5
Optimal Cycle: 0 Level Of Service: D

Street Name: Sellers Ave. Delta Rd.
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Stop Sign Stop Sign Stop Sign Stop Sign
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0

Volume Module:
Base Vol: 108 278 50 4 49 5 96 289 155 49 111 35
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 108 278 50 4 49 5 96 289 155 49 111 35
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 108 278 50 4 49 5 96 289 155 49 111 35
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 108 278 50 4 49 5 96 289 155 49 111 35
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 108 278 50 4 49 5 96 289 155 49 111 35

Saturation Flow Module:
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.25 0.64 0.11 0.07 0.84 0.09 0.18 0.53 0.29 0.25 0.57 0.18
Final Sat.: 139 358 64 31 377 38 110 332 178 131 297 94

Capacity Analysis Module:
Vol/Sat: 0.78 0.78 0.78 0.13 0.13 0.13 0.87 0.87 0.87 0.37 0.37 0.37
Crit Moves: **** **** ****
Delay/Veh: 25.7 25.7 25.7 11.0 11.0 11.0 33.8 33.8 33.8 12.7 12.7 12.7
Delay Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
AdjDel/Veh: 25.7 25.7 25.7 11.0 11.0 11.0 33.8 33.8 33.8 12.7 12.7 12.7
LOS by Move: D D D B B B D D D B B B
ApproachDel: 25.7 11.0 33.8 12.7
Delay Adj: 1.00 1.00 1.00
ApprAdjDel: 25.7 11.0 33.8 12.7
LOS by Appr: D B D B
AllWayAvgQ: 2.6 2.6 2.6 0.1 0.1 0.1 4.3 4.3 4.3 0.5 0.5 0.5

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #3 Brentwood Blvd. @ E. Sims Road

Average Delay (sec/veh): 9.8 Worst Case Level Of Service: F[129.1]

Table with columns for Street Name (Brentwood Blvd., E. Sims Road), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 0, 0, 0, 0, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS, and values for each approach.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #4 Brentwood Blvd @ Lone Tree Way (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 1.152
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd and Lone Tree Way (S) with various traffic movement details.

Volume Module: Table showing traffic volume adjustments including Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table showing saturation flow values for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table showing capacity analysis values for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #5 Arroyo Seco Road @ Lone Tree Way

Average Delay (sec/veh): 4.1 Worst Case Level Of Service: F[3605.0]

Table with columns for Street Name, Approach, Movement, Control, Rights, and Lanes. Rows include Arroyo Seco Road and Lone Tree Way with various movement details.

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume for various movements.

Critical Gap Module: Table showing Critical Gp and FollowUpTim for various movements.

Capacity Module: Table showing Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap for various movements.

Level Of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #6 Brentwood Blvd. @ Sunrise Drive

Average Delay (sec/veh): 80.8 Worst Case Level Of Service: F[1909.2]

Street Name: Brentwood Blvd. Sunrise Dr.

Table with columns for Approach, Movement, Control, Rights, Lanes for North Bound, South Bound, East Bound, and West Bound.

Volume Module: Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume

Critical Gap Module: Critical Gp, FollowUpTim

Capacity Module: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #7 Brentwood Blvd. @ Gregory Lane

Average Delay (sec/veh): 70.1 Worst Case Level Of Service: F[1658.2]

Street Name: Brentwood Blvd. Gregory Ln.

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #8 Brentwood Blvd. @ Hanson Lane

Average Delay (sec/veh): 117.1 Worst Case Level Of Service: F[1851.9]

Table with columns for Street Name (Brentwood Blvd., Hanson Ln), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflict Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #9 Brentwood Blvd. @ Homecoming Way

Average Delay (sec/veh): 2.7 Worst Case Level Of Service: F[368.8]

Table with columns for Street Name (Brentwood Blvd., Homecoming Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0 0 0 1 0, 1 0 1 0 0, 0 0 0 0 0, 0 0 1! 0 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for Brentwood Blvd. and Homecoming Way.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for Brentwood Blvd. and Homecoming Way.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for Brentwood Blvd. and Homecoming Way.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS, and values for Brentwood Blvd. and Homecoming Way.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #10 Brentwood Blvd. @ Grant Street / Sunset Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 1.110
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Street Name: Brentwood Blvd. Grant Street / Sunset Rd.
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Protected Protected Split Phase Split Phase
Rights: Include Include Include Include
Min. Green: 4 10 10 4 10 10 4 4 4 4 4 4
Lanes: 1 0 1 0 1 1 0 0 1 0 0 0 1! 0 0 0 1 0 0 1

Volume Module:
Base Vol: 37 974 100 137 1409 37 16 47 21 176 88 186
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 37 974 100 137 1409 37 16 47 21 176 88 186
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 37 974 100 137 1409 37 16 47 21 176 88 186
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 37 974 100 137 1409 37 16 47 21 176 88 186
RTOR Reduct: 0 0 100 0 0 0 0 0 0 0 0 137
RTOR Vol: 37 974 0 137 1409 37 16 47 21 176 88 49
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 37 974 0 137 1409 37 16 47 21 176 88 49

Saturation Flow Module:
Sat/Lane: 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 1.00 1.00 1.00 0.97 0.03 0.19 0.56 0.25 0.67 0.33 1.00
Final Sat.: 1650 1650 1650 1650 1608 42 314 923 413 1100 550 1650

Capacity Analysis Module:
Vol/Sat: 0.02 0.59 0.00 0.08 0.88 0.88 0.05 0.05 0.05 0.16 0.16 0.03
Crit Volume: 37 1446 84 264
Crit Moves: **** **** **** ****

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #11 Brentwood Blvd. @ Sunset Ct.

Average Delay (sec/veh): 13.8 Worst Case Level Of Service: F[150.1]

Table with columns for Street Name (Brentwood Blvd., Sunset Ct.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume, with values for each approach and movement.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim, with values for each approach and movement.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap, with values for each approach and movement.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS, with values for each approach and movement.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #12 Brentwood Blvd. @ Havenwood Ave.

Average Delay (sec/veh): 22.7 Worst Case Level Of Service: F[1262.0]

Table with columns for Street Name (Brentwood Blvd., Havenwood Ave.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 1, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #13 Brentwood Blvd. @ Applewood Common

Cycle (sec): 100 Critical Vol./Cap.(X): 0.918
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: E

Table with columns for Street Name (Brentwood Blvd., Applewood Common), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green (4, 8, 8), and Lanes (1, 0, 1, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across various movement categories.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across various movement categories.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #14 Brentwood Blvd. @ Sand Creek Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.843
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 119 Level Of Service: D

Table with columns for Street Name (Brentwood Blvd., Sand Creek Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green (4, 10, 10), and Lanes (1, 0, 1, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various traffic scenarios.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across various traffic scenarios.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across various traffic scenarios.

Level of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #15 Brentwood Blvd. @ Technology Way

Cycle (sec): 100 Critical Vol./Cap.(X): 0.462
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 35 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Technology Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #16 Brentwood Blvd @ Nancy Street (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.480
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 36 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Nancy Street (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #17 Brentwood Blvd. @ Village Drive

Average Delay (sec/veh): 17.3 Worst Case Level Of Service: F[510.6]

Table with columns for Street Name (Brentwood Blvd., Village Drive), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #18 Brentwood Blvd. @ Central Blvd / Sycamore Ave (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.572
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 43 Level Of Service: A

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Central Blvd / Sycamore Ave (S) with various movement details.

Volume Module: Table showing traffic volume adjustments including Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table showing saturation flow values for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table showing capacity analysis values for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #20 Central Street @ Second Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.505
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 46 Level Of Service: A

Table with columns for Street Name (Second Street, Central Boulevard), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include, Ignore), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #21 Central Blvd @ Walnut Blvd

Cycle (sec): 100 Critical Vol./Cap.(X): 0.535
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 49 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Central Blvd), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #22 Brentwood Blvd @ Pine Street

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: A[10.0]

Table with columns for Street Name (Brentwood Blvd, Pine Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #23 Brentwood Blvd @ Maple Street

Average Delay (sec/veh): 4.5 Worst Case Level Of Service: B[13.1]

Table with columns for Street Name (Brentwood Blvd, Maple Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across various approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across various approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across various approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across various approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #24 Brentwood Blvd @ Oak Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.266
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 34 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Oak St), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green (4, 10, 10), and Lanes (1, 0, 1, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across various movement categories.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across various movement categories.

Level Of Service Computation Report
 CCTALOS Method (Base Volume Alternative)

 Intersection #25 Walnut Blvd @ Oak St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.511
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 47 Level Of Service: A

Street Name: Walnut Blvd					Oak St.								
Approach:		North Bound			South Bound			East Bound		West Bound			
Movement:		L	T	R	L	T	R	L	T	R			
Control:		Protected			Protected			Split Phase		Split Phase			
Rights:		Include			Include			Include		Include			
Min. Green:		5	7	7	0	7	7	5	5	5	5		
Lanes:		1	0	1	0	1	0	0	0	1	0	0	1

Volume Module:

Base Vol:	11	255	297	355	398	27	4	6	6	210	7	131
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	11	255	297	355	398	27	4	6	6	210	7	131
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	11	255	297	355	398	27	4	6	6	210	7	131
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	11	255	297	355	398	27	4	6	6	210	7	131
RTOR Reduct:	0	0	210	0	0	0	0	0	0	0	0	131
RTOR Vol:	11	255	87	355	398	27	4	6	6	210	7	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	11	255	87	355	398	27	4	6	6	210	7	0

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	1.00	1.00	0.94	0.06	0.25	0.37	0.38	0.97	0.03	1.00
Final Sat.:	1650	1650	1650	1650	1545	105	413	619	619	1597	53	1650

Capacity Analysis Module:

Vol/Sat:	0.01	0.15	0.05	0.22	0.26	0.26	0.01	0.01	0.01	0.13	0.13	0.00
Crit Volume:	255			355			16		217			
Crit Moves:	****			****			****		****			

 Level Of Service Computation Report
 CCTALOS Method (Base Volume Alternative)

 Intersection #26 Brentwood Blvd. @ Second St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.234
 Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 25 Level Of Service: A

Street Name:	Brentwood Blvd.						Second St.								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected			Permitted			Permitted					
Rights:	Include			Include			Include			Include					
Min. Green:	4	8	8	4	8	8	4	4	4	4	4	4			
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0	1	1	0

Volume Module:

Base Vol:	48	234	3	121	338	24	46	203	65	29	264	174
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	48	234	3	121	338	24	46	203	65	29	264	174
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	48	234	3	121	338	24	46	203	65	29	264	174
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	48	234	3	121	338	24	46	203	65	29	264	174
RTOR Reduct:	0	0	0	0	0	0	0	0	0	0	0	121
RTOR Vol:	48	234	3	121	338	24	46	203	65	29	264	53
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	48	234	3	121	338	24	46	203	65	29	264	53

Saturation Flow Module:

Sat/Lane:	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.97	0.03	1.00	1.87	0.13	1.00	1.51	0.49	1.00	2.00	1.00
Final Sat.:	1720	3396	44	1720	3212	228	1720	2606	834	1720	3440	1720

Capacity Analysis Module:

Vol/Sat:	0.03	0.07	0.07	0.07	0.11	0.11	0.03	0.08	0.08	0.02	0.08	0.03
Crit Volume:	118			121			134			29		
Crit Moves:	****			****			****			****		

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #27 Brentwood Blvd. @ Chestnut St.

Average Delay (sec/veh): 0.8 Worst Case Level Of Service: B[11.9]

Street Name:		Brentwood Blvd.				Chestnut St.			
Approach:	North Bound		South Bound		East Bound		West Bound		
Movement:	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled		Uncontrolled		Stop Sign		Stop Sign		
Rights:	Include		Include		Include		Include		
Lanes:	0	0	0	1	0	0	0	0	0

Volume Module:

Base Vol:	0	215	9	19	396	0	0	0	0	16	0	14
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	215	9	19	396	0	0	0	0	16	0	14
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	215	9	19	396	0	0	0	0	16	0	14
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	215	9	19	396	0	0	0	0	16	0	14

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	6.5	6.2
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	224	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	654	654	220
Potent Cap.:	xxxx	xxxx	xxxxxx	1299	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	432	386	820
Move Cap.:	xxxx	xxxx	xxxxxx	1299	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	427	381	820
Volume/Cap:	xxxx	xxxx	xxxx	0.01	xxxx	xxxx	xxxx	xxxx	xxxx	0.04	0.00	0.02

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.0	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	xxxxxx	xxxx	xxxxxx	7.8	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	*	*	*	A	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	550	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	0.0	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	0.2	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	7.8	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	11.9	xxxxxx
Shared LOS:	*	*	*	A	*	*	*	*	*	*	B	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			11.9		
ApproachLOS:	*			*			*			B		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 CCTALOS Method (Base Volume Alternative)

 Intersection #28 Brentwood Blvd. @ Balfour Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.442
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 41 Level Of Service: A

Street Name:	Brentwood Blvd.						Balfour Rd.									
Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Protected			Protected			Protected			Protected						
Rights:	Ignore			Include			Include			Include						
Min. Green:	8	10	10	6	10	10	5	5	5	4	5	5				
Lanes:	1	0	2	0	1	1	0	2	0	1	1	0	1	0	1	0

Volume Module:

Base Vol:	173	133	18	46	255	142	176	374	324	54	304	68
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	173	133	18	46	255	142	176	374	324	54	304	68
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	173	133	18	46	255	142	176	374	324	54	304	68
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	173	133	18	46	255	142	176	374	324	54	304	68
RTOR Reduct:	0	0	0	0	0	142	0	0	173	0	0	0
RTOR Vol:	173	133	18	46	255	0	176	374	151	54	304	68
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	173	133	18	46	255	0	176	374	151	54	304	68

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	1.00	1.00	1.00	1.63	0.37
Final Sat.:	1650	3300	1650	1650	3300	1650	1650	1650	1650	1650	2697	603

Capacity Analysis Module:

Vol/Sat:	0.10	0.04	0.01	0.03	0.08	0.00	0.11	0.23	0.09	0.03	0.11	0.11
Crit Volume:	173			128			374			54		
Crit Moves:	****			****			****			****		

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #29 Sellers Ave. @ Sunet Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.790

Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 22.0

Optimal Cycle: 0 Level Of Service: C

Street Name: Sellers Ave. Sunset Rd.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0

Volume Module:

Base Vol:	109	318	19	31	255	57	98	178	121	6	66	26
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	109	318	19	31	255	57	98	178	121	6	66	26
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	109	318	19	31	255	57	98	178	121	6	66	26
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	109	318	19	31	255	57	98	178	121	6	66	26
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	109	318	19	31	255	57	98	178	121	6	66	26

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.24	0.72	0.04	0.09	0.74	0.17	0.25	0.45	0.30	0.06	0.67	0.27
Final Sat.:	138	402	24	49	402	90	135	245	166	26	282	111

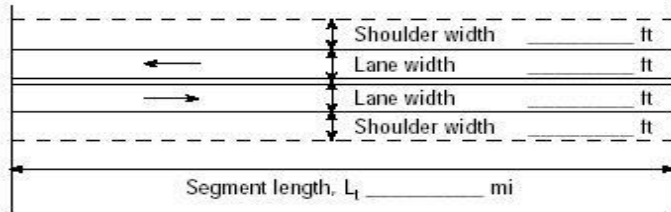
Capacity Analysis Module:

Vol/Sat:	0.79	0.79	0.79	0.63	0.63	0.63	0.73	0.73	0.73	0.23	0.23	0.23
Crit Moves:	****			****			****			****		
Delay/Veh:	26.9	26.9	26.9	18.3	18.3	18.3	22.4	22.4	22.4	11.8	11.8	11.8
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	26.9	26.9	26.9	18.3	18.3	18.3	22.4	22.4	22.4	11.8	11.8	11.8
LOS by Move:	D	D	D	C	C	C	C	C	C	B	B	B
ApproachDel:	26.9			18.3			22.4			11.8		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	26.9			18.3			22.4			11.8		
LOS by Appr:	D			C			C			B		
AllWayAvgQ:	2.8	2.8	2.8	1.4	1.4	1.4	2.0	2.0	2.0	0.2	0.2	0.2

Note: Queue reported is the number of cars per lane.

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	E Sims Rd. and Delta Rd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1112 veh/h Directional split 70 / 30 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1221	
v_p * highest directional split proportion ² (pc/h)		855	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	40 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	40.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		2.1	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		28.5	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1209	
v_p * highest directional split proportion ² (pc/h)		846	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		65.4	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		10.3	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		75.8	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.38	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		91	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	334
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	3.2
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	E Sims Rd. and Delta Rd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative
Project Description:			
Input Data			
		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1543 veh/h Directional split 62 / 38 Peak-hour factor, PHF 0.96 No-passing zone 100 % Trucks and Buses, P _T 10 % % Recreational vehicles, P _R 0 % Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f _G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))		0.990	
Two-way flow rate ¹ , v _p (pc/h)=V/(PHF * f _G * f _{HV})		1623	
v _p * highest directional split proportion ² (pc/h)		1006	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S _{FM}	40 mi/h	Base free-flow speed, BFFS _{FM}	mi/h
Observed volume, V _f	0 veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS FFS=S _{FM} +0.00776(V _f /f _{HV})	mi/h	Adj. for access points, f _A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A)	40.0 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)		1.5	
Average travel speed, ATS (mi/h) ATS=FFS-0.00776v _p -f _{np}		25.9	
Percent Time-Spent-Following			
Grade Adjustment factor, f _G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))		1.000	
Two-way flow rate ¹ , v _p (pc/h)=V/(PHF * f _G * f _{HV})		1607	
v _p * highest directional split proportion ² (pc/h)		996	
Base percent time-spent-following, BPTSF(%)=100(1-e ^{-0.000879v_p})		75.6	
Adj. for directional distribution and no-passing zone, f _{d/np} (%)(Exh. 20-12)		6.8	
Percent time-spent-following, PTSF(%)=BPTSF+f _{d/np}		82.5	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, v/c=V _p /3,200		0.51	
Peak 15-min veh-miles of travel, VMT ₁₅ (veh-mi)=0.25L _i (V/PHF)		121	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	463
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	4.7
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

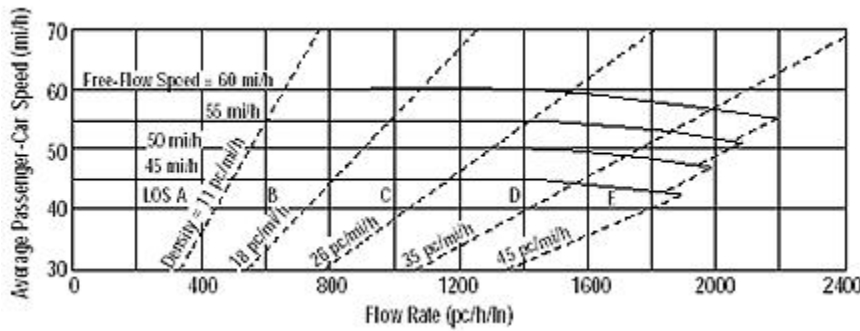
TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	Grant St. to Sunset Ct.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The diagram labels: Shoulder width (ft), Lane width (ft), Lane width (ft), Shoulder width (ft), and Segment length, L_i (mi).</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1680 veh/h Directional split 51 / 49 Peak-hour factor, PHF 0.97 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 10	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1749	
v_p * highest directional split proportion ² (pc/h)		892	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		1.4	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		15.1	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1732	
v_p * highest directional split proportion ² (pc/h)		883	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		78.2	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		6.0	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		84.1	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.55	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		43	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	168
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	2.9
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	Grant St. to Sunset Ct.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The diagram labels: Shoulder width (ft), Lane width (ft), Lane width (ft), Shoulder width (ft), and Segment length, L_i (mi).</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 2308 veh/h Directional split 51 / 49 Peak-hour factor, PHF 0.90 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 10	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		2590	
v_p * highest directional split proportion ² (pc/h)		1321	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		1.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		8.9	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		2564	
v_p * highest directional split proportion ² (pc/h)		1308	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		89.5	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		2.5	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		92.0	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.81	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		64	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	231
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	7.2
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Brentwood Blvd.
Agency or Company		From/To	Central Blvd. to Spruce S
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	289	Peak-Hour Factor, PHF	0.90
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

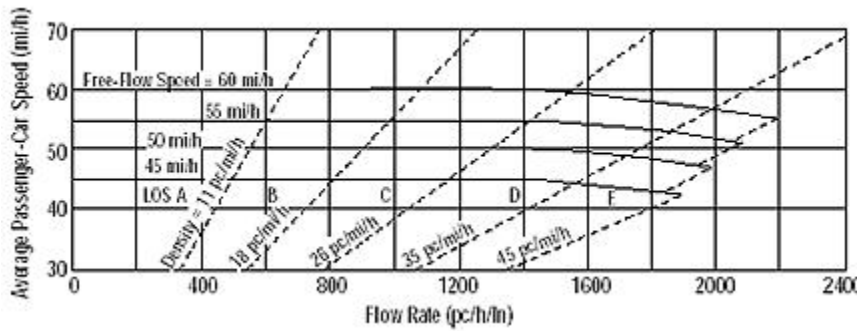
Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	168	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	3.7	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_i
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_i

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Central Blvd. to Spruce S
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	350	Peak-Hour Factor, PHF	0.95
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

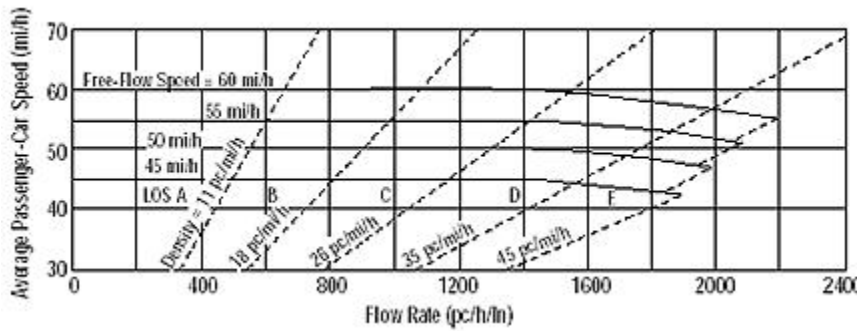
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 193
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 4.3
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Central Blvd. to Spruce S
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	541	Peak-Hour Factor, PHF	0.78
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

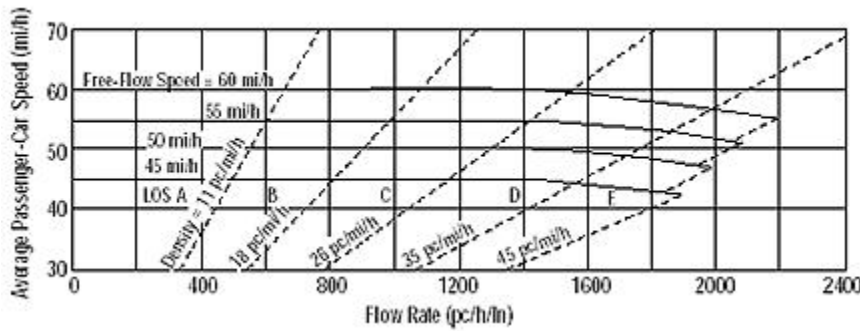
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 364
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 8.1
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Central Blvd. to Spruce S
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	483	Peak-Hour Factor, PHF	0.99
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

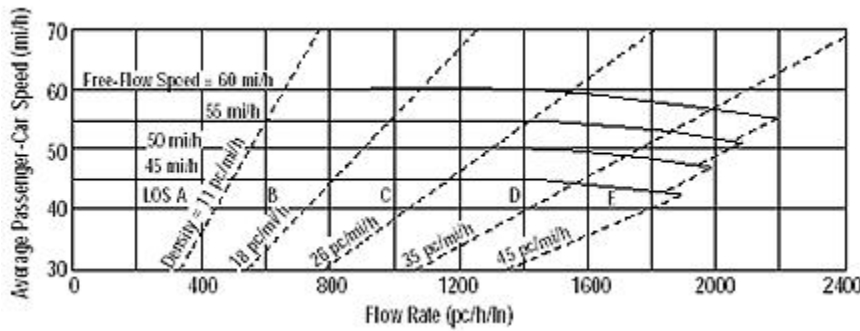
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 256
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 5.7
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Pine Ave. to Oak Ave.
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	290	Peak-Hour Factor, PHF	0.90
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

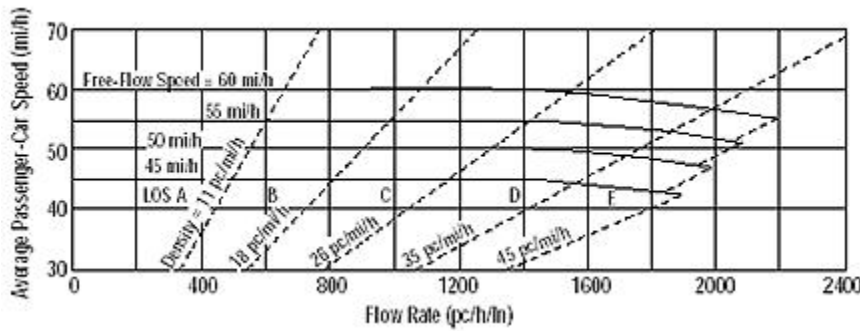
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 169
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 3.8
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Pine Ave. to Oak Ave.
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	160	Peak-Hour Factor, PHF	0.88
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

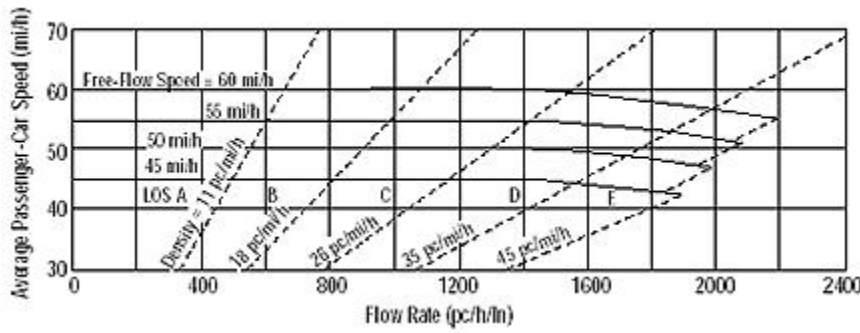
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 95
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 2.1
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Pine Ave. to Oak Ave.
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	285	Peak-Hour Factor, PHF	0.97
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

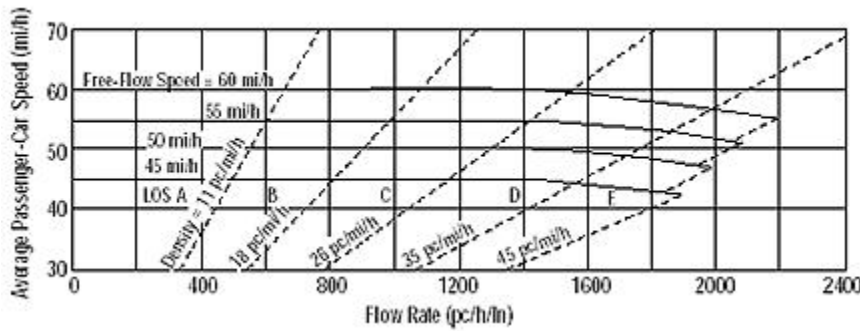
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 154
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 3.4
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Pine Ave. to Oak Ave.
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	350	Peak-Hour Factor, PHF	0.97
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 189
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 4.2
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Delta Rd.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 662 veh/h Directional split 62 / 38 Peak-hour factor, PHF 0.76 No-passing zone 0 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 40	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		875	
v_p * highest directional split proportion ² (pc/h)		543	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	50 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	50.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		0.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		43.2	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		873	
v_p * highest directional split proportion ² (pc/h)		541	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		53.6	
Adj. for directional distribution and no-passing zone, $f_{dnp}(\%)(Exh. 20-12)$		0.0	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{dnp}$		53.6	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		B	
Volume to capacity ratio, $v/c=V_p/3,200$		0.27	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		218	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	662
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	5.0
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Delta Rd.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The diagram labels: Shoulder width (ft), Lane width (ft), Lane width (ft), Shoulder width (ft), and Segment length, L_i (mi).</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 888 veh/h Directional split 61 / 39 Peak-hour factor, PHF 0.91 No-passing zone 0 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 40	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		980	
v_p * highest directional split proportion ² (pc/h)		598	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	50 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	50.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		0.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		42.4	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		978	
v_p * highest directional split proportion ² (pc/h)		597	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		57.7	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		0.0	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		57.7	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.31	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		244	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	888
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	5.8
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

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 Suite 120
 Roseville, CA 95747
 Phone:
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OPERATIONAL ANALYSIS

Analyst: Kimley-Horn and Associates
 Agency/Co:
 Date: 4/14/08
 Analysis Period: AM Peak
 Highway: Lone Tree
 From/To: West of Brentwood
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative
 Project ID:

FREE-FLOW SPEED

	Direction	1		2	
Lane width		12.0	ft	12.0	ft
Lateral clearance:					
Right edge		6.0	ft	6.0	ft
Left edge		6.0	ft	6.0	ft
Total lateral clearance		12.0	ft	12.0	ft
Access points per mile		0		0	
Median type		Divided		Divided	
Free-flow speed:		Base		Base	
FFS or BFFS		50.0	mph	50.0	mph
Lane width adjustment, FLW		0.0	mph	0.0	mph
Lateral clearance adjustment, FLC		0.0	mph	0.0	mph
Median type adjustment, FM		0.0	mph	0.0	mph
Access points adjustment, FA		0.0	mph	0.0	mph
Free-flow speed		50.0	mph	50.0	mph

VOLUME

	Direction	1		2	
Volume, V		993	vph	917	vph
Peak-hour factor, PHF		0.92		0.92	
Peak 15-minute volume, v15		270		249	
Trucks and buses		2	%	2	%
Recreational vehicles		4	%	4	%
Terrain type		Level		Level	
Grade		0.00	%	0.00	%
Segment length		0.00	mi	0.00	mi
Number of lanes		2		2	
Driver population adjustment, fP		1.00		1.00	
Trucks and buses PCE, ET		1.5*		1.5	
Recreational vehicles PCE, ER		1.2		1.2	
Heavy vehicle adjustment, fHV		0.982		0.982	
Flow rate, vp		549	pcphpl	507	pcphpl

RESULTS

	Direction	1		2	
Flow rate, vp		549	pcphp1	507	pcphp1
Free-flow speed, FFS		50.0	mph	50.0	mph
Avg. passenger-car travel speed, S		50.0	mph	50.0	mph
Level of service, LOS		A		A	
Density, D		11.0-	pc/mi/ln	10.1	pc/mi/ln

Overall results are not computed when free-flow speed is less than 45 mph.

Steve Pyburn
 Kimley-Horn and Associates, Inc.
 1430 Blue Oaks Bl
 Suite 120
 Roseville, CA 95747
 Phone:
 E-mail:

Fax:

 OPERATIONAL ANALYSIS

Analyst: Kimley-Horn and Associates
 Agency/Co:
 Date: 4/14/08
 Analysis Period: PM Peak
 Highway: Lone Tree
 From/To: West of Brentwood
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative
 Project ID:

 FREE-FLOW SPEED

Direction	1		2	
Lane width	12.0	ft	12.0	ft
Lateral clearance:				
Right edge	6.0	ft	6.0	ft
Left edge	6.0	ft	6.0	ft
Total lateral clearance	12.0	ft	12.0	ft
Access points per mile	0		0	
Median type	Divided		Divided	
Free-flow speed:	Base		Base	
FFS or BFFS	50.0	mph	50.0	mph
Lane width adjustment, FLW	0.0	mph	0.0	mph
Lateral clearance adjustment, FLC	0.0	mph	0.0	mph
Median type adjustment, FM	0.0	mph	0.0	mph
Access points adjustment, FA	0.0	mph	0.0	mph
Free-flow speed	50.0	mph	50.0	mph

 VOLUME

Direction	1		2	
Volume, V	1167	vph	1265	vph
Peak-hour factor, PHF	0.92		0.92	
Peak 15-minute volume, v15	317		344	
Trucks and buses	2	%	2	%
Recreational vehicles	4	%	4	%
Terrain type	Level		Level	
Grade	0.00	%	0.00	%
Segment length	0.00	mi	0.00	mi
Number of lanes	2		2	
Driver population adjustment, fP	1.00		1.00	
Trucks and buses PCE, ET	1.5*		1.5	
Recreational vehicles PCE, ER	1.2		1.2	
Heavy vehicle adjustment, fHV	0.982		0.982	
Flow rate, vp	645	pcphpl	699	pcphpl

 RESULTS

	Direction	1		2	
Flow rate, vp		645	pcphp1	699	pcphp1
Free-flow speed, FFS		50.0	mph	50.0	mph
Avg. passenger-car travel speed, S		50.0	mph	50.0	mph
Level of service, LOS		B		B	
Density, D		12.9	pc/mi/ln	14.0	pc/mi/ln

Overall results are not computed when free-flow speed is less than 45 mph.

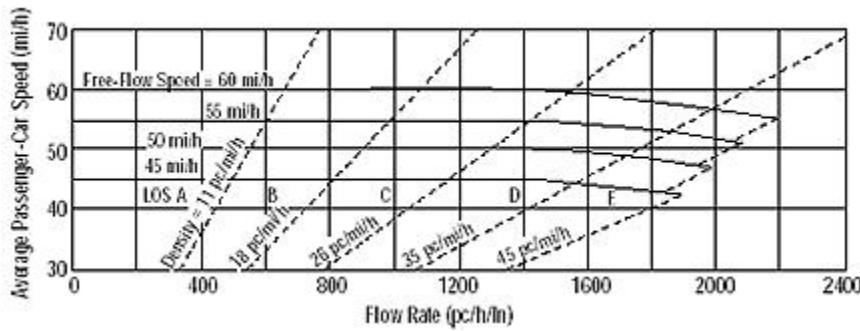
TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sunset Ave
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 616 veh/h Directional split 63 / 37 Peak-hour factor, PHF 0.88 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		703	
v_p * highest directional split proportion ² (pc/h)		443	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.4	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		21.1	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		701	
v_p * highest directional split proportion ² (pc/h)		442	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		46.0	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		17.6	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		63.6	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.22	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		175	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	616
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	8.3
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sunset Ave
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 734 veh/h Directional split 61 / 39 Peak-hour factor, PHF 0.88 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		837	
v_p * highest directional split proportion ² (pc/h)		511	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		2.9	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		20.6	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		836	
v_p * highest directional split proportion ² (pc/h)		510	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		52.0	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		14.0	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		66.1	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.26	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		209	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	734
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	10.2
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Central Blvd.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	630	Peak-Hour Factor, PHF	0.86
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

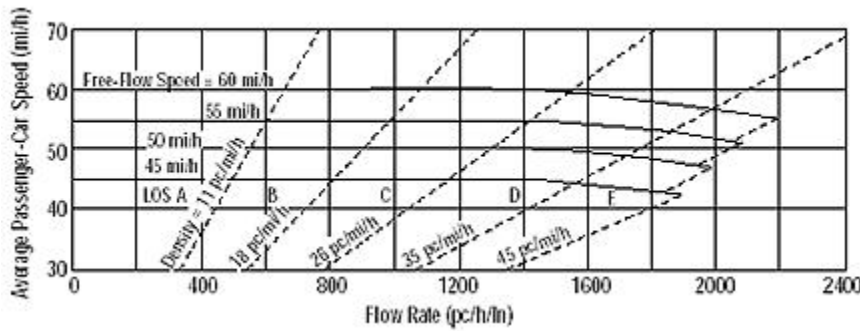
Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	369	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	8.2	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Central Blvd.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	393	Peak-Hour Factor, PHF	0.88
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

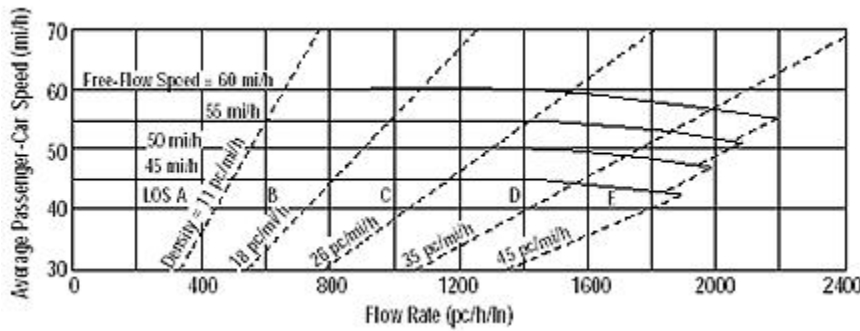
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 225
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 5.0
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_l
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_l

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Central Blvd.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	539	Peak-Hour Factor, PHF	0.84
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

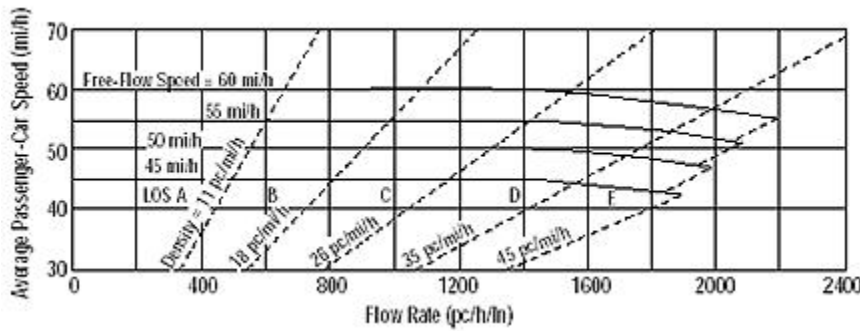
Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	324	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	7.2	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Central Blvd.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	945	Peak-Hour Factor, PHF	0.76
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 627
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 13.9
 LOS: B

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Grant St.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 309 veh/h Directional split 71 / 29 Peak-hour factor, PHF 0.71 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 7	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		441	
v_p * highest directional split proportion ² (pc/h)		313	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		4.4	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		22.2	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		436	
v_p * highest directional split proportion ² (pc/h)		310	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		31.8	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		23.2	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		55.1	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.14	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		109	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	309
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	4.9
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Grant St.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 247 veh/h Directional split 66 / 34 Peak-hour factor, PHF 0.87 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 7	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		288	
v_p * highest directional split proportion ² (pc/h)		190	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.9	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		23.8	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		284	
v_p * highest directional split proportion ² (pc/h)		187	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		22.1	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		23.9	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		46.0	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		B	
Volume to capacity ratio, $v/c=V_p/3,200$		0.09	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		71	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	247
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	3.0
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sycamore Ave.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 481 veh/h Directional split 74 / 26 Peak-hour factor, PHF 0.87 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 35	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		561	
v_p * highest directional split proportion ² (pc/h)		415	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		4.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		21.6	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		554	
v_p * highest directional split proportion ² (pc/h)		410	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		38.6	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		23.0	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		61.5	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.18	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		69	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	241
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	3.2
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sycamore Ave.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 513 veh/h Directional split 69 / 31 Peak-hour factor, PHF 0.82 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 35	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		628	
v_p * highest directional split proportion ² (pc/h)		433	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.8	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		21.4	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		627	
v_p * highest directional split proportion ² (pc/h)		433	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		42.4	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		20.0	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		62.4	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.20	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		78	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	257
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	3.7
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

Appendix E:

*Analysis Worksheets for
Cumulative plus Proposed Project Conditions*

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 Brentwood Blvd. @ Delta Road

Average Delay (sec/veh): 227.7 Worst Case Level Of Service: F[862.5]

Table with columns for Street Name (Brentwood Blvd., Delta Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach and movement.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach and movement.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach and movement.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #2 Sellers Ave @ Delta

Cycle (sec): 100 Critical Vol./Cap.(X): 0.488
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 11.6
Optimal Cycle: 0 Level Of Service: B

Street Name: Sellers Ave. Delta Rd.

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Stop Sign Stop Sign Stop Sign Stop Sign
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0

Volume Module:

Base Vol: 46 131 72 6 94 17 19 111 88 142 159 15
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 46 131 72 6 94 17 19 111 88 142 159 15
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 46 131 72 6 94 17 19 111 88 142 159 15
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 46 131 72 6 94 17 19 111 88 142 159 15
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 46 131 72 6 94 17 19 111 88 142 159 15

Saturation Flow Module:

Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.18 0.53 0.29 0.05 0.80 0.15 0.09 0.51 0.40 0.45 0.50 0.05
Final Sat.: 113 323 177 29 452 82 57 330 262 291 326 31

Capacity Analysis Module:

Vol/Sat: 0.41 0.41 0.41 0.21 0.21 0.21 0.34 0.34 0.34 0.49 0.49 0.49
Crit Moves: ****
Delay/Veh: 11.8 11.8 11.8 10.1 10.1 10.1 10.5 10.5 10.5 12.9 12.9 12.9
Delay Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
AdjDel/Veh: 11.8 11.8 11.8 10.1 10.1 10.1 10.5 10.5 10.5 12.9 12.9 12.9
LOS by Move: B B B B B B B B B B B B
ApproachDel: 11.8 10.1 10.5 12.9
Delay Adj: 1.00 1.00
ApprAdjDel: 11.8 10.1 10.5 12.9
LOS by Appr: B B B
AllWayAvgQ: 0.6 0.6 0.6 0.2 0.2 0.2 0.4 0.4 0.4 0.8 0.8 0.8

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #3 Brentwood Blvd. @ E. Sims Road

Average Delay (sec/veh): 1.7 Worst Case Level Of Service: E[40.8]

Table with columns for Street Name (Brentwood Blvd., E. Sims Road), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 0, 0, 0, 0, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach and movement.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach and movement.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach and movement.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #4 Brentwood Blvd @ Lone Tree Way (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.840
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 143 Level Of Service: D

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd and Lone Tree Way (S) with various movement details.

Volume Module: Table showing traffic volume adjustments including Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table showing saturation flow values for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table showing capacity analysis values for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #5 Arroyo Seco Road @ Lone Tree Way

Average Delay (sec/veh): 8.1 Worst Case Level Of Service: F[3605.0]

Table with columns for Street Name (Arroyo Seco Road, Lone Tree Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Stop Sign, Uncontrolled), Rights (Include), and Lanes (1, 0, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS, and values for each approach.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #6 Brentwood Blvd. @ Sunrise Drive

Average Delay (sec/veh): 0.9 Worst Case Level Of Service: F[62.9]

Table with columns for Street Name (Brentwood Blvd., Sunrise Dr.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #7 Brentwood Blvd. @ Gregory Lane

Average Delay (sec/veh): 0.9 Worst Case Level Of Service: F[62.6]

Street Name: Brentwood Blvd. Gregory Ln.

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	0	0	0	1	0	0	1

Volume Module:

Base Vol:	10	1191	0	0	968	8	14	0	17	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	10	1191	0	0	968	8	14	0	17	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	10	1191	0	0	968	8	14	0	17	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	10	1191	0	0	968	8	14	0	17	0	0	0

Critical Gap Module:

Critical Gp:	4.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	6.5	6.2	7.1	6.5	6.2
FollowUpTim:	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	4.0	3.3	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	976	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	2183	2183	972	2192	2187	1191
Potent Cap.:	676	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	50	46	306	33	46	228
Move Cap.:	676	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	50	45	306	30	45	228
Volume/Cap:	0.01	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	0.28	0.00	0.06	0.00	0.00	0.00

Level Of Service Module:

2Way95thQ:	0.0	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	10.4	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	B	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxx	92	xxxxxx	xxxx	0	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	1.3	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	62.6	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	F	*	*	*	*
ApproachDel:	xxxxxxx			xxxxxxx			62.6			xxxxxxx		
ApproachLOS:	*			*			F			*		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #8 Brentwood Blvd. @ Hanson Lane

Average Delay (sec/veh): 5.6 Worst Case Level Of Service: F[160.8]

Table with columns for Street Name (Brentwood Blvd., Hanson Ln), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #9 Brentwood Blvd. @ Homecoming Way

Average Delay (sec/veh): 1.0 Worst Case Level Of Service: F[58.4]

Table with columns for Street Name (Brentwood Blvd., Homecoming Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume for each approach and movement.

Critical Gap Module: Table showing Critical Gap and FollowUp Time for each approach and movement.

Capacity Module: Table showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap. for each approach and movement.

Level Of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, Approach Del, and Approach LOS for each approach and movement.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #10 Brentwood Blvd. @ Grant Street / Sunset Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.768
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 98 Level Of Service: C

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Grant Street / Sunset Rd. with sub-rows for North, South, East, and West bounds.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume. Rows list various adjustment factors and resulting volumes.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. Rows show saturation flow values and adjustments for different lane configurations.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves. Rows show capacity analysis results for different lane configurations.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #11 Brentwood Blvd. @ Sunset Ct.

Average Delay (sec/veh): 0.8 Worst Case Level Of Service: C[17.7]

Table with columns for Street Name (Brentwood Blvd., Sunset Ct.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume for each approach and movement.

Critical Gap Module: Table showing Critical Gp and FollowUpTim for each approach and movement.

Capacity Module: Table showing Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap for each approach and movement.

Level Of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS for each approach and movement.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #12 Brentwood Blvd. @ Havenwood Ave.

Average Delay (sec/veh): 0.8 Worst Case Level Of Service: D[32.4]

Table with columns for Street Name (Brentwood Blvd., Havenwood Ave.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 1, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #13 Brentwood Blvd. @ Applewood Common

Cycle (sec): 100 Critical Vol./Cap.(X): 0.657
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 66 Level Of Service: B

Table with columns for Street Name (Brentwood Blvd., Applewood Common), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and FinalVolume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #14 Brentwood Blvd. @ Sand Creek Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.470
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 35 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Sand Creek Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green (4, 10, 10), and Lanes (1, 0, 1, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume, with values for 12 different movement categories.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat., with values for 12 different movement categories.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves, with values for 12 different movement categories.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #15 Brentwood Blvd. @ Technology Way

Cycle (sec): 100 Critical Vol./Cap.(X): 0.273
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 26 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Technology Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green (4, 6, 6, 0, 6, 6, 4, 4, 4, 0, 0, 0), and Lanes (1, 0, 2, 0, 0, 0, 0, 1, 1, 0, 1, 0, 0, 0, 0, 0).

Volume Module: Table with columns for Base Vol (27, 929, 0, 0, 808, 11, 23, 0, 60, 0, 0, 0), Growth Adj (1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00), Initial Bse (27, 929, 0, 0, 808, 11, 23, 0, 60, 0, 0, 0), User Adj (1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00), PHF Adj (1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00), PHF Volume (27, 929, 0, 0, 808, 11, 23, 0, 60, 0, 0, 0), Reduct Vol (0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0), Reduced Vol (27, 929, 0, 0, 808, 11, 23, 0, 60, 0, 0, 0), RTOR Reduct (0, 0, 0, 0, 0, 0, 0, 0, 27, 0, 0, 0), RTOR Vol (27, 929, 0, 0, 808, 11, 23, 0, 33, 0, 0, 0), PCE Adj (1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00), MLF Adj (1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00), FinalVolume (27, 929, 0, 0, 808, 11, 23, 0, 33, 0, 0, 0).

Saturation Flow Module: Table with columns for Sat/Lane (1720, 1720, 1720, 1720, 1720, 1720, 1720, 1720, 1720, 1720, 1720, 1720), Adjustment (1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00, 1.00), Lanes (1.00, 2.00, 0.00, 0.00, 1.97, 0.03, 1.00, 0.00, 1.00, 0.00, 0.00, 0.00), Final Sat. (1720, 3440, 0, 0, 3394, 46, 1720, 0, 1720, 0, 0, 0).

Capacity Analysis Module: Table with columns for Vol/Sat (0.02, 0.27, 0.00, 0.00, 0.24, 0.24, 0.01, 0.00, 0.02, 0.00, 0.00, 0.00), Crit Volume (27, 410, 33, 0), Crit Moves (****, ****, ****).

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #16 Brentwood Blvd @ Nancy Street (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.385
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 30 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Nancy Street (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Table for Volume Module showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and FinalVolume across various movement categories.

Table for Saturation Flow Module showing Sat/Lane, Adjustment, Lanes, and Final Sat. values.

Table for Capacity Analysis Module showing Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #17 Brentwood Blvd. @ Village Drive

Average Delay (sec/veh): 1.2 Worst Case Level Of Service: D[33.1]

Table with columns for Street Name (Brentwood Blvd., Village Drive), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #18 Brentwood Blvd. @ Central Blvd / Sycamore Ave (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.322
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 32 Level Of Service: A

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Central Blvd / Sycamore Ave (S) with various movement and control details.

Volume Module: Table showing traffic volume adjustments including Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table showing saturation flow rates and adjustments for different lane configurations.

Capacity Analysis Module: Table showing capacity analysis metrics such as Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #20 Central Street @ Second Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.485
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 44 Level Of Service: A

Table with columns for Street Name (Second Street, Central Boulevard), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include, Ignore), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #21 Central Blvd @ Walnut Blvd

Cycle (sec): 100 Critical Vol./Cap.(X): 0.456
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 42 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Central Blvd), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green (5, 7, 7), and Lanes (1 0 1 0 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #22 Brentwood Blvd @ Pine Street

Average Delay (sec/veh): 0.2 Worst Case Level Of Service: A[9.6]

Table with columns for Street Name (Brentwood Blvd, Pine Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, and Lanes.

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume for each approach.

Critical Gap Module: Table showing Critical Gap and FollowUpTime for each approach.

Capacity Module: Table showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap for each approach.

Level Of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS for each approach.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #23 Brentwood Blvd @ Maple Street

Average Delay (sec/veh): 5.0 Worst Case Level Of Service: B[11.8]

Table with columns for Street Name (Brentwood Blvd, Maple Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #24 Brentwood Blvd @ Oak Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.168
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 34 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Oak St), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green (4, 10, 10), and Lanes (1, 0, 1, 1, 0).

Volume Module table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Saturation Flow Module table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across various movement categories.

Capacity Analysis Module table with columns for Vol/Sat, Crit Volume, and Crit Moves across various movement categories.

Level Of Service Computation Report
 CCTALOS Method (Base Volume Alternative)

 Intersection #25 Walnut Blvd @ Oak St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.511
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 47 Level Of Service: A

Street Name: Walnut Blvd						Oak St.					
Approach: North Bound			South Bound			East Bound			West Bound		
Movement: L - T - R			L - T - R			L - T - R			L - T - R		
Control: Protected			Protected			Split Phase			Split Phase		
Rights: Include			Include			Include			Include		
Min. Green:	5	7	7	0	7	7	5	5	5	5	5
Lanes:	1	0	1	0	1	0	0	1	0	0	1

Volume Module:

Base Vol:	6	262	566	273	193	4	1	1	0	112	2	49
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	6	262	566	273	193	4	1	1	0	112	2	49
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	6	262	566	273	193	4	1	1	0	112	2	49
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	6	262	566	273	193	4	1	1	0	112	2	49
RTOR Reduct:	0	0	112	0	0	0	0	0	0	0	0	49
RTOR Vol:	6	262	454	273	193	4	1	1	0	112	2	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	6	262	454	273	193	4	1	1	0	112	2	0

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	1.00	1.00	0.98	0.02	0.50	0.50	0.00	0.98	0.02	1.00
Final Sat.:	1650	1650	1650	1650	1616	34	825	825	0	1621	29	1650

Capacity Analysis Module:

Vol/Sat:	0.00	0.16	0.28	0.17	0.12	0.12	0.00	0.00	0.00	0.07	0.07	0.00
Crit Volume:			454		273			2			114	
Crit Moves:			****		****			****			****	

 Level Of Service Computation Report
 CCTALOS Method (Base Volume Alternative)

 Intersection #26 Brentwood Blvd. @ Second St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.214
 Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 25 Level Of Service: A

Street Name:	Brentwood Blvd.						Second St.								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected			Permitted			Permitted					
Rights:	Include			Include			Include			Include					
Min. Green:	4	8	8	4	8	8	4	4	4	4	4	4			
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0	1	1	0

Volume Module:

Base Vol:	55	213	12	103	221	12	40	139	23	45	225	165
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	55	213	12	103	221	12	40	139	23	45	225	165
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	55	213	12	103	221	12	40	139	23	45	225	165
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	55	213	12	103	221	12	40	139	23	45	225	165
RTOR Reduct:	0	0	0	0	0	0	0	0	0	0	0	103
RTOR Vol:	55	213	12	103	221	12	40	139	23	45	225	62
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	55	213	12	103	221	12	40	139	23	45	225	62

Saturation Flow Module:

Sat/Lane:	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.89	0.11	1.00	1.90	0.10	1.00	1.72	0.28	1.00	2.00	1.00
Final Sat.:	1720	3257	183	1720	3263	177	1720	2952	488	1720	3440	1720

Capacity Analysis Module:

Vol/Sat:	0.03	0.07	0.07	0.06	0.07	0.07	0.02	0.05	0.05	0.03	0.07	0.04
Crit Volume:	113			103			40			113		
Crit Moves:	****			****			****			****		

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #27 Brentwood Blvd. @ Chestnut St.

Average Delay (sec/veh): 0.6 Worst Case Level Of Service: B[10.3]

Street Name: Brentwood Blvd.			Chestnut St.		
Approach:	North Bound	South Bound	East Bound	West Bound	
Movement:	L - T - R	L - T - R	L - T - R	L - T - R	
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign	
Rights:	Include	Include	Include	Include	
Lanes:	0 0 0 1 0	0 1 0 0 0	0 0 0 0 0	0 0 1 0 0	

Volume Module:

Base Vol:	0	269	18	11	104	0	0	0	0	5	0	10
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	269	18	11	104	0	0	0	0	5	0	10
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	269	18	11	104	0	0	0	0	5	0	10
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	269	18	11	104	0	0	0	0	5	0	10

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	6.5	6.2
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	287	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	404	404	278
Potent Cap.:	xxxx	xxxx	xxxxxx	1231	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	603	536	761
Move Cap.:	xxxx	xxxx	xxxxxx	1231	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	598	531	761
Volume/Cap:	xxxx	xxxx	xxxx	0.01	xxxx	xxxx	xxxx	xxxx	xxxx	0.01	0.00	0.01

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.0	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	xxxxxx	xxxx	xxxxxx	8.0	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	*	*	*	A	*	*	*	*	*	*	*	*
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	698	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	0.0	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	0.1	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	8.0	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	10.3	xxxxxx
Shared LOS:	*	*	*	A	*	*	*	*	*	*	B	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			10.3		
ApproachLOS:	*			*			*			B		

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report
 CCTALOS Method (Base Volume Alternative)

 Intersection #28 Brentwood Blvd. @ Balfour Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.340
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 40 Level Of Service: A

Street Name:	Brentwood Blvd.						Balfour Rd.								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected			Protected			Protected					
Rights:	Ignore			Include			Include			Include					
Min. Green:	8	10	10	6	10	10	5	5	5	4	5	5			
Lanes:	1	0	2	0	1	1	0	2	0	1	1	0	1	1	0

Volume Module:

Base Vol:	218	299	13	8	58	45	118	162	154	30	344	49
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	218	299	13	8	58	45	118	162	154	30	344	49
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	218	299	13	8	58	45	118	162	154	30	344	49
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	218	299	13	8	58	45	118	162	154	30	344	49
RTOR Reduct:	0	0	0	0	0	45	0	0	154	0	0	0
RTOR Vol:	218	299	13	8	58	0	118	162	0	30	344	49
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	218	299	13	8	58	0	118	162	0	30	344	49

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	1.00	1.00	1.00	1.75	0.25
Final Sat.:	1650	3300	1650	1650	3300	1650	1650	1650	1650	1650	2889	411

Capacity Analysis Module:

Vol/Sat:	0.13	0.09	0.01	0.00	0.02	0.00	0.07	0.10	0.00	0.02	0.12	0.12
Crit Volume:	218				29		118				197	
Crit Moves:	****				****		****				****	

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #29 Sellers Ave. @ Sunet Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.648
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 13.8
 Optimal Cycle: 0 Level Of Service: B

Street Name:		Sellers Ave.						Sunset Rd.					
Approach:	North Bound			South Bound			East Bound			West Bound			
Movement:	L	T	R	L	T	R	L	T	R	L	T	R	
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign			
Rights:	Include			Include			Include			Include			
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0	
Lanes:	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0	

Volume Module:

Base Vol:	128	291	13	10	222	83	49	65	71	18	61	22
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	128	291	13	10	222	83	49	65	71	18	61	22
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	128	291	13	10	222	83	49	65	71	18	61	22
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	128	291	13	10	222	83	49	65	71	18	61	22
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	128	291	13	10	222	83	49	65	71	18	61	22

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.30	0.67	0.03	0.03	0.71	0.26	0.26	0.35	0.39	0.18	0.60	0.22
Final Sat.:	197	449	20	21	464	173	149	198	216	92	313	113

Capacity Analysis Module:

Vol/Sat:	0.65	0.65	0.65	0.48	0.48	0.48	0.33	0.33	0.33	0.19	0.19	0.19
Crit Moves:	****			****			****			****		
Delay/Veh:	16.7	16.7	16.7	12.5	12.5	12.5	11.2	11.2	11.2	10.3	10.3	10.3
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	16.7	16.7	16.7	12.5	12.5	12.5	11.2	11.2	11.2	10.3	10.3	10.3
LOS by Move:	C	C	C	B	B	B	B	B	B	B	B	B
ApproachDel:	16.7			12.5			11.2			10.3		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	16.7			12.5			11.2			10.3		
LOS by Appr:	C			B			B			B		
AllWayAvgQ:	1.6	1.6	1.6	0.8	0.8	0.8	0.4	0.4	0.4	0.2	0.2	0.2

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 Brentwood Blvd. @ Delta Road

Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx]

Table with columns for Street Name (Brentwood Blvd., Delta Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #2 Sellers Ave @ Delta

Cycle (sec): 100 Critical Vol./Cap.(X): 0.822
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 21.1
Optimal Cycle: 0 Level Of Service: C

Street Name: Sellers Ave. Delta Rd.
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Stop Sign Stop Sign Stop Sign Stop Sign
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0

Volume Module:
Base Vol: 92 237 42 4 49 5 95 285 153 47 107 34
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 92 237 42 4 49 5 95 285 153 47 107 34
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 92 237 42 4 49 5 95 285 153 47 107 34
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 92 237 42 4 49 5 95 285 153 47 107 34
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 92 237 42 4 49 5 95 285 153 47 107 34

Saturation Flow Module:
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.25 0.64 0.11 0.07 0.84 0.09 0.18 0.53 0.29 0.25 0.57 0.18
Final Sat.: 140 361 64 32 393 40 116 347 186 137 313 99

Capacity Analysis Module:
Vol/Sat: 0.66 0.66 0.66 0.12 0.12 0.12 0.82 0.82 0.82 0.34 0.34 0.34
Crit Moves: **** **** ****
Delay/Veh: 18.8 18.8 18.8 10.6 10.6 10.6 27.2 27.2 27.2 11.8 11.8 11.8
Delay Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
AdjDel/Veh: 18.8 18.8 18.8 10.6 10.6 10.6 27.2 27.2 27.2 11.8 11.8 11.8
LOS by Move: C C C B B B D D D B B B
ApproachDel: 18.8 10.6 27.2 11.8
Delay Adj: 1.00 1.00 1.00
ApprAdjDel: 18.8 10.6 27.2 11.8
LOS by Appr: C B D B
AllWayAvgQ: 1.5 1.5 1.5 0.1 0.1 0.1 3.4 3.4 3.4 0.4 0.4 0.4

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #3 Brentwood Blvd. @ E. Sims Road

Average Delay (sec/veh): 14.9 Worst Case Level Of Service: F[166.7]

Table with columns for Street Name (Brentwood Blvd., E. Sims Road), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 0, 0, 0, 0, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach and movement.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach and movement.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach and movement.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #4 Brentwood Blvd @ Lone Tree Way (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 1.103
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with columns for Street Name (Brentwood Blvd, Lone Tree Way (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include, Ovl), Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #5 Arroyo Seco Road @ Lone Tree Way

Average Delay (sec/veh): 9.0 Worst Case Level Of Service: F[3605.0]

Table with columns for Street Name (Arroyo Seco Road, Lone Tree Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Stop Sign, Uncontrolled), Rights (Include), and Lanes (1, 0, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across various movements.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across various movements.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across various movements.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across various movements.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #6 Brentwood Blvd. @ Sunrise Drive

Average Delay (sec/veh): 10.9 Worst Case Level Of Service: F[550.1]

Table with columns for Street Name (Brentwood Blvd., Sunrise Dr.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across various movement categories.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across various movement categories.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across various movement categories.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across various movement categories.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #7 Brentwood Blvd. @ Gregory Lane

Average Delay (sec/veh): 9.0 Worst Case Level Of Service: F[455.7]

Table with columns for Street Name (Brentwood Blvd., Gregory Ln.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach and movement.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach and movement.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach and movement.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #8 Brentwood Blvd. @ Hanson Lane

Average Delay (sec/veh): 40.2 Worst Case Level Of Service: F[1336.6]

Table with columns for Street Name (Brentwood Blvd., Hanson Ln), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1, 0, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #9 Brentwood Blvd. @ Homecoming Way

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: F[133.5]

Table with columns for Street Name (Brentwood Blvd., Homecoming Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #10 Brentwood Blvd. @ Grant Street / Sunset Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 1.015
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Street Name: Brentwood Blvd. Grant Street / Sunset Rd.
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Protected Protected Split Phase Split Phase
Rights: Include Include Include Include
Min. Green: 4 10 10 4 10 10 4 4 4 4 4 4 4
Lanes: 1 0 1 0 1 1 0 0 1 0 0 0 1! 0 0 0 1 0 0 1

Volume Module:
Base Vol: 35 907 93 128 1323 34 15 44 20 136 68 144
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 35 907 93 128 1323 34 15 44 20 136 68 144
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 35 907 93 128 1323 34 15 44 20 136 68 144
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 35 907 93 128 1323 34 15 44 20 136 68 144
RTOR Reduct: 0 0 93 0 0 0 0 0 0 0 0 0 128
RTOR Vol: 35 907 0 128 1323 34 15 44 20 136 68 16
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 35 907 0 128 1323 34 15 44 20 136 68 16

Saturation Flow Module:
Sat/Lane: 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 1.00 1.00 1.00 0.97 0.03 0.19 0.56 0.25 0.67 0.33 1.00
Final Sat.: 1650 1650 1650 1650 1609 41 313 919 418 1100 550 1650

Capacity Analysis Module:
Vol/Sat: 0.02 0.55 0.00 0.08 0.82 0.82 0.05 0.05 0.05 0.12 0.12 0.01
Crit Volume: 35 1357 79 204
Crit Moves: **** **** **** ****

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #11 Brentwood Blvd. @ Sunset Ct.

Average Delay (sec/veh): 2.7 Worst Case Level Of Service: E[40.2]

Table with columns for Street Name (Brentwood Blvd., Sunset Ct.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume for each approach and movement.

Critical Gap Module: Table showing Critical Gp and FollowUpTim for each approach and movement.

Capacity Module: Table showing Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap for each approach and movement.

Level Of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS for each approach and movement.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #12 Brentwood Blvd. @ Havenwood Ave.

Average Delay (sec/veh): 8.6 Worst Case Level Of Service: F[513.5]

Table with columns for Street Name (Brentwood Blvd., Havenwood Ave.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (1 0 1 0 1, 1 0 0 1 0, 0 0 0 0 1, 0 0 1! 0 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume, and values for each approach.

Critical Gap Module: Table with columns for Critical Gp, FollowUpTim, and values for each approach.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., Volume/Cap, and values for each approach.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #13 Brentwood Blvd. @ Applewood Common

Cycle (sec): 100 Critical Vol./Cap.(X): 0.810
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 120 Level Of Service: D

Table with columns for Street Name (Brentwood Blvd., Applewood Common), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green (4, 8, 8), and Lanes (1, 0, 1, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume, with values for 12 approaches.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat., with values for 12 approaches.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves, with values for 12 approaches.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #14 Brentwood Blvd. @ Sand Creek Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.770
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 81 Level Of Service: C

Table with columns for Street Name (Brentwood Blvd., Sand Creek Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green (4, 10, 10), and Lanes (1, 0, 1, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #15 Brentwood Blvd. @ Technology Way

Cycle (sec): 100 Critical Vol./Cap.(X): 0.423
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 32 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd., Technology Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected), Rights (Include), Min. Green, and Lanes.

Table for Volume Module showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Table for Saturation Flow Module showing Sat/Lane, Adjustment, Lanes, and Final Sat. values.

Table for Capacity Analysis Module showing Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #16 Brentwood Blvd @ Nancy Street (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.542
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 41 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Nancy Street (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #17 Brentwood Blvd. @ Village Drive

Average Delay (sec/veh): 13.0 Worst Case Level Of Service: F[352.5]

Table with columns for Street Name (Brentwood Blvd., Village Drive), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, and Lanes.

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume for each approach.

Critical Gap Module: Table showing Critical Gap and FollowUp Time for each approach.

Capacity Module: Table showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap for each approach.

Level Of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, Approach Del, and Approach LOS for each approach.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #18 Brentwood Blvd. @ Central Blvd / Sycamore Ave (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.520
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 39 Level Of Service: A

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Central Blvd / Sycamore Ave (S) with various movement and control details.

Volume Module: Table showing various volume adjustments like Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table showing Sat/Lane, Adjustment, Lanes, and Final Sat. values for different approaches.

Capacity Analysis Module: Table showing Vol/Sat, Crit Volume, and Crit Moves for different approaches.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #20 Central Street @ Second Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.478
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 44 Level Of Service: A

Table with columns for Street Name (Second Street, Central Boulevard), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include, Ignore), Rights, Min. Green, and Lanes.

Table for Volume Module showing various adjustment factors like Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Table for Saturation Flow Module showing Sat/Lane, Adjustment, Lanes, and Final Sat.

Table for Capacity Analysis Module showing Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #21 Central Blvd @ Walnut Blvd

Cycle (sec): 100 Critical Vol./Cap.(X): 0.519
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 47 Level Of Service: A

Table with columns for Street Name (Walnut Blvd, Central Blvd), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Table for Volume Module showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various approaches.

Table for Saturation Flow Module showing Sat/Lane, Adjustment, Lanes, and Final Sat. values for different approaches.

Table for Capacity Analysis Module showing Vol/Sat, Crit Volume, and Crit Moves for different approaches.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #22 Brentwood Blvd @ Pine Street

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: A[9.7]

Table with columns for Street Name (Brentwood Blvd, Pine Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, and Lanes.

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume for each approach.

Critical Gap Module: Table showing Critical Gap and FollowUpTime for each approach.

Capacity Module: Table showing Conflict Vol, Potent Cap., Move Cap., and Volume/Cap for each approach.

Level Of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS for each approach.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #23 Brentwood Blvd @ Maple Street

Average Delay (sec/veh): 4.4 Worst Case Level Of Service: B[12.6]

Table with columns for Street Name (Brentwood Blvd, Maple Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume, with values for each approach and movement.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim, with values for each approach and movement.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap, with values for each approach and movement.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS, with values for each approach and movement.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #24 Brentwood Blvd @ Oak Street

Cycle (sec): 100 Critical Vol./Cap.(X): 0.257
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 34 Level Of Service: A

Table with columns for Street Name (Brentwood Blvd, Oak St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Split Phase), Rights (Include), Min. Green (4, 10, 10), and Lanes (1, 0, 1, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume across various movement categories.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across various movement categories.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across various movement categories.

Level Of Service Computation Report
 CCTALOS Method (Base Volume Alternative)

 Intersection #25 Walnut Blvd @ Oak St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.501
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 46 Level Of Service: A

Street Name: Walnut Blvd						Oak St.						
Approach: North Bound			South Bound			East Bound			West Bound			
Movement: L - T - R			L - T - R			L - T - R			L - T - R			
Control: Protected			Protected			Split Phase			Split Phase			
Rights: Include			Include			Include			Include			
Min. Green:	5	7	7	0	7	7	5	5	5	5	5	5
Lanes:	1	0	1	0	1	0	0	0	1	0	0	1

Volume Module:

Base Vol:	11	248	290	350	392	26	4	6	6	206	7	129
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	11	248	290	350	392	26	4	6	6	206	7	129
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	11	248	290	350	392	26	4	6	6	206	7	129
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	11	248	290	350	392	26	4	6	6	206	7	129
RTOR Reduct:	0	0	206	0	0	0	0	0	0	0	0	129
RTOR Vol:	11	248	84	350	392	26	4	6	6	206	7	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	11	248	84	350	392	26	4	6	6	206	7	0

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	1.00	1.00	0.94	0.06	0.25	0.37	0.38	0.97	0.03	1.00
Final Sat.:	1650	1650	1650	1650	1547	103	413	619	619	1596	54	1650

Capacity Analysis Module:

Vol/Sat:	0.01	0.15	0.05	0.21	0.25	0.25	0.01	0.01	0.01	0.13	0.13	0.00
Crit Volume:	248			350			16			213		
Crit Moves:	****			****			****			****		

Level Of Service Computation Report
 CCTALOS Method (Base Volume Alternative)

 Intersection #26 Brentwood Blvd. @ Second St.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.273
 Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 26 Level Of Service: A

Street Name:	Brentwood Blvd.						Second St.								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected			Permitted			Permitted					
Rights:	Include			Include			Include			Include					
Min. Green:	4	8	8	4	8	8	4	4	4	4	4	4			
Lanes:	1	0	1	1	0	1	0	1	1	0	1	0	1	1	0

Volume Module:

Base Vol:	46	221	3	107	300	21	44	193	61	45	413	273
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	46	221	3	107	300	21	44	193	61	45	413	273
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	46	221	3	107	300	21	44	193	61	45	413	273
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	46	221	3	107	300	21	44	193	61	45	413	273
RTOR Reduct:	0	0	0	0	0	0	0	0	0	0	0	107
RTOR Vol:	46	221	3	107	300	21	44	193	61	45	413	166
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	46	221	3	107	300	21	44	193	61	45	413	166

Saturation Flow Module:

Sat/Lane:	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720	1720
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.97	0.03	1.00	1.87	0.13	1.00	1.52	0.48	1.00	2.00	1.00
Final Sat.:	1720	3394	46	1720	3215	225	1720	2614	826	1720	3440	1720

Capacity Analysis Module:

Vol/Sat:	0.03	0.07	0.07	0.06	0.09	0.09	0.03	0.07	0.07	0.03	0.12	0.10
Crit Volume:	112			107			44			207		
Crit Moves:	****			****			****			****		

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #27 Brentwood Blvd. @ Chestnut St.

Average Delay (sec/veh): 0.7 Worst Case Level Of Service: B[11.6]

Street Name:		Brentwood Blvd.				Chestnut St.			
Approach:	North Bound		South Bound		East Bound		West Bound		
Movement:	L	T	R	L	T	R	L	T	R
Control:	Uncontrolled		Uncontrolled		Stop Sign		Stop Sign		
Rights:	Include		Include		Include		Include		
Lanes:	0	0	0	1	0	0	0	0	0

Volume Module:

Base Vol:	0	203	9	18	367	0	0	0	0	14	0	12
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	203	9	18	367	0	0	0	0	14	0	12
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	203	9	18	367	0	0	0	0	14	0	12
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	203	9	18	367	0	0	0	0	14	0	12

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	4.2	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	6.4	6.5	6.2
FollowUpTim:	xxxxxx	xxxx	xxxxxx	2.3	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	3.5	4.0	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	212	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	611	611	208
Potent Cap.:	xxxx	xxxx	xxxxxx	1312	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	457	409	833
Move Cap.:	xxxx	xxxx	xxxxxx	1312	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	453	403	833
Volume/Cap:	xxxx	xxxx	xxxx	0.01	xxxx	xxxx	xxxx	xxxx	xxxx	0.03	0.00	0.01

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	0.0	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Control Del:	xxxxxx	xxxx	xxxxxx	7.8	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
LOS by Move:	*	*	*	A	*	*	*	*	*	*	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	573	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	0.0	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	0.1	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	7.8	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	11.6	xxxxxx
Shared LOS:	*	*	*	A	*	*	*	*	*	*	B	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			11.6		
ApproachLOS:	*			*			*			B		

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 CCTALOS Method (Base Volume Alternative)

 Intersection #28 Brentwood Blvd. @ Balfour Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.442
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 41 Level Of Service: A

Street Name:	Brentwood Blvd.						Balfour Rd.								
Approach:	North Bound			South Bound			East Bound			West Bound					
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected			Protected			Protected			Protected					
Rights:	Ignore			Include			Include			Include					
Min. Green:	8	10	10	6	10	10	5	5	5	4	5	5			
Lanes:	1	0	2	0	1	1	0	2	0	1	1	0	1	1	0

Volume Module:

Base Vol:	172	132	18	43	237	132	181	386	335	52	292	65
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	172	132	18	43	237	132	181	386	335	52	292	65
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	172	132	18	43	237	132	181	386	335	52	292	65
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	172	132	18	43	237	132	181	386	335	52	292	65
RTOR Reduct:	0	0	0	0	0	132	0	0	172	0	0	0
RTOR Vol:	172	132	18	43	237	0	181	386	163	52	292	65
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	172	132	18	43	237	0	181	386	163	52	292	65

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	1.00	1.00	1.00	1.64	0.36
Final Sat.:	1650	3300	1650	1650	3300	1650	1650	1650	1650	1650	2699	601

Capacity Analysis Module:

Vol/Sat:	0.10	0.04	0.01	0.03	0.07	0.00	0.11	0.23	0.10	0.03	0.11	0.11
Crit Volume:	172			119			386			52		
Crit Moves:	****			****			****			****		

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #29 Sellers Ave. @ Sunet Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.722
 Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 18.5
 Optimal Cycle: 0 Level Of Service: C

Street Name:	Sellers Ave.						Sunset Rd.					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0	0	0	1! 0 0

Volume Module:

Base Vol:	102	297	18	29	239	53	92	168	114	6	69	27
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	102	297	18	29	239	53	92	168	114	6	69	27
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	102	297	18	29	239	53	92	168	114	6	69	27
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	102	297	18	29	239	53	92	168	114	6	69	27
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	102	297	18	29	239	53	92	168	114	6	69	27

Saturation Flow Module:

Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.24	0.72	0.04	0.09	0.74	0.17	0.25	0.45	0.30	0.06	0.68	0.26
Final Sat.:	141	411	25	50	415	92	138	252	171	26	297	116

Capacity Analysis Module:

Vol/Sat:	0.72	0.72	0.72	0.58	0.58	0.58	0.67	0.67	0.67	0.23	0.23	0.23
Crit Moves:	****			****			****			****		
Delay/Veh:	21.6	21.6	21.6	16.1	16.1	16.1	19.0	19.0	19.0	11.3	11.3	11.3
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	21.6	21.6	21.6	16.1	16.1	16.1	19.0	19.0	19.0	11.3	11.3	11.3
LOS by Move:	C	C	C	C	C	C	C	C	C	B	B	B
ApproachDel:	21.6			16.1			19.0			11.3		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	21.6			16.1			19.0			11.3		
LOS by Appr:	C			C			C			B		
AllWayAvgQ:	2.0	2.0	2.0	1.1	1.1	1.1	1.5	1.5	1.5	0.2	0.2	0.2

Note: Queue reported is the number of cars per lane.

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	E Sims Rd. and Delta Rd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative+PP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1090 veh/h Directional split 70 / 30 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1197	
v_p * highest directional split proportion ² (pc/h)		838	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	40 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	40.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		2.1	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		28.6	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1197	
v_p * highest directional split proportion ² (pc/h)		838	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		65.1	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		10.4	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		75.5	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.37	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		89	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	327
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	3.1
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	E Sims Rd. and Delta Rd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The diagram labels: Shoulder width (ft), Lane width (ft), Lane width (ft), Shoulder width (ft), and Segment length, L_i (mi).</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1487 veh/h Directional split 63 / 37 Peak-hour factor, PHF 0.96 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1564	
v_p * highest directional split proportion ² (pc/h)		985	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	40 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	40.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		1.5	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		26.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1549	
v_p * highest directional split proportion ² (pc/h)		976	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		74.4	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(\text{Exh. 20-12})$		7.2	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		81.6	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.49	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		116	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	446
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	4.4
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

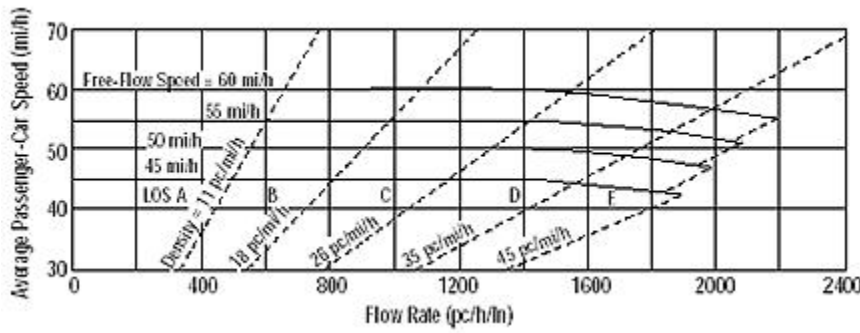
TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	Grant St. to Sunset Ct.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1609 veh/h Directional split 52 / 48 Peak-hour factor, PHF 0.97 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 10	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1675	
v_p * highest directional split proportion ² (pc/h)		871	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		1.4	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		15.6	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		1659	
v_p * highest directional split proportion ² (pc/h)		863	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		76.7	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		6.4	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		83.1	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.52	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		41	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	161
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	2.6
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Brentwood Blvd.
Agency or Company		From/To	Grant St. to Sunset Ct.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The diagram labels: Shoulder width (ft), Lane width (ft), Lane width (ft), Shoulder width (ft), and Segment length, L_i (mi).</p>		<input checked="" type="checkbox"/> Class I highway <input type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 2152 veh/h Directional split 52 / 48 Peak-hour factor, PHF 0.90 No-passing zone 100 % Trucks and Buses, P_T 10 % % Recreational vehicles, P_R 0 % Access points/ mi 10	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.990	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		2415	
v_p * highest directional split proportion ² (pc/h)		1256	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		1.1	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		10.2	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.0	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		2391	
v_p * highest directional split proportion ² (pc/h)		1243	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		87.8	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		3.1	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		90.8	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		E	
Volume to capacity ratio, $v/c=V_p/3,200$		0.75	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		60	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	215
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	5.9
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Central Blvd. to Spruce S
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	279	Peak-Hour Factor, PHF	0.90
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

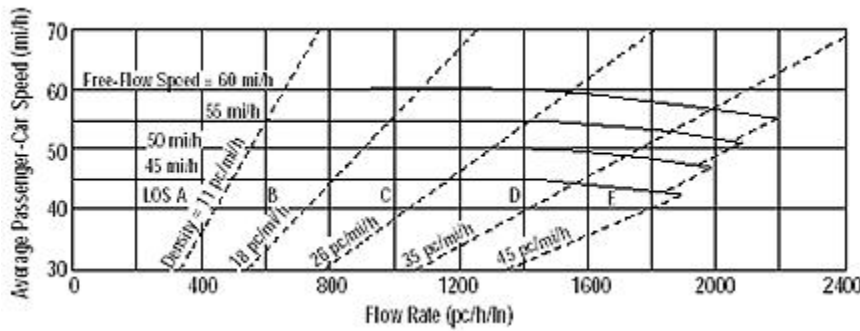
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 162
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 3.6
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v _p	L
Design (N)	FFS, LOS, v _p	N
Design (v _p)	FFS, LOS, N	v _i
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v _p)	FFS, LOS, N	v _i

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Brentwood Blvd.
Agency or Company		From/To	Central Blvd. to Spruce S
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative+PP

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	335	Peak-Hour Factor, PHF	0.95
AADT(veh/h)		%Trucks and Buses, P _T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

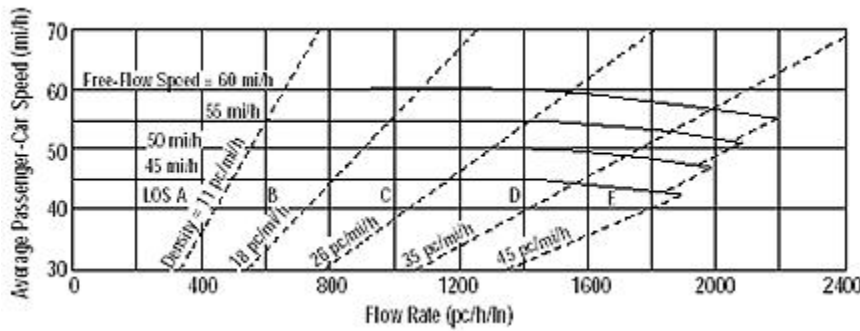
Calculate Flow Adjustments			
f _p	1.00	E _R	1.2
E _T	1.5	f _{HV}	0.952

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f _{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f _{LC} (mi/h)	
Access Points, A (A/mi)	0	f _A (mi/h)	
Median Type, M		f _M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v _p (pc/h/ln)	185	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v _p (pc/h)	
D (pc/mi/ln)	4.1	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v _p	L
Design (N)	FFS, LOS, v _p	N
Design (v _p)	FFS, LOS, N	v _i
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v _p)	FFS, LOS, N	v _i

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Central Blvd. to Spruce S
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	530	Peak-Hour Factor, PHF	0.78
AADT(veh/h)		%Trucks and Buses, P _T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f _p	1.00	E _R	1.2
E _T	1.5	f _{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

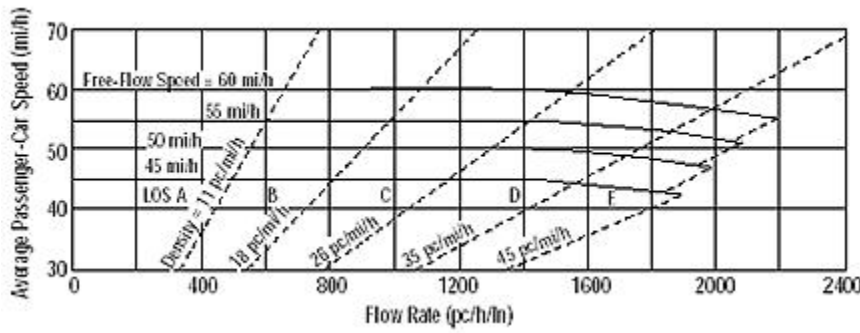
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 356
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 7.9
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Central Blvd. to Spruce S
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	429	Peak-Hour Factor, PHF	0.99
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft)	12.0
Total Lateral Clearance, LC (ft)	12.0
Access Points, A (A/mi)	0
Median Type, M	
FFS (measured)	45.0
Base Free-Flow Speed, BFFS	

Calc Speed Adj and FFS

f_{LW} (mi/h)	
f_{LC} (mi/h)	
f_A (mi/h)	
f_M (mi/h)	
FFS (mi/h)	45.0

Operations

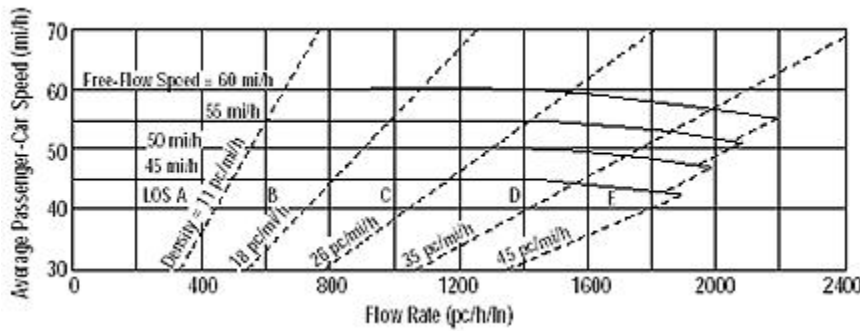
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 227
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 5.0
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Pine Ave. to Oak Ave.
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	280	Peak-Hour Factor, PHF	0.90
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

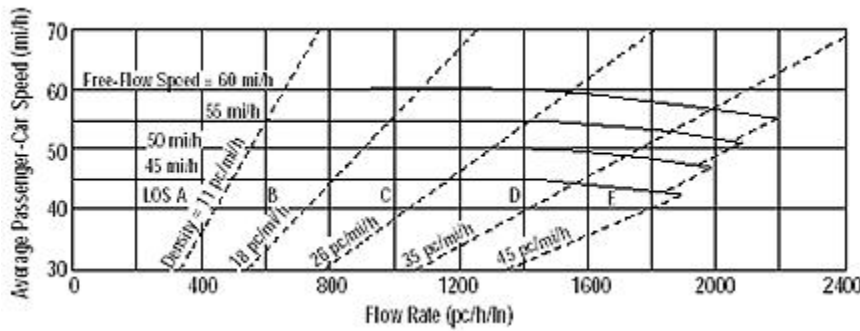
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 163
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 3.6
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Pine Ave. to Oak Ave.
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	150	Peak-Hour Factor, PHF	0.88
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

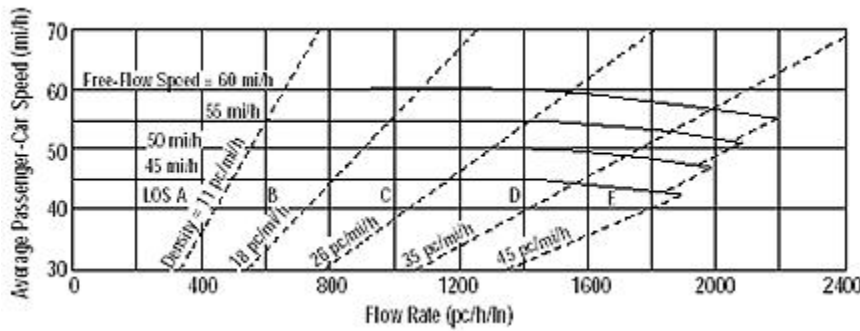
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 89
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 2.0
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Brentwood Blvd.
 From/To: Pine Ave. to Oak Ave.
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	270	Peak-Hour Factor, PHF	0.97
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

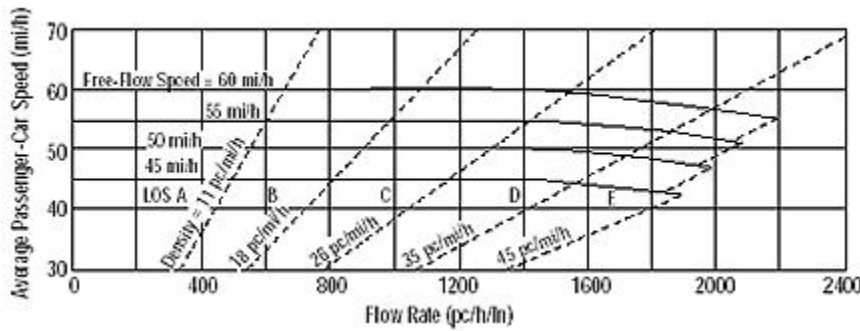
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 146
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 3.2
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Brentwood Blvd.
Agency or Company		From/To	Pine Ave. to Oak Ave.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative+PP

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	330	Peak-Hour Factor, PHF	0.97
AADT(veh/h)		%Trucks and Buses, P_T	10
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.952

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	178	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	4.0	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Delta Rd.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative+PP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i mi. The diagram includes two lanes, each with a width of Lane width ft, and two shoulders, each with a width of Shoulder width ft. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 629 veh/h Directional split 65 / 35 Peak-hour factor, PHF 0.76 No-passing zone 0 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 40	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		831	
v_p * highest directional split proportion ² (pc/h)		540	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	50 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	50.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		0.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		43.6	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		829	
v_p * highest directional split proportion ² (pc/h)		539	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		51.7	
Adj. for directional distribution and no-passing zone, $f_{dnp}(\%)(\text{Exh. 20-12})$		0.0	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{dnp}$		51.7	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		B	
Volume to capacity ratio, $v/c=V_p/3,200$		0.26	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		207	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	629
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	4.8
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Delta Rd.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative+PP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 824 veh/h Directional split 65 / 35 Peak-hour factor, PHF 0.91 No-passing zone 0 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 40	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		909	
v_p * highest directional split proportion ² (pc/h)		591	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	50 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	50.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		0.0	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		42.9	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		907	
v_p * highest directional split proportion ² (pc/h)		590	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		54.9	
Adj. for directional distribution and no-passing zone, $f_{dnp}(\%)(\text{Exh. 20-12})$		0.0	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{dnp}$		54.9	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		B	
Volume to capacity ratio, $v/c=V_p/3,200$		0.28	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		226	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	824
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	5.3
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

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 Roseville, CA 95747
 Phone:
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 OPERATIONAL ANALYSIS

Analyst: Kimley-Horn and Associates
 Agency/Co:
 Date: 4/14/08
 Analysis Period: AM Peak
 Highway: Lone Tree
 From/To: West of Brentwood
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative Plus Project
 Project ID:

 FREE-FLOW SPEED

Direction	1		2	
Lane width	12.0	ft	12.0	ft
Lateral clearance:				
Right edge	6.0	ft	6.0	ft
Left edge	6.0	ft	6.0	ft
Total lateral clearance	12.0	ft	12.0	ft
Access points per mile	0		0	
Median type	Divided		Divided	
Free-flow speed:	Base		Base	
FFS or BFFS	50.0	mph	50.0	mph
Lane width adjustment, FLW	0.0	mph	0.0	mph
Lateral clearance adjustment, FLC	0.0	mph	0.0	mph
Median type adjustment, FM	0.0	mph	0.0	mph
Access points adjustment, FA	0.0	mph	0.0	mph
Free-flow speed	50.0	mph	50.0	mph

 VOLUME

Direction	1		2	
Volume, V	940	vph	867	vph
Peak-hour factor, PHF	0.92		0.92	
Peak 15-minute volume, v15	255		236	
Trucks and buses	2	%	2	%
Recreational vehicles	4	%	4	%
Terrain type	Level		Level	
Grade	0.00	%	0.00	%
Segment length	0.00	mi	0.00	mi
Number of lanes	2		2	
Driver population adjustment, fP	1.00		1.00	
Trucks and buses PCE, ET	1.5*		1.5	
Recreational vehicles PCE, ER	1.2		1.2	
Heavy vehicle adjustment, fHV	0.982		0.982	
Flow rate, vp	520	pcphpl	479	pcphpl

 RESULTS

	Direction	1		2	
Flow rate, vp		520	pcphp1	479	pcphp1
Free-flow speed, FFS		50.0	mph	50.0	mph
Avg. passenger-car travel speed, S		50.0	mph	50.0	mph
Level of service, LOS		A		A	
Density, D		10.4	pc/mi/ln	9.6	pc/mi/ln

Overall results are not computed when free-flow speed is less than 45 mph.

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 OPERATIONAL ANALYSIS

Analyst: Kimley-Horn and Associates
 Agency/Co:
 Date: 4/14/08
 Analysis Period: PM Peak
 Highway: Lone Tree
 From/To: West of Brentwood
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative Plus Project
 Project ID:

 FREE-FLOW SPEED

Direction	1		2	
Lane width	12.0	ft	12.0	ft
Lateral clearance:				
Right edge	6.0	ft	6.0	ft
Left edge	6.0	ft	6.0	ft
Total lateral clearance	12.0	ft	12.0	ft
Access points per mile	0		0	
Median type	Divided		Divided	
Free-flow speed:	Base		Base	
FFS or BFFS	50.0	mph	50.0	mph
Lane width adjustment, FLW	0.0	mph	0.0	mph
Lateral clearance adjustment, FLC	0.0	mph	0.0	mph
Median type adjustment, FM	0.0	mph	0.0	mph
Access points adjustment, FA	0.0	mph	0.0	mph
Free-flow speed	50.0	mph	50.0	mph

 VOLUME

Direction	1		2	
Volume, V	1119	vph	1212	vph
Peak-hour factor, PHF	0.92		0.92	
Peak 15-minute volume, v15	304		329	
Trucks and buses	2	%	2	%
Recreational vehicles	4	%	4	%
Terrain type	Level		Level	
Grade	0.00	%	0.00	%
Segment length	0.00	mi	0.00	mi
Number of lanes	2		2	
Driver population adjustment, fP	1.00		1.00	
Trucks and buses PCE, ET	1.5*		1.5	
Recreational vehicles PCE, ER	1.2		1.2	
Heavy vehicle adjustment, fHV	0.982		0.982	
Flow rate, vp	619	pcphpl	670	pcphpl

 RESULTS

	Direction	1		2	
Flow rate, vp		619	pcphpl	670	pcphpl
Free-flow speed, FFS		50.0	mph	50.0	mph
Avg. passenger-car travel speed, S		50.0	mph	50.0	mph
Level of service, LOS		B		B	
Density, D		12.4	pc/mi/ln	13.4	pc/mi/ln

Overall results are not computed when free-flow speed is less than 45 mph.

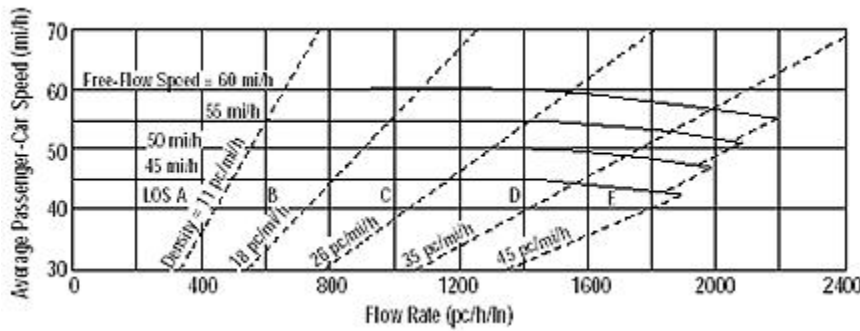
TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sunset Ave
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative+PP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 568 veh/h Directional split 63 / 37 Peak-hour factor, PHF 0.88 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		648	
v_p * highest directional split proportion ² (pc/h)		408	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.7	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		21.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		647	
v_p * highest directional split proportion ² (pc/h)		408	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		43.4	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		19.3	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		62.7	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.20	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		161	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	568
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	7.6
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sunset Ave
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 614 veh/h Directional split 57 / 43 Peak-hour factor, PHF 0.88 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 20	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.2	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		701	
v_p * highest directional split proportion ² (pc/h)		400	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.4	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		21.1	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		699	
v_p * highest directional split proportion ² (pc/h)		398	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		45.9	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		17.7	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		63.6	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.22	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		174	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	614
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	8.2
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Central Blvd.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	556	Peak-Hour Factor, PHF	0.86
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

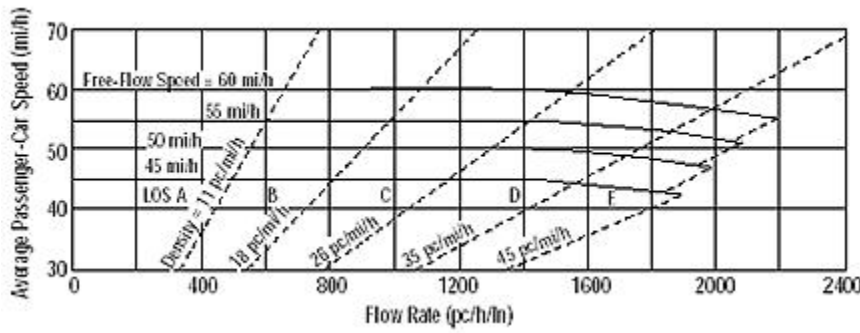
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 326
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 7.2
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Central Blvd.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	386	Peak-Hour Factor, PHF	0.88
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

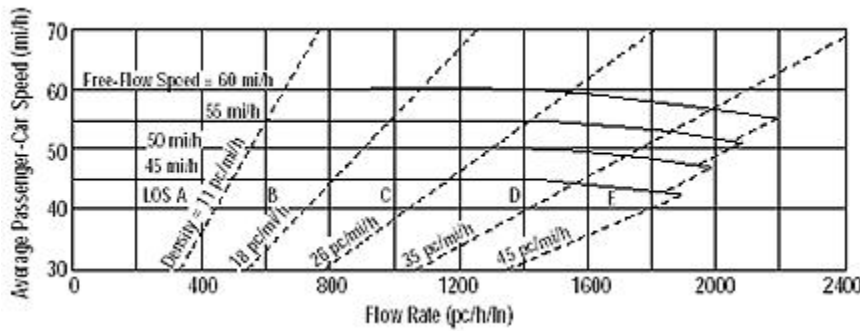
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 221
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 4.9
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Central Blvd.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative+PP

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	501	Peak-Hour Factor, PHF	0.84
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

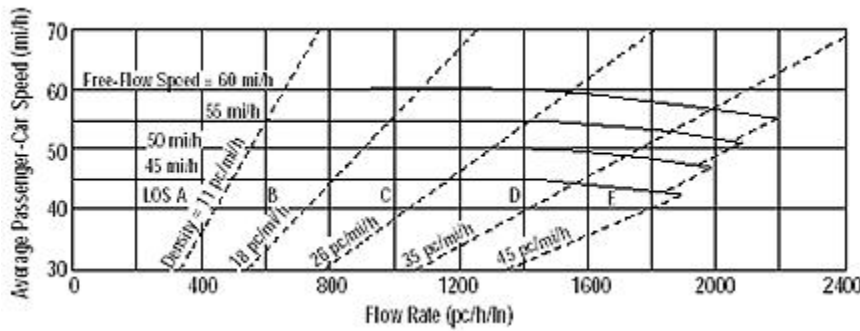
Calculate Flow Adjustments			
f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f_{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	
Access Points, A (A/mi)	0	f_A (mi/h)	
Median Type, M		f_M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v_p (pc/h/ln)	301	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v_p (pc/h)	
D (pc/mi/ln)	6.7	Max Service Flow Rate (pc/h/ln)	
LOS	A	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	C
Operational (LOS)	FFS, N, v_p	L
Design (N)	FFS, LOS, v_p	N
Design (v_p)	FFS, LOS, N	v_f
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v_p)	FFS, LOS, N	v_f

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/7/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Central Blvd.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Cumulative+PP

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	879	Peak-Hour Factor, PHF	0.76
AADT(veh/h)		%Trucks and Buses, P_T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f_p	1.00	E_R	1.2
E_T	1.5	f_{HV}	0.990

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 584
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 13.0
 LOS: B

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Grant St.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 280 veh/h Directional split 73 / 27 Peak-hour factor, PHF 0.71 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 7	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		400	
v_p * highest directional split proportion ² (pc/h)		292	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		4.5	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		22.4	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		395	
v_p * highest directional split proportion ² (pc/h)		288	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		29.3	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		24.7	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		54.0	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		B	
Volume to capacity ratio, $v/c=V_p/3,200$		0.13	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		99	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	280
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	4.4
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Grant St.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 217 veh/h Directional split 63 / 37 Peak-hour factor, PHF 0.87 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 7	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		253	
v_p * highest directional split proportion ² (pc/h)		159	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		3.8	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		24.3	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		250	
v_p * highest directional split proportion ² (pc/h)		158	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		19.7	
Adj. for directional distribution and no-passing zone, f_{dnp} (%) (Exh. 20-12)		23.8	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{dnp}$		43.5	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		B	
Volume to capacity ratio, $v/c=V_p/3,200$		0.08	
Peak 15-min veh-miles of travel, VMT_{15} (veh-mi)= $0.25L_i(V/PHF)$		62	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	217
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	2.6
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sycamore Ave.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Cumulative+PP
Project Description:			
Input Data			
<p>Diagram showing a cross-section of a two-way two-lane highway segment. It includes two lanes with arrows indicating traffic flow in opposite directions. On either side of the lanes are shoulders. The segment length is labeled as L_i in miles.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 413 veh/h Directional split 78 / 22 Peak-hour factor, PHF 0.87 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 35	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		481	
v_p * highest directional split proportion ² (pc/h)		375	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		4.3	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		22.0	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		476	
v_p * highest directional split proportion ² (pc/h)		371	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		34.2	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(Exh. 20-12)$		25.8	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		60.0	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.15	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		59	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	207
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	2.7
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET			
General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway	Sycamore Ave.
Agency or Company		From/To	East of Brentwood Blvd.
Date Performed	11/7/2007	Jurisdiction	City of Brentwood
Analysis Time Period	PM Peak	Analysis Year	Cumulative+PP
Project Description:			
Input Data			
<p>Diagram showing a two-way two-lane highway segment. The segment length is L_i in miles. The diagram includes two lanes, each with a width, and two shoulders, each with a width. Arrows indicate traffic flow in both directions.</p>		<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 442 veh/h Directional split 75 / 25 Peak-hour factor, PHF 0.82 No-passing zone 100 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 4% Access points/ mi 35	
Average Travel Speed			
Grade adjustment factor, f_G (Exhibit 20-7)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)		1.7	
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.986	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		547	
v_p * highest directional split proportion ² (pc/h)		410	
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field Measured speed, S_{FM}	30 mi/h	Base free-flow speed, $BFFS_{FM}$	mi/h
Observed volume, V_f	0 veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5)	mi/h
Free-flow speed, FFS $FFS=S_{FM}+0.00776(V_f/f_{HV})$	mi/h	Adj. for access points, f_A (Exhibit 20-6)	mi/h
		Free-flow speed, FFS ($FSS=BFFS-f_{LS}-f_A$)	30.0 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)		4.1	
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$		21.7	
Percent Time-Spent-Following			
Grade Adjustment factor, f_G (Exhibit 20-8)		1.00	
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)		1.1	
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)		1.0	
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.998	
Two-way flow rate ¹ , v_p (pc/h)= $V/(PHF * f_G * f_{HV})$		540	
v_p * highest directional split proportion ² (pc/h)		405	
Base percent time-spent-following, $BPTSF(\%)=100(1-e^{-0.000879v_p})$		37.8	
Adj. for directional distribution and no-passing zone, $f_{d/np}(\%)(\text{Exh. 20-12})$		23.6	
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{d/np}$		61.4	
Level of Service and Other Performance Measures			
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)		C	
Volume to capacity ratio, $v/c=V_p/3,200$		0.17	
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi})=0.25L_i(V/PHF)$		67	

Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_t$	221
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / \text{ATS}$	3.1
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated anlysis-the LOS is F.	

Appendix F:

*Analysis Worksheets for
Mitigated Conditions*

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

 Intersection #4 Brentwood Blvd @ Lone Tree Way (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.720
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 81 Level Of Service: C

Street Name:	Brentwood Blvd						Lone Tree Way (S)					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Ignore			Include		
Min. Green:	4	8	8	4	8	8	4	4	4	4	4	4
Lanes:	2	0	2	0	1	1	1	0	1	0	1	0

Volume Module:

Base Vol:	300	536	23	11	604	196	135	8	140	56	10	9
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	300	536	23	11	604	196	135	8	140	56	10	9
Added Vol:	260	729	250	111	369	40	228	113	324	44	20	19
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	560	1265	273	122	973	236	363	121	464	100	30	28
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	560	1265	273	122	973	236	363	121	464	100	30	28
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	560	1265	273	122	973	236	363	121	464	100	30	28
RTOR Reduct:	0	0	100	0	0	236	0	0	0	0	0	28
RTOR Vol:	560	1265	173	122	973	0	363	121	464	100	30	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Volume:	560	1265	173	122	973	0	363	121	464	100	30	0

Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	0.91	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	2.00	1.00	1.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Sat.:	3000	3300	1650	1650	3300	1650	1650	1650	1650	1650	1650	1650

Capacity Analysis Module:

Vol/Sat:	0.19	0.38	0.10	0.07	0.29	0.00	0.22	0.07	0.28	0.06	0.02	0.00
Crit Volume:	280			487			363			30		
Crit Moves:	****			****			****			****		

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #5 Arroyo Seco Road @ Lone Tree Way

Cycle (sec): 100 Critical Vol./Cap.(X): 0.642
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 13.3
Optimal Cycle: 0 Level Of Service: B

Table with columns for Street Name (Arroyo Seco Road, Lone Tree Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Stop Sign), Rights (Include), Min. Green, and Lanes.

Table for Volume Module showing Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and FinalVolume across various lanes.

Table for Saturation Flow Module showing Adjustment, Lanes, and Final Sat. values.

Table for Capacity Analysis Module showing Vol/Sat, Crit Moves, Delay/Veh, Delay Adj, AdjDel/Veh, LOS by Move, ApproachDel, Delay Adj, ApprAdjDel, LOS by Appr, and AllWayAvgQ.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #6 Brentwood Blvd. @ Sunrise Drive

Cycle (sec): 100 Critical Vol./Cap.(X): 0.748

Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx

Optimal Cycle: 90 Level Of Service: C

Street Name: Brentwood Blvd. Sunrise Dr.

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

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Control: Protected Protected Protected Protected

Rights: Include Include Ignore Include

Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0 0

Lanes: 1 0 2 0 0 0 0 2 0 0 1 0 0 0 0 0

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Volume Module:

Base Vol: 15 959 0 0 840 1 14 0 11 0 0 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 15 959 0 0 840 1 14 0 11 0 0 0

Added Vol: 15 1202 0 0 728 8 37 0 17 0 0 0

PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0

Initial Fut: 30 2161 0 0 1568 9 51 0 28 0 0 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 0.87 0.87 0.87 0.93 0.93 0.93 0.75 0.75 0.75 1.00 1.00 1.00

PHF Volume: 35 2498 0 0 1693 10 68 0 37 0 0 0

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0

Reduced Vol: 35 2498 0 0 1693 10 68 0 37 0 0 0

RTOR Reduct: 0 0 0 0 0 10 0 0 0 0 0 0

RTOR Vol: 35 2498 0 0 1693 0 68 0 37 0 0 0

PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

FinalVolume: 35 2498 0 0 1693 0 68 0 37 0 0 0

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Saturation Flow Module:

Sat/Lane: 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720

Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 0.91 1.00 1.00 1.00 1.00 1.00

Lanes: 1.00 2.00 0.00 0.00 2.00 1.00 2.00 0.00 1.00 0.00 0.00 0.00

Final Sat.: 1720 3440 0 0 3440 1720 3127 0 1720 0 0 0

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Capacity Analysis Module:

Vol/Sat: 0.02 0.73 0.00 0.00 0.49 0.00 0.02 0.00 0.02 0.00 0.00 0.00

Crit Volume: 1249 0 34 0

Crit Moves: **** **** ****

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #7 Brentwood Blvd. @ Gregory Lane

Average Delay (sec/veh): 0.2 Worst Case Level Of Service: C[19.3]

Table with columns for Street Name (Brentwood Blvd., Gregory Ln.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 2, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #9 Brentwood Blvd. @ Homecoming Way

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: E[36.1]

Table with columns for Street Name (Brentwood Blvd., Homecoming Way), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across various movements.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim, showing values like 6.9 and 3.3.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap, showing values like 1241, 166, 166, and 0.31.

Level of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS, showing values like 1.2, 36.1, and E.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #10 Brentwood Blvd. @ Grant Street / Sunset Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.784
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 106 Level Of Service: C

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Grant Street / Sunset Rd. with sub-rows for North, South, East, and West bounds.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and rows for different street directions.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. for different street directions.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves for different street directions.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #11 Brentwood Blvd. @ Sunset Ct.

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: D[32.0]

Table with columns for Street Name (Brentwood Blvd., Sunset Ct.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 2).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #12 Brentwood Blvd. @ Havenwood Ave.

Average Delay (sec/veh): 1.0 Worst Case Level Of Service: E[35.3]

Table with columns for Street Name, Approach, Movement, Control, Rights, and Lanes. Rows include Brentwood Blvd. and Havenwood Ave. with various movement details like Uncontrolled, Stop Sign, and lane counts.

Volume Module: Table showing traffic volume data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across different movements.

Critical Gap Module: Table showing Critical Gap and FollowUpTim values for different movements.

Capacity Module: Table showing Capacity-related metrics like Cnflict Vol, Potent Cap., Move Cap., and Volume/Cap. for various movements.

Level of Service Module: Table showing Level of Service (LOS) and ApproachDel values for different movements and approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #13 Brentwood Blvd. @ Applewood Common

Cycle (sec): 100 Critical Vol./Cap.(X): 0.759
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 95 Level Of Service: C

Table with columns for Street Name (Brentwood Blvd., Applewood Common), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Ignore, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and values for each approach.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. values for each approach.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves values for each approach.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #14 Brentwood Blvd. @ Sand Creek Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.762
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 78 Level Of Service: C

Table with columns for Street Name (Brentwood Blvd., Sand Creek Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include, Ignore), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and values for each approach.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. values for each approach.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves values for each approach.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #17 Brentwood Blvd. @ Village Drive

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: C[16.1]

Table with columns for Street Name (Brentwood Blvd., Village Drive), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 0, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume across various movements.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim, showing values like 6.2 and 3.3.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap, showing values like 778, 397, 397, and 0.18.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS, showing values like 0.6, 16.1, and C.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #18 Brentwood Blvd. @ Central Blvd / Sycamore Ave (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.644
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 52 Level Of Service: B

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Central Blvd / Sycamore Ave (S) with North, South, East, and West bound movements.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and rows for different approaches.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. for different approaches.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves for different approaches.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #22 Brentwood Blvd @ Pine Street

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: B[13.9]

Table with columns for Street Name (Brentwood Blvd, Pine Street), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control, Rights, and Lanes.

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across different movements.

Critical Gap Module: Table showing Critical Gp and FollowUpTim values for various movements.

Capacity Module: Table showing Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap. for different movements.

Level of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #27 Brentwood Blvd. @ Chestnut St.

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: C[19.6]

Table with columns for Street Name (Brentwood Blvd., Chestnut St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across four approaches.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across four approaches.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across four approaches.

Level of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across four approaches.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #28 Brentwood Blvd. @ Balfour Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.623
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 60 Level Of Service: B

Table with columns for Street Name (Brentwood Blvd., Balfour Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and rows for different approaches.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across different approaches.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across different approaches.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #4 Brentwood Blvd @ Lone Tree Way (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.800
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 114 Level Of Service: D

Table with columns for Street Name (Brentwood Blvd, Lone Tree Way (S)), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Ignore, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and rows for different approaches.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. across different approaches.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves across different approaches.

Level Of Service Computation Report

2000 HCM 4-Way Stop Method (Future Volume Alternative)

Intersection #5 Arroyo Seco Road @ Lone Tree Way

Cycle (sec): 100 Critical Vol./Cap.(X): 0.464

Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): 9.7

Optimal Cycle: 0 Level Of Service: A

Street Name: Arroyo Seco Road Lone Tree Way

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

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Control: Stop Sign Stop Sign Stop Sign Stop Sign

Rights: Include Include Include Include

Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0 0

Lanes: 0 0 1! 0 0 0 0 0 0 1 0 0 1 0 0

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Volume Module:

Base Vol: 29 0 1 0 0 0 0 0 25 47 0 16 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 29 0 1 0 0 0 0 0 25 47 0 16 0

Added Vol: 0 0 0 0 0 0 423 106 0 0 0 0 0

PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0

Initial Fut: 29 0 1 0 0 423 106 25 47 0 16 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Volume: 29 0 1 0 0 423 106 25 47 0 16 0

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0

Reduced Vol: 29 0 1 0 0 423 106 25 47 0 16 0

PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

FinalVolume: 29 0 1 0 0 423 106 25 47 0 16 0

-----|-----|-----|-----|-----|

Saturation Flow Module:

Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Lanes: 0.97 0.00 0.03 0.00 0.00 1.00 0.60 0.14 0.26 0.00 1.00 0.00

Final Sat.: 661 0 23 0 0 911 417 98 185 0 648 0

-----|-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat: 0.04 xxxx 0.04 xxxx xxxx 0.46 0.25 0.25 0.25 xxxx 0.02 xxxx

Crit Moves: **** **** ****

Delay/Veh: 8.2 0.0 8.2 0.0 0.0 10.0 9.3 9.3 9.3 0.0 8.2 0.0

Delay Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

AdjDel/Veh: 8.2 0.0 8.2 0.0 0.0 10.0 9.3 9.3 9.3 0.0 8.2 0.0

LOS by Move: A * A * * A A A A * A *

ApproachDel: 8.2 10.0 9.3 8.2

Delay Adj: 1.00 1.00 1.00 1.00

ApprAdjDel: 8.2 10.0 9.3 8.2

LOS by Appr: A A A A

AllWayAvgQ: 0.0 0.0 0.0 0.8 0.8 0.8 0.3 0.3 0.3 0.0 0.0 0.0

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #6 Brentwood Blvd. @ Sunrise Drive

Cycle (sec): 100 Critical Vol./Cap.(X): 0.898
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: D

Street Name: Brentwood Blvd. Sunrise Dr.

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Protected Protected Protected Protected

Rights: Include Include Ignore Include

Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0

Lanes: 1 0 2 0 0 0 0 2 0 0 0 1 0 0 0 0 0

-----|-----|-----|-----|

Volume Module:

Base Vol: 66 1253 0 0 1148 23 45 0 21 0 0 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 66 1253 0 0 1148 23 45 0 21 0 0 0

Added Vol: 41 848 0 0 1212 22 34 0 16 0 0 0

PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0

Initial Fut: 107 2101 0 0 2360 45 79 0 37 0 0 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 0.85 0.85 0.85 0.87 0.87 0.87 0.68 0.68 0.68 1.00 1.00 1.00

PHF Volume: 126 2469 0 0 2710 52 116 0 54 0 0 0

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0

Reduced Vol: 126 2469 0 0 2710 52 116 0 54 0 0 0

RTOR Reduct: 0 0 0 0 0 52 0 0 0 0 0 0

RTOR Vol: 126 2469 0 0 2710 0 116 0 54 0 0 0

PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

FinalVolume: 126 2469 0 0 2710 0 116 0 54 0 0 0

-----|-----|-----|-----|

Saturation Flow Module:

Sat/Lane: 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720

Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 0.91 1.00 1.00 1.00 1.00 1.00

Lanes: 1.00 2.00 0.00 0.00 2.00 1.00 2.00 0.00 1.00 0.00 0.00 0.00

Final Sat.: 1720 3440 0 0 3440 1720 3127 0 1720 0 0 0

-----|-----|-----|-----|

Capacity Analysis Module:

Vol/Sat: 0.07 0.72 0.00 0.00 0.79 0.00 0.04 0.00 0.03 0.00 0.00 0.00

Crit Volume: 126 1355 58 0

Crit Moves: **** **** ****

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #7 Brentwood Blvd. @ Gregory Lane

Average Delay (sec/veh): 0.6 Worst Case Level Of Service: F[56.9]

Table with columns for Street Name (Brentwood Blvd., Gregory Ln.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 2, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #9 Brentwood Blvd. @ Homecoming Way

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: E[38.6]

Street Name:	Brentwood Blvd.				Homecoming Way															
Approach:	North Bound		South Bound		East Bound		West Bound													
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Uncontrolled		Uncontrolled		Stop Sign		Stop Sign													
Rights:	Include		Include		Include		Include													
Lanes:	0	0	1	1	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	1

Volume Module:

Base Vol:	0	1192	20	0	1194	0	0	0	0	0	0	0	6
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Initial Bse:	0	1192	20	0	1194	0	0	0	0	0	0	0	6
Added Vol:	0	886	24	0	1215	0	0	0	0	0	0	0	14
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	2078	44	0	2409	0	0	0	0	0	0	0	20
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
PHF Adj:	0.82	0.82	0.82	0.86	0.86	0.86	1.00	1.00	1.00	0.42	0.42	0.42	
PHF Volume:	0	2528	54	0	2811	0	0	0	0	0	0	0	48
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	2528	54	0	2811	0	0	0	0	0	0	0	48

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	6.9
FollowUpTim:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	1291
Potent Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	154
Move Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	154
Volume/Cap:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.31

Level of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	1.2
Control Del:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	38.6
LOS by Move:	*	*	*	*	*	*	*	*	*	*	*	E
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx		xxxxxx		xxxxxx		xxxxxx		xxxxxx		38.6	
ApproachLOS:	*		*		*		*		*		E	

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #10 Brentwood Blvd. @ Grant Street / Sunset Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.851
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 153 Level Of Service: D

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Grant Street / Sunset Rd. with sub-rows for North, South, East, and West bounds.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and rows for different street directions.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. for different street directions.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves for different street directions.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #11 Brentwood Blvd. @ Sunset Ct.

Average Delay (sec/veh): 0.6 Worst Case Level Of Service: E[43.9]

Street Name: Brentwood Blvd. Sunset Ct.

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, and Lanes.

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume for each movement.

Critical Gap Module: Table showing Critical Gp and FollowUpTim for each movement.

Capacity Module: Table showing Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap for each movement.

Level Of Service Module: Table showing 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #12 Brentwood Blvd. @ Havenwood Ave.

Average Delay (sec/veh): 0.8 Worst Case Level Of Service: E[42.8]

Table with columns for Street Name (Brentwood Blvd., Havenwood Ave.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 0, 2, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #13 Brentwood Blvd. @ Applewood Common

Cycle (sec): 100 Critical Vol./Cap.(X): 0.883
Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: D

Table with columns for Street Name (Brentwood Blvd., Applewood Common), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Ignore, Include), Rights, Min. Green, and Lanes.

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #14 Brentwood Blvd. @ Sand Creek Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.847
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 122 Level Of Service: D

Table with columns for Street Name (Brentwood Blvd., Sand Creek Rd.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Protected, Include, Ignore), Rights, Min. Green, and Lanes.

Table for Volume Module showing various traffic volume metrics such as Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, and Final Volume.

Table for Saturation Flow Module showing Sat/Lane, Adjustment, Lanes, and Final Sat. values.

Table for Capacity Analysis Module showing Vol/Sat, Crit Volume, and Crit Moves.

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #17 Brentwood Blvd. @ Village Drive

Average Delay (sec/veh): 0.5 Worst Case Level Of Service: C[18.6]

Table with columns for Street Name (Brentwood Blvd., Village Drive), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across various movements.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim, showing values like 6.2 and 3.3.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap., showing values like 850, 360, 360, and 0.27.

Level Of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS, showing values like 1.1, 18.6, and C.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
CCTALOS Method (Future Volume Alternative)

Intersection #18 Brentwood Blvd. @ Central Blvd / Sycamore Ave (S)

Cycle (sec): 100 Critical Vol./Cap.(X): 0.675
Loss Time (sec): 9 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 57 Level Of Service: B

Table with columns for Street Name, Approach, Movement, Control, Rights, Min. Green, and Lanes. Rows include Brentwood Blvd. and Central Blvd / Sycamore Ave (S) with various movement and control details.

Volume Module: Table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, RTOR Reduct, RTOR Vol, PCE Adj, MLF Adj, Final Volume) and rows for different approaches.

Saturation Flow Module: Table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. for different approaches.

Capacity Analysis Module: Table with columns for Vol/Sat, Crit Volume, and Crit Moves for different approaches.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #22 Brentwood Blvd @ Pine Street

Average Delay (sec/veh): 0.1 Worst Case Level Of Service: B[13.5]

Street Name:	Brentwood Blvd			Pine Street																
Approach:	North Bound		South Bound		East Bound		West Bound													
Movement:	L	T	R	L	T	R	L	T	R	L	T	R								
Control:	Uncontrolled			Uncontrolled			Stop Sign			Stop Sign										
Rights:	Include			Include			Include			Include										
Lanes:	0	0	1	1	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	1

Volume Module:

Base Vol:	0	885	24	0	832	0	0	0	0	0	0	22
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	885	24	0	832	0	0	0	0	0	0	22
Added Vol:	0	282	0	0	513	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1167	24	0	1345	0	0	0	0	0	0	22
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1167	24	0	1345	0	0	0	0	0	0	22
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	1167	24	0	1345	0	0	0	0	0	0	22

Critical Gap Module:

Critical Gp:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	6.9
FollowUpTim:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	596
Potent Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	447
Move Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	447
Volume/Cap:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.05

Level of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	0.2
Control Del:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	13.5
LOS by Move:	*	*	*	*	*	*	*	*	*	*	*	B
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
SharedQueue:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shrd ConDel:	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx					13.5
ApproachLOS:	*			*			*					B

Note: Queue reported is the number of cars per lane.

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #27 Brentwood Blvd. @ Chestnut St.

Average Delay (sec/veh): 0.2 Worst Case Level Of Service: C[18.0]

Table with columns for Street Name (Brentwood Blvd., Chestnut St.), Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Uncontrolled, Stop Sign), Rights (Include), and Lanes (0, 1, 0, 0, 0, 0, 0, 1).

Volume Module: Table with columns for Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume across 12 lanes.

Critical Gap Module: Table with columns for Critical Gp and FollowUpTim across 12 lanes, with values 6.2 and 3.3.

Capacity Module: Table with columns for Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap across 12 lanes, with values 998, 296, 296, and 0.06.

Level of Service Module: Table with columns for 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS across 12 lanes, with values 0.2, 18.0, C, and 18.0.

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report
 CCTALOS Method (Future Volume Alternative)

 Intersection #28 Brentwood Blvd. @ Balfour Rd.

Cycle (sec): 100 Critical Vol./Cap.(X): 0.772
 Loss Time (sec): 12 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
 Optimal Cycle: 100 Level Of Service: C

Street Name:	Brentwood Blvd.					Balfour Rd.									
Approach:	North Bound		South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R			
Control:	Protected		Protected			Protected			Protected						
Rights:	Include		Include			Include			Include						
Min. Green:	8	10	10	6	10	10	5	5	5	4	5	5			
Lanes:	2	0	2	0	1	1	0	2	0	1	1	0	1	1	0

Volume Module:

Base Vol:	475	443	51	134	430	228	275	556	484	66	386	144
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	475	443	51	134	430	228	275	556	484	66	386	144
Added Vol:	0	183	5	40	335	0	0	0	0	8	0	24
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	475	626	56	174	765	228	275	556	484	74	386	168
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	475	626	56	174	765	228	275	556	484	74	386	168
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	475	626	56	174	765	228	275	556	484	74	386	168
RTOR Reduct:	0	0	56	0	0	228	0	0	261	0	0	0
RTOR Vol:	475	626	0	174	765	0	275	556	223	74	386	168
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	475	626	0	174	765	0	275	556	223	74	386	168

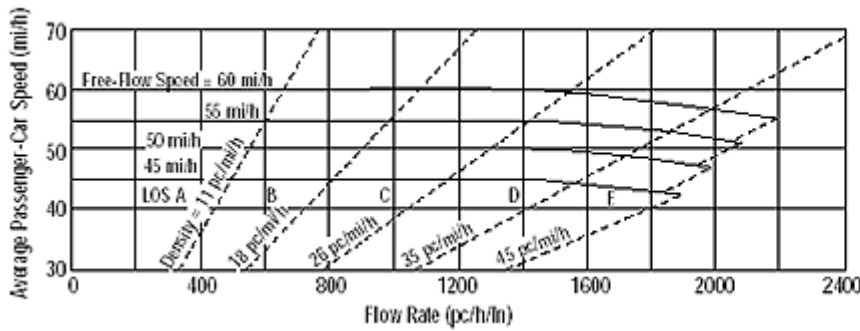
Saturation Flow Module:

Sat/Lane:	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650	1650
Adjustment:	0.91	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	2.00	1.00	1.00	2.00	1.00	1.00	1.00	1.00	1.00	1.39	0.61
Final Sat.:	3000	3300	1650	1650	3300	1650	1650	1650	1650	1650	2299	1001

Capacity Analysis Module:

Vol/Sat:	0.16	0.19	0.00	0.11	0.23	0.00	0.17	0.34	0.14	0.04	0.17	0.17
Crit Volume:	238			383			556			74		
Crit Moves:	****			****			****			****		

MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	Q
Operational (LOS)	FFS, N, v _p	L
Design (N)	FFS, LOS, v _p	N
Design (v _p)	FFS, LOS, N	v _i
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v _p)	FFS, LOS, N	v _i

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/16/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Lone Tree Wy.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP+PP (MITG.)

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	727	Peak-Hour Factor, PHF	0.92
AADT(veh/h)		%Trucks and Buses, P _T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	4
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f _p	1.00	E _R	1.2
E _T	1.5	f _{HV}	0.982

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

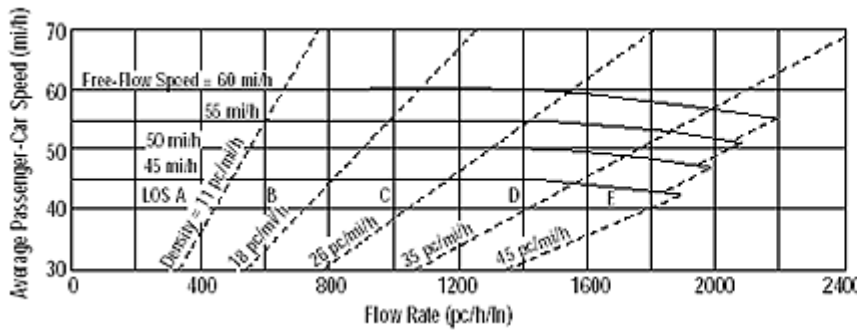
Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 402
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 8.9
 LOS: A

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	Output
Operational (LOS)	FFS, N, v _p	L
Design (N)	FFS, LOS, v _p	N
Design (v _p)	FFS, LOS, N	v _p
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v _p)	FFS, LOS, N	v _p

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/16/2007
 Analysis Time Period: AM Peak

Site Information

Highway/Direction to Travel: Lone Tree Wy.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP+PP (MITG.)

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	826	Peak-Hour Factor, PHF	0.92
AADT(veh/h)		%Trucks and Buses, P _T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	4
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f _p	1.00	E _R	1.2
E _T	1.5	f _{HV}	0.982

Speed Inputs

Lane Width, LW (ft)	12.0
Total Lateral Clearance, LC (ft)	12.0
Access Points, A (A/mi)	0
Median Type, M	
FFS (measured)	45.0
Base Free-Flow Speed, BFFS	

Calc Speed Adj and FFS

f _{LW} (mi/h)	
f _{LC} (mi/h)	
f _A (mi/h)	
f _M (mi/h)	
FFS (mi/h)	45.0

Operations

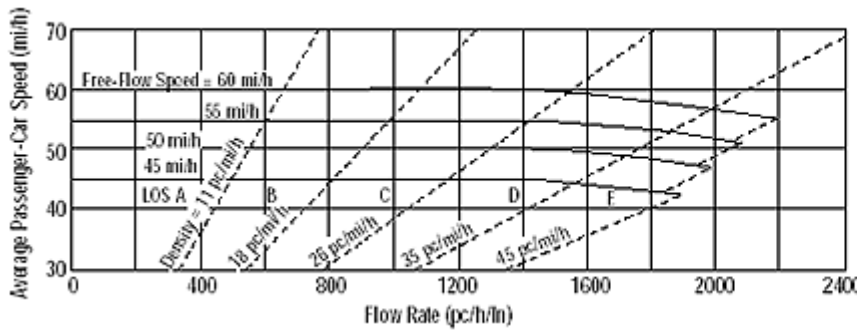
Operational (LOS)	
Flow Rate, v _p (pc/h/ln)	456
Speed, S (mi/h)	45.0
D (pc/mi/ln)	10.1
LOS	A

Design

Design (N)	
Required Number of Lanes, N	
Flow Rate, v _p (pc/h)	
Max Service Flow Rate (pc/h/ln)	
Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	Q
Operational (LOS)	FFS, N, v _p	L
Design (N)	FFS, LOS, v _p	N
Design (v _p)	FFS, LOS, N	v _p
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v _p)	FFS, LOS, N	v _p

General Information		Site Information	
Analyst	Kimley-Horn and Associates	Highway/Direction to Travel	Lone Tree Wy.
Agency or Company		From/To	West of Brentwood Blvd.
Date Performed	11/16/2007	Jurisdiction	City of Brentwood
Analysis Time Period	AM Peak	Analysis Year	Existing+AP+PP (MITG.)

Project Description

Oper.(LOS)
 Des. (N)
 Plan. (vp)

Flow Inputs			
Volume, V (veh/h)	948	Peak-Hour Factor, PHF	0.92
AADT(veh/h)		%Trucks and Buses, P _T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	4
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

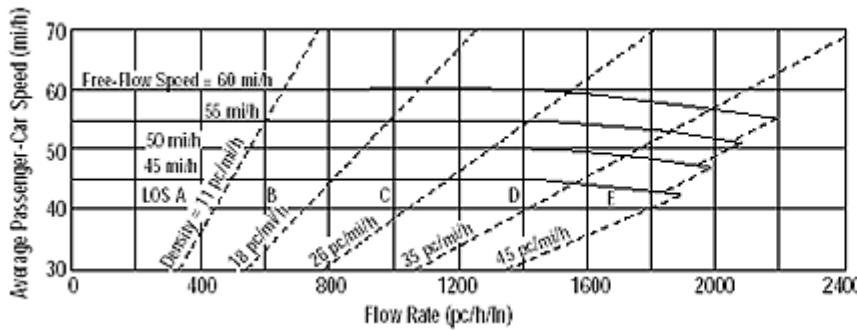
Calculate Flow Adjustments			
f _p	1.00	E _R	1.2
E _T	1.5	f _{HV}	0.982

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	f _{LW} (mi/h)	
Total Lateral Clearance, LC (ft)	12.0	f _{LC} (mi/h)	
Access Points, A (A/mi)	0	f _A (mi/h)	
Median Type, M		f _M (mi/h)	
FFS (measured)	45.0	FFS (mi/h)	45.0
Base Free-Flow Speed, BFFS			

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, v _p (pc/h/ln)	524	Required Number of Lanes, N	
Speed, S (mi/h)	45.0	Flow Rate, v _p (pc/h)	
D (pc/mi/ln)	11.6	Max Service Flow Rate (pc/h/ln)	
LOS	B	Design LOS	

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	Output
Operational (LOS)	FFS, N, v _p	L
Design (N)	FFS, LOS, v _p	N
Design (v _p)	FFS, LOS, N	v _p
Planning (LOS)	FFS, N, AADT	L
Planning (N)	FFS, LOS, AADT	N
Planning (v _p)	FFS, LOS, N	v _p

General Information

Analyst: Kimley-Horn and Associates
 Agency or Company:
 Date Performed: 11/16/2007
 Analysis Time Period: PM Peak

Site Information

Highway/Direction to Travel: Lone Tree Wy.
 From/To: West of Brentwood Blvd.
 Jurisdiction: City of Brentwood
 Analysis Year: Existing+AP+PP (MITG.)

Project Description

Oper.(LOS) Des. (N) Plan. (vp)

Flow Inputs

Volume, V (veh/h)	1036	Peak-Hour Factor, PHF	0.92
AADT(veh/h)		%Trucks and Buses, P _T	2
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	4
Peak-Hour Direction Prop, D		General Terrain:	Level
DDHV (veh/h)		Grade Length (mi)	0.00
Driver Type Adjustment	1.00	Up/Down %	0.00
		Number of Lanes	2

Calculate Flow Adjustments

f _p	1.00	E _R	1.2
E _T	1.5	f _{HV}	0.982

Speed Inputs

Lane Width, LW (ft): 12.0
 Total Lateral Clearance, LC (ft): 12.0
 Access Points, A (A/mi): 0
 Median Type, M:
 FFS (measured): 45.0
 Base Free-Flow Speed, BFFS:

Calc Speed Adj and FFS

f_{LW} (mi/h)
 f_{LC} (mi/h)
 f_A (mi/h)
 f_M (mi/h)
 FFS (mi/h): 45.0

Operations

Operational (LOS)
 Flow Rate, v_p (pc/h/ln): 573
 Speed, S (mi/h): 45.0
 D (pc/mi/ln): 12.7
 LOS: B

Design

Design (N)
 Required Number of Lanes, N
 Flow Rate, v_p (pc/h)
 Max Service Flow Rate (pc/h/ln)
 Design LOS

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APPENDIX E



July 30, 2007

Mr. Nick Pappani
Raney Planning and Management, Inc.
1401 Halyard Drive, Suite 120
West Sacramento, CA 95691

■
Suite 120
1430 Blue Oaks Boulevard
Roseville, California
95747

Re: Brentwood Boulevard Specific Plan DEIR- Supplemental Analysis
Brentwood, California
KHA Project No.: 097464000

Dear Mr. Pappani:

I am writing to present the results of our traffic evaluation for the additional transportation facilities analyzed for the Brentwood Boulevard Specific Plan (the “proposed project”). The purpose of this letter is to summarize the evaluation of weekday Level of Service (LOS) and the traffic signal needs assessment for selected roadway facilities.

Facilities Analyzed

This supplemental analysis includes a roadway segment Level of Service (LOS) analysis for the following roadways:

1. Sellers Avenue between E. Cypress Road and Delta Road
2. Sellers Avenue between Delta Road and Sunset Road
3. Sellers Avenue between Sunset Road and Chestnut Street
4. Sellers Avenue between Chestnut Street and Balfour Road
5. Sellers Avenue between Balfour Road and SR-4
6. O’Hara Avenue North of Central Boulevard

These facilities are depicted in Attachment A. This letter also includes a summary of a planning level assessment of the need for signalization based on peak-hour warrant methodologies for the following location:

1. O’Hara Avenue at the proposed southerly project roadway.

The analysis of the above-listed facilities was conducted in a manner described in the Brentwood Boulevard Specific Plan Traffic Impact Analysis, dated April 11, 2008, (the “TIA”). Where appropriate, traffic count data from the TIA was used for this additional analysis.

Roadway Segment Analysis

Existing Conditions

Analysis of Existing Conditions at the study facilities was based on peak-hour traffic volumes presented in the TIA, if available. Additional peak-hour traffic counts collected at the intersection of Sellers Avenue and Balfour Road, and 24 hour traffic counts on the roadway segment of O’Hara Avenue, north of Central Boulevard. These additional counts were conducted in June 2008. For this scenario,



all study area roadways are assumed to be two lane facilities. Table 1 displays the peak-hour operating conditions for the study roadway segments.

Table 1 – Roadway Segment Levels of Service – Existing Conditions

#	Roadway Segment	AM Peak-Hour		PM Peak-Hour	
		% Time-Spent Following	LOS	% Time-Spent Following	LOS
1	Sellers Ave. between E. Cypress Rd. and Delta Rd.	38.3	A	38.1	A
2	Sellers Ave. between Delta Rd. and Sunset Rd.	45.6	B	38.1	A
3	Sellers Ave. between Sunset Rd. and Chestnut St.	44.7	B	42.3	B
4	Sellers Ave. between Chestnut St. and Balfour Rd.	35.6	A	37.6	A
5	Sellers Ave. between Balfour Rd. and SR-4	34.1	A	40.1	B
6	O'Hara Ave. North of Central Boulevard	64.2	C	76.7	D

As indicated in Table 1, the study roadway segments operate at LOS A through LOS D during the AM and PM peak-hours. Analysis worksheets for this scenario are provided in Attachment B.

Existing plus Approved Projects Conditions

Analysis of Existing plus Approved Projects Conditions at the study facilities was based on peak-hour traffic volumes presented in the TIA. For this scenario, all study area roadways are assumed to be two lane facilities. Table 2 displays the peak-hour operating conditions for the study roadway segments.

As indicated in Table 2, the study roadway segments operate at LOS A through LOS D during the AM and PM peak- hours. Analysis worksheets for this scenario are provided in Attachment C.



**Table 2 – Roadway Segment Levels of Service –
Existing plus Approved Projects Conditions**

#	Roadway Segment	AM Peak-Hour		PM Peak-Hour	
		% Time-Spent Following	LOS	% Time-Spent Following	LOS
1	Sellers Ave. between E. Cypress Rd. and Delta Rd.	38.3	A	38.1	A
2	Sellers Ave. between Delta Rd. and Sunset Rd.	45.6	B	38.1	A
3	Sellers Ave. between Sunset Rd. and Chestnut St.	45.6	B	43.4	B
4	Sellers Ave. between Chestnut St. and Balfour Rd.	36.5	A	38.7	A
5	Sellers Ave. between Balfour Rd. and SR-4	35.4	A	41.5	B
6	O'Hara Ave. North of Central Boulevard	64.2	C	76.7	D

Existing plus Approved Projects plus Proposed Project Conditions

Analysis of Existing plus Approved Projects plus Proposed Project Conditions at the study facilities was based on peak-hour traffic volumes presented in the TIA. For this scenario, all study area roadways are assumed to be two lane facilities. Table 3 displays the peak-hour operating conditions for the study roadway segments.

As indicated in Table 3, the study area roadway segments operate at LOS A through LOS D during the AM and PM peak- hours. Analysis worksheets for this scenario are provided in Attachment D.

Cumulative (2030) Conditions

Analysis of Cumulative Conditions at the study facilities was based on peak-hour traffic volumes presented in the TIA. For this analysis scenario, O'Hara Avenue is assumed to be converted to a four lane roadway and all other study roadway segments are assumed to remain two lane roadways. Table 4 and Table 5 display the peak-hour operating conditions for the two lane and four lane study roadway segments, respectively.

As indicated in Table 4 and Table 5, the study roadway segments operate at LOS A through LOS C during the AM and PM peak-hours. Analysis worksheets for this scenario are provided in Attachment E.



**Table 3 – Roadway Segment Levels of Service – Existing plus Approved Projects
and Existing plus Approved Projects plus Proposed Project Conditions**

#	Roadway Segment	Scenario	AM Peak-Hour		PM Peak-Hour	
			% Time-Spent Following	LOS	% Time-Spent Following	LOS
1	Sellers Ave. between E. Cypress Rd. and Delta Rd.	EPAP	38.3	A	38.1	A
		EPAP + PP	38.3	A	38.1	A
2	Sellers Ave. between Delta Rd. and Sunset Rd.	EPAP	45.6	B	38.1	A
		EPAP + PP	48.2	B	41.3	B
3	Sellers Ave. between Sunset Rd. and Chestnut St.	EPAP	45.6	B	43.4	B
		EPAP + PP	46.7	B	44.6	B
4	Sellers Ave. between Chestnut St. and Balfour Rd.	EPAP	36.5	A	38.7	A
		EPAP + PP	37.8	A	39.9	A
5	Sellers Ave. between Balfour Rd. and SR-4	EPAP	35.4	A	41.5	B
		EPAP + PP	36.9	A	42.8	B
6	O'Hara Ave. North of Central Boulevard	EPAP	64.2	C	76.7	D
		EPAP + PP	65.9	C	78.2	D

Note: EPAP = Existing plus Approved Projects
EPAP + PP = Existing plus Approved Projects plus Proposed Project

Table 4 – Roadway Segment Levels of Service for Two Lane Roadways – Cumulative Conditions

#	Roadway Segment	AM Peak-Hour		PM Peak-Hour	
		% Time-Spent Following	LOS	% Time-Spent Following	LOS
1	Sellers Ave. between E. Cypress Rd. and Delta Rd.	30.3	A	45.1	B
2	Sellers Ave. between Delta Rd. and Sunset Rd.	43.0	B	48.3	B
3	Sellers Ave. between Sunset Rd. and Chestnut St.	53.2	B	54.7	B
4	Sellers Ave. between Chestnut St. and Balfour Rd.	54.7	B	56.2	C
5	Sellers Ave. between Balfour Rd. and SR-4	62.1	C	63.2	C



Table 5 – Roadway Segment Levels of Service for Four Lane Roadways – Cumulative Conditions

#	Roadway Segment	Direction	AM Peak-Hour		PM Peak-Hour	
			Density (pc/mi/ln)	LOS	Density (pc/mi/ln)	LOS
6	O'Hara Ave. North of Central Boulevard	Northbound	2.4	A	6.6	A
		Southbound	7.0	A	4.4	A

Cumulative plus Proposed Project Conditions

Analysis of Cumulative plus Proposed Project Conditions at the study facilities was based on peak-hour traffic volumes presented in the TIA. Table 6 and Table 7 display the peak-hour operating conditions for the two lane and four lane study roadway segments, respectively.

Table 6 – Roadway Segment Levels of Service for Two lane Roadways – Cumulative and Cumulative plus Proposed Project Conditions

#	Roadway Segment	Scenario	AM Peak-Hour		PM Peak-Hour	
			% Time-Spent Following	LOS	% Time-Spent Following	LOS
1	Sellers Ave. between E. Cypress Rd. and Delta Rd.	Cum	30.3	A	45.1	B
		Cum + PP	30.0	A	42.7	B
2	Sellers Ave. between Delta Rd. and Sunset Rd.	Cum	43.0	B	48.3	B
		Cum + PP	42.2	B	44.8	B
3	Sellers Ave. between Sunset Rd. and Chestnut St.	Cum	53.2	B	54.7	B
		Cum + PP	50.9	B	52.4	B
4	Sellers Ave. between Chestnut St. and Balfour Rd.	Cum	54.7	B	56.2	C
		Cum + PP	53.9	B	53.8	B
5	Sellers Ave. between Balfour Rd. and SR-4	Cum	62.1	C	63.2	C
		Cum + PP	62.8	C	62.0	C

Note: Cum = Cumulative
Cum + PP = Cumulative plus Proposed Project

As indicated in Table 6 and Table 7, the study roadway segments operate at LOS A through LOS C during the AM and PM peak-hours. Analysis worksheets for this scenario are provided in Attachment F.



Table 7 – Roadway Segment Levels of Service for Four lane Roadways – Cumulative plus Proposed Project Conditions

#	Roadway Segment	Direction	Scenario	AM Peak-Hour		PM Peak-Hour	
				Density (pc/mi/ln)	LOS	Density (pc/mi/ln)	LOS
6	O'Hara Ave. North of Central Boulevard	NB	Cum	2.4	A	6.6	A
			Cum + PP	2.4	A	6.2	A
		SB	Cum	7.0	A	4.4	A
			Cum + PP	7.0	A	4.4	A
Note: Cum = Cumulative Cum + PP = Cumulative plus Proposed Project							

Assessment of Impacts

As shown in Table 3, Table 6 and Table 7, the project does not cause any of the roadway segments studied to fall below the City's LOS D threshold. As a result, the impact of the project on these roadways is expected to be less than significant.

Traffic Signal Needs Assessment

A planning level assessment of the need for traffic signalization was performed for the following un-signalized intersection:

- O'Hara Avenue at the Proposed Southerly Project Roadway

The peak-hour traffic signal warrant was analyzed for the same scenarios studied for the LOS analysis. This evaluation was consistent with the peak-hour warrant methodologies noted in Section 4C of the *California Manual on Uniform Traffic Control Devices (CMUTCD)*, September 26, 2006.

The study intersection satisfies the peak-hour traffic signal warrant under the Existing plus Approved Projects plus Proposed Project and Cumulative plus Proposed Project analysis scenarios for both the AM and PM peak-hours. Detailed results of this analysis are presented in Attachment G.

Conclusions

Based on the documented analyses, we would like to offer the following conclusions:

- The proposed project will not cause the study roadway segments included in this supplemental analysis to degrade below the City's level of service threshold of LOS D. As a result, the project does not result in a significant impact on any of the study area roadways.
- The addition of the proposed project will result in the peak-hour signal warrant at the intersection of O'Hara Avenue and the proposed southerly project



roadway being satisfied under the Existing plus Approved Projects Conditions and Cumulative Conditions.

Please contact me at (916) 797-3811 if you have any questions or require additional information.

Very truly yours,

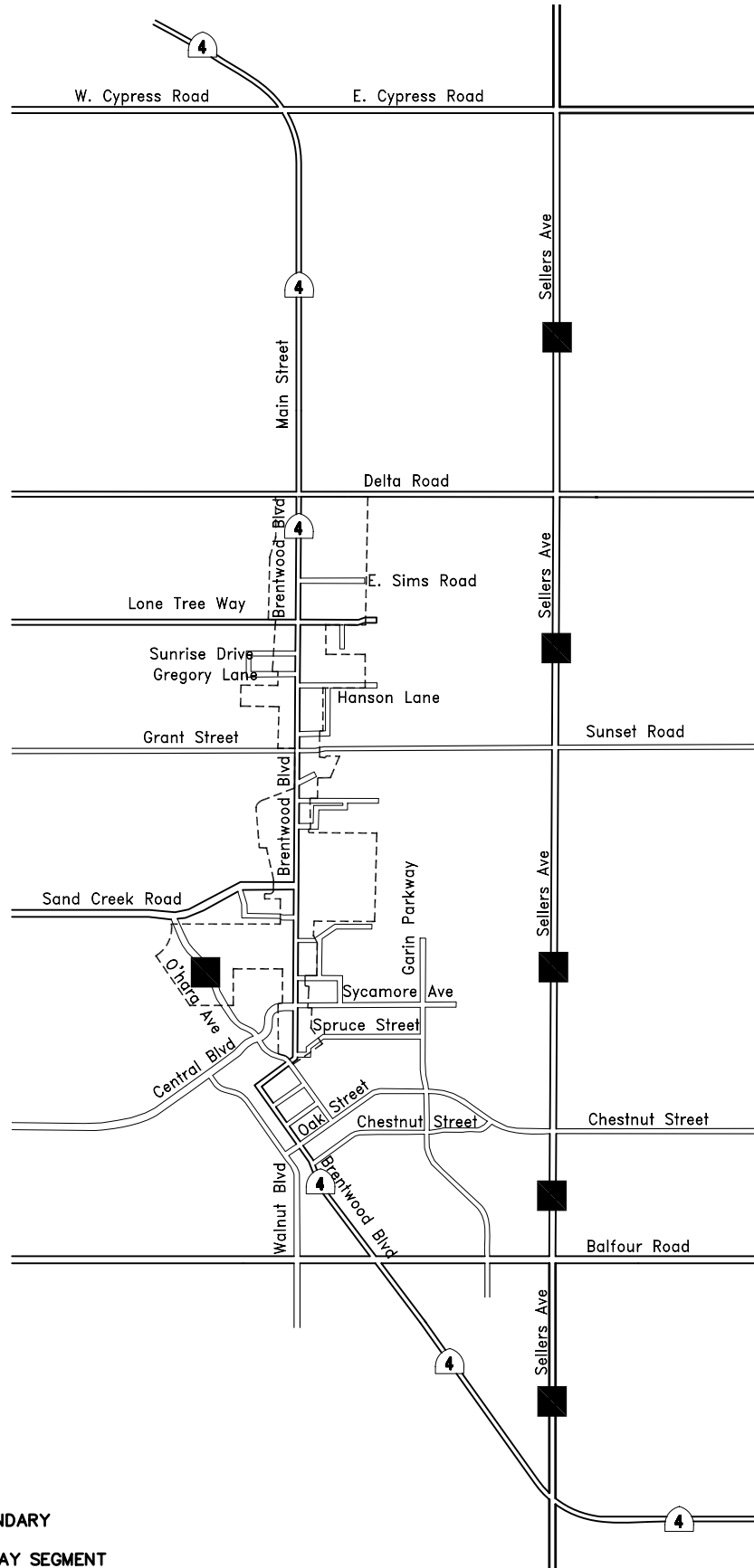
KIMLEY-HORN AND ASSOCIATES, INC.

A handwritten signature in blue ink that reads "Stephen M. Pyburn".

Stephen M. Pyburn, C.E., T.E.
Senior Project Manager
PE No. C49598 & TR1904

Enclosures:

- Attachment A – Project Location and Study Roadway Segments
- Attachment B – Analysis Worksheets for Existing Conditions
- Attachment C – Analysis Worksheets for Existing plus
Approved Projects Conditions
- Attachment D – Analysis Worksheets for Existing plus
Approved Projects plus Proposed Project Conditions
- Attachment E – Analysis Worksheets for Cumulative Conditions
- Attachment F – Analysis Worksheets for Cumulative plus Proposed Project
Conditions
- Attachment G – Analysis Worksheets Peak-Hour Signal Warrants



LEGEND:

- - - PROJECT BOUNDARY
- STUDY ROADWAY SEGMENT



Attachment B:

Analysis Worksheets for Existing Conditions

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	O'Hara Avenue North of Central Boulevard City of Brentwood Existing
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 673 veh/h Directional split 57 / 43 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 0
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	734
v_p * highest directional split proportion ² (pc/h)	418
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 0.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 43.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	3.3
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	34.7
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	733
v_p * highest directional split proportion ² (pc/h)	418
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	47.5
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	16.7
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	64.2
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	C
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.23
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	183

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	673
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	5.3
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Balfour Rd. and SR-4 City of Brentwood Existing
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 248 veh/h Directional split 59 / 41 Peak-hour factor, PHF 0.92 No-passing zone 25 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	273
v _p * highest directional split proportion ² (pc/h)	161
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 45.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 40.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	1.2
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	37.4
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	270
v _p * highest directional split proportion ² (pc/h)	159
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	21.1
Adj. for directional distribution and no-passing zone, f _{d/np} (%)(Exh. 20-12)	13.0
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{d/np}	34.1
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c v/c=V _p /3,200	0.09
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ =0.25L ₁ (V/PHF)	67

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	248
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ =VMT ₁₅ /ATS	1.8
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Chestnut Rd. to Balfour Rd. City of Brentwood Existing
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 459 veh/h Directional split 50 / 50 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.986
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	506
v_p * highest directional split proportion ² (pc/h)	253
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 40.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	36.8
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	500
v_p * highest directional split proportion ² (pc/h)	250
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	35.6
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	35.6
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.16
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	62

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	230
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	1.7
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue E. Cypress Rd to Delta Rd. City of Brentwood Existing
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 407 veh/h Directional split 62 / 38 Peak-hour factor, PHF 0.92 No-passing zone 10 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.986
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	449
v_p * highest directional split proportion ² (pc/h)	278
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 50.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 45.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.8
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p-f_{np}$	41.4
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	443
v_p * highest directional split proportion ² (pc/h)	275
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	32.3
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	6.1
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	38.3
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.14
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	166

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	611
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	4.0
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Delta Rd to Sunset Rd. City of Brentwood Existing
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 635 veh/h Directional split 63 / 37 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	693
v_p * highest directional split proportion ² (pc/h)	437
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 50.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 45.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	40.3
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	692
v_p * highest directional split proportion ² (pc/h)	436
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	45.6
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	45.6
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.22
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	173

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	635
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	4.3
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Sunset Rd. to Chestnut Rd. City of Brentwood Existing
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 619 veh/h Directional split 58 / 42 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.996
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	676
v _p * highest directional split proportion ² (pc/h)	392
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f ^{0.85} /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS-0.00776v _p ^{0.85} -f _{np}	40.5
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	674
v _p * highest directional split proportion ² (pc/h)	391
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	44.7
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	44.7
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c v/c=V _p /3,200	0.21
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	252

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	929
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	6.2
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	O'Hara Avenue North of Central Boulevard City of Brentwood Existing
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1165 veh/h Directional split 62 / 38 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 0
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	1269
v_p * highest directional split proportion ² (pc/h)	787
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 0.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 43.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	2.0
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$	31.9
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.0
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	1.000
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	1266
v_p * highest directional split proportion ² (pc/h)	785
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	67.1
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	9.5
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	76.7
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	D
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.40
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	317

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	1165
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	9.9
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Balfour Rd. and SR-4 City of Brentwood Existing
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 326 veh/h Directional split 64 / 36 Peak-hour factor, PHF 0.92 No-passing zone 25 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12 Shear North Arrow
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	359
v _p * highest directional split proportion ² (pc/h)	230
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 45.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 40.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	1.7
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	36.2
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	355
v _p * highest directional split proportion ² (pc/h)	227
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	26.8
Adj. for directional distribution and no-passing zone, f _{d/np} (%)(Exh. 20-12)	13.3
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{d/np}	40.1
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c v/c=V _p / 3,200	0.11
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	89

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	326
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	2.5
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Chestnut Rd. to Balfour Rd. City of Brentwood Existing
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 489 veh/h Directional split 62 / 38 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	539
v _p * highest directional split proportion ² (pc/h)	334
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 45.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f ^{0.85} /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 40.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p ^{0.85} /f _{np}	36.5
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	533
v _p * highest directional split proportion ² (pc/h)	330
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	37.4
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.2
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	37.6
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c v/c=V _p /3,200	0.17
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	66

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	245
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	1.8
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue E. Cypress Rd to Delta Rd. City of Brentwood Existing
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 403 veh/h Directional split 56 / 44 Peak-hour factor, PHF 0.92 No-passing zone 10 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0 % Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	444
v _p * highest directional split proportion ² (pc/h)	249
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f ^{0.85} /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.8
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p ^{0.85} /f _{np}	41.4
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	439
v _p * highest directional split proportion ² (pc/h)	246
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	32.0
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	6.1
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	38.1
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c v/c=V _p /3,200	0.14
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ =0.25L ₁ (V/PHF)	164

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	605
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ =VMT ₁₅ /ATS	4.0
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Delta Rd to Sunset Rd. City of Brentwood Existing
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 500 veh/h Directional split 54 / 46 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	551
v _p * highest directional split proportion ² (pc/h)	298
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS FFS=S _{FM} *0.00776(V _f /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	41.4
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	545
v _p * highest directional split proportion ² (pc/h)	294
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	38.1
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.1
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	38.1
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c v/c=V _p /3,200	0.17
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ =0.25L ₁ (V/PHF)	136

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	500
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ =VMT ₁₅ /ATS	3.3
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Sunset Rd. to Chestnut Rd. City of Brentwood Existing
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 575 veh/h Directional split 58 / 42 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.996
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	628
v _p * highest directional split proportion ² (pc/h)	364
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f ^{0.85} /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS-0.00776v _p ^{0.75} -f _{np}	40.8
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	626
v _p * highest directional split proportion ² (pc/h)	363
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	42.3
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	42.3
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c v/c=V _p /3,200	0.20
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	234

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	863
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	5.7
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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Attachment C:

*Analysis Worksheets for Existing
plus Approved Projects Conditions*

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	O'Hara Avenue North of Central Boulevard City of Brentwood EPAP
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 673 veh/h Directional split 57 / 43 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 0
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	734
v_p * highest directional split proportion ² (pc/h)	418
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 0.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 43.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	3.3
Average travel speed, ATS (mi/h) $ATS=FFS-0.00776v_p-f_{np}$	34.7
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	733
v_p * highest directional split proportion ² (pc/h)	418
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	47.5
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	16.7
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	64.2
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	C
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.23
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	183

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	673
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	5.3
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Balfour Rd. and SR-4 City of Brentwood EPAP
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 264 veh/h Directional split 60 / 40 Peak-hour factor, PHF 0.92 No-passing zone 25 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12 Shear North Arrow
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.986
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	291
v_p * highest directional split proportion ² (pc/h)	175
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 40.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	1.3
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	37.1
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	288
v_p * highest directional split proportion ² (pc/h)	173
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	22.4
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	13.0
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	35.4
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.09
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	72

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	264
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	1.9
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Chestnut Rd. to Balfour Rd. City of Brentwood EPAP
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 475 veh/h Directional split 51 / 49 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	524
v _p * highest directional split proportion ² (pc/h)	267
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 45.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 40.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS-0.00776v _p -f _{np}	36.6
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	517
v _p * highest directional split proportion ² (pc/h)	264
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	36.5
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	36.5
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c v/c=V _p / 3,200	0.16
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	65

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	238
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	1.8
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue E. Cypress Rd to Delta Rd. City of Brentwood EPAP
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 407 veh/h Directional split 62 / 38 Peak-hour factor, PHF 0.92 No-passing zone 10 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	449
v _p * highest directional split proportion ² (pc/h)	278
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.8
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	41.4
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	443
v _p * highest directional split proportion ² (pc/h)	275
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	32.3
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	6.1
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	38.3
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c v/c=V _p /3,200	0.14
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ =0.25L ₁ (V/PHF)	166

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	611
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	4.0
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Delta Rd to Sunset Rd. City of Brentwood EPAP
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 635 veh/h Directional split 63 / 37 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12 Shear North Arrow
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.996
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	693
v _p * highest directional split proportion ² (pc/h)	437
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS-0.00776v _p -f _{np}	40.3
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	692
v _p * highest directional split proportion ² (pc/h)	436
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	45.6
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	45.6
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c v/c=V _p / 3,200	0.22
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	173

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	635
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	4.3
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Sunset Rd. to Chestnut Rd. City of Brentwood EPAP
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 635 veh/h Directional split 58 / 42 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12 Shear North Arrow
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.996
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	693
v _p * highest directional split proportion ² (pc/h)	402
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS= S _{FM} *0.00776(V _f f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	40.3
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	692
v _p * highest directional split proportion ² (pc/h)	401
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	45.6
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	45.6
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c v/c=V _p / 3,200	0.22
Peak 15-min veh-miles of travel,VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	259

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	953
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	6.4
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	O'Hara Avenue North of Central Boulevard City of Brentwood EPAP
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 1165 veh/h Directional split 62 / 38 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 0
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	1269
v_p * highest directional split proportion ² (pc/h)	787
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 0.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 43.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	2.0
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	31.9
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.0
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	1.000
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	1266
v_p * highest directional split proportion ² (pc/h)	785
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	67.1
Adj. for directional distribution and no-passing zone, f_{dnp} (%)(Exh. 20-12)	9.5
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{dnp}$	76.7
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	D
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.40
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	317

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	1165
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	9.9
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Balfour Rd. and SR-4 City of Brentwood EPAP
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 346 veh/h Directional split 64 / 36 Peak-hour factor, PHF 0.92 No-passing zone 25 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	381
v _p * highest directional split proportion ² (pc/h)	244
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 45.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 40.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	1.8
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	35.9
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	377
v _p * highest directional split proportion ² (pc/h)	241
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	28.2
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	13.2
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	41.5
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c v/c=V _p / 3,200	0.12
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	94

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	346
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	2.6
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Chestnut Rd. to Balfour Rd. City of Brentwood EPAP
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 509 veh/h Directional split 62 / 38 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.986
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	561
v_p * highest directional split proportion ² (pc/h)	348
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 40.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	36.3
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	554
v_p * highest directional split proportion ² (pc/h)	343
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	38.6
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	0.1
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	38.7
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.18
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	69

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	255
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	1.9
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue E. Cypress Rd to Delta Rd. City of Brentwood EPAP
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 403 veh/h Directional split 56 / 44 Peak-hour factor, PHF 0.92 No-passing zone 10 % Trucks and Buses, P _T 2% % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	444
v _p * highest directional split proportion ² (pc/h)	249
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f / f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.8
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	41.4
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	439
v _p * highest directional split proportion ² (pc/h)	246
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	32.0
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	6.1
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	38.1
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c v/c=V _p /3,200	0.14
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	164

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	605
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	4.0
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Delta Rd to Sunset Rd. City of Brentwood EPAP
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 500 veh/h Directional split 54 / 46 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	551
v _p * highest directional split proportion ² (pc/h)	298
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS FFS=S _{FM} *0.00776(V _f /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	41.4
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	545
v _p * highest directional split proportion ² (pc/h)	294
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	38.1
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.1
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	38.1
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c v/c=V _p /3,200	0.17
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	136

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	500
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	3.3
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Sunset Rd. to Chestnut Rd. City of Brentwood EPAP
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 595 veh/h Directional split 58 / 42 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	649
v_p * highest directional split proportion ² (pc/h)	376
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 50.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 45.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p-f_{np}$	40.7
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	648
v_p * highest directional split proportion ² (pc/h)	376
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	43.4
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	43.4
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.20
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	243

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	893
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	6.0
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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Attachment D:

*Analysis Worksheets for Existing plus Approved Projects
plus Proposed Project Conditions*

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	O'Hara Avenue North of Central Boulevard City of Brentwood EPAP + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 748 veh/h Directional split 55 / 45 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 0
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF \cdot f_G \cdot f_{HV})$	816
v_p * highest directional split proportion ² (pc/h)	449
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM} \cdot 0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 0.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS \cdot f_{LS} \cdot f_A)$ 43.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	3.0
Average travel speed, ATS (mi/h) $ATS=FFS \cdot 0.00776 v_p \cdot f_{np}$	34.4
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF \cdot f_G \cdot f_{HV})$	815
v_p * highest directional split proportion ² (pc/h)	448
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879 v_p})$	51.1
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	14.7
Percent time-spent-following, PTSF(%) $PTSF=BPTSF \cdot f_{d/np}$	65.9
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	C
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.25
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	203

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V \cdot L_1$	748
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	5.9
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Balfour Rd. and SR-4 City of Brentwood EPAP + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 286 veh/h Directional split 58 / 42 Peak-hour factor, PHF 0.92 No-passing zone 25 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.986
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	315
v_p * highest directional split proportion ² (pc/h)	183
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 40.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	1.5
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	36.8
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	311
v_p * highest directional split proportion ² (pc/h)	180
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	23.9
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	13.0
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	36.9
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.10
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	78

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	286
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	2.1
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Chestnut Rd. to Balfour Rd. City of Brentwood EPAP + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 497 veh/h Directional split 50 / 50 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	548
v _p * highest directional split proportion ² (pc/h)	274
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 45.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 40.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS-0.00776v _p -f _{np}	36.4
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	541
v _p * highest directional split proportion ² (pc/h)	271
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	37.8
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	37.8
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c v/c=V _p / 3,200	0.17
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	68

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	249
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	1.9
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue E. Cypress Rd to Delta Rd. City of Brentwood EPAP + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 407 veh/h Directional split 62 / 38 Peak-hour factor, PHF 0.92 No-passing zone 10 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	449
v _p * highest directional split proportion ² (pc/h)	278
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.8
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	41.4
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	443
v _p * highest directional split proportion ² (pc/h)	275
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	32.3
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	6.1
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	38.3
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c v/c=V _p /3,200	0.14
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	166

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	611
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	4.0
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Delta Rd to Sunset Rd. City of Brentwood EPAP + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 688 veh/h Directional split 62 / 38 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.996
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	751
v _p * highest directional split proportion ² (pc/h)	466
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS FFS=S _{FM} *0.00776(V _f /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	39.9
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	749
v _p * highest directional split proportion ² (pc/h)	464
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	48.2
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	48.2
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c v/c=V _p /3,200	0.23
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ =0.25L ₁ (V/PHF)	187

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	688
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ =VMT ₁₅ /ATS	4.7
Notes	
1. If Vp >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split Vp>= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Sunset Rd. to Chestnut Rd. City of Brentwood EPAP + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 657 veh/h Directional split 57 / 43 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.996
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	717
v _p * highest directional split proportion ² (pc/h)	409
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS-0.00776v _p -f _{np}	40.1
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	716
v _p * highest directional split proportion ² (pc/h)	408
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	46.7
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	46.7
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c v/c=V _p /3,200	0.22
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ =0.25L ₁ (V/PHF)	268

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	986
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	6.7
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	6/17/2008 Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	O'Hara Avenue North of Central Boulevard City of Brentwood EPAP + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input type="checkbox"/> Level <input checked="" type="checkbox"/> Rolling Two-way hourly volume 1247 veh/h Directional split 60 / 40 Peak-hour factor, PHF 0.92 No-passing zone 100 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 0 Shear North Arrow
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	1358
v _p * highest directional split proportion ² (pc/h)	815
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 45.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS FFS=S _{FM} *0.00776(V _f ^{0.85} /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 0.0 mi/h
	Free-flow speed, FFS (FSS=BFFS*f _{LS} *f _A) 43.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	1.8
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p ^{0.85} /f _{np}	31.4
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.0
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	1.000
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	1355
v _p * highest directional split proportion ² (pc/h)	813
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	69.6
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	8.6
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	78.2
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	D
Volume to capacity ratio v/c v/c=V _p / 3,200	0.42
Peak 15-min veh-miles of travel,VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	339

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	1247
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	10.8
Notes	
1. If Vp >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split Vp>= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Balfour Rd. and SR-4 City of Brentwood EPAP + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 368 veh/h Directional split 63 / 37 Peak-hour factor, PHF 0.92 No-passing zone 25 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12 Shear North Arrow
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.986
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	406
v_p * highest directional split proportion ² (pc/h)	256
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 40.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	1.9
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	35.6
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	401
v_p * highest directional split proportion ² (pc/h)	253
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	29.7
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	13.1
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	42.8
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.13
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	100

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	368
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	2.8
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Chestnut Rd. to Balfour Rd. City of Brentwood EPAP + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 531 veh/h Directional split 61 / 39 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.986
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	585
v_p * highest directional split proportion ² (pc/h)	357
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 40.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	36.2
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	578
v_p * highest directional split proportion ² (pc/h)	353
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	39.8
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	0.1
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	39.9
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.18
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	72

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	266
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	2.0
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue E. Cypress Rd to Delta Rd. City of Brentwood EPAP + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 403 veh/h Directional split 56 / 44 Peak-hour factor, PHF 0.92 No-passing zone 10 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	444
v _p * highest directional split proportion ² (pc/h)	249
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS FFS=S _{FM} *0.00776(V _f ^{0.85} /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.8
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p ^{0.85} /f _{np}	41.4
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	439
v _p * highest directional split proportion ² (pc/h)	246
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	32.0
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	6.1
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	38.1
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c v/c=V _p /3,200	0.14
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	164

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	605
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	4.0
Notes	
1. If Vp >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split Vp>= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Delta Rd to Sunset Rd. City of Brentwood EPAP + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 557 veh/h Directional split 54 / 46 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.996
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	608
v _p * highest directional split proportion ² (pc/h)	328
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS FFS=S _{FM} *0.00776(V _f /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	41.0
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	607
v _p * highest directional split proportion ² (pc/h)	328
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	41.3
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	41.3
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c v/c=V _p /3,200	0.19
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ =0.25L ₁ (V/PHF)	151

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	557
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ =VMT ₁₅ /ATS	3.7
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Sunset Rd. to Chestnut Rd. City of Brentwood EPAP + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 617 veh/h Directional split 57 / 43 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.996
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	673
v _p * highest directional split proportion ² (pc/h)	384
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS= S _{FM} *0.00776(V _f ^{0.85} /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p ^{0.75} /f _{np}	40.5
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	672
v _p * highest directional split proportion ² (pc/h)	383
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	44.6
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	44.6
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c v/c=V _p /3,200	0.21
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	251

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	926
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	6.2
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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Appendix E:

Analysis Worksheets for Cumulative Conditions

MULTILANE HIGHWAYS WORKSHEET(Direction 1)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>C</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>L</td> </tr> <tr> <td>Design (H)</td> <td>FFS, LOS, v_p</td> <td>N</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>L</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p</td> </tr> </tbody> </table>		Application	Input	C	Operational (LOS)	FFS, N, v _p	L	Design (H)	FFS, LOS, v _p	N	Design (v _p)	FFS, LOS, N	v _p	Planning (LOS)	FFS, N, AADT	L	Planning (N)	FFS, LOS, AADT	N	Planning (v _p)	FFS, LOS, N	v _p
Application	Input	C																						
Operational (LOS)	FFS, N, v _p	L																						
Design (H)	FFS, LOS, v _p	N																						
Design (v _p)	FFS, LOS, N	v _p																						
Planning (LOS)	FFS, N, AADT	L																						
Planning (N)	FFS, LOS, AADT	N																						
Planning (v _p)	FFS, LOS, N	v _p																						
General Information		Site Information																						
Analyst Agency or Company Date Performed Analysis Time Period		Highway/Direction to Travel From/To Jurisdiction Analysis Year																						
Project Description																								
<input checked="" type="checkbox"/> Oper.(LOS) <input type="checkbox"/> Des. (N) <input type="checkbox"/> Plan. (vp)																								
Flow Inputs																								
Volume, V (veh/h)		Peak-Hour Factor, PHF																						
AADT(veh/h)		%Trucks and Buses, P _T																						
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R																						
Peak-Hour Direction Prop, D		General Terrain:																						
DDHV (veh/h)		Grade Length (mi)																						
Driver Type Adjustment		Up/Down %																						
		Number of Lanes																						
Calculate Flow Adjustments																								
f _p		E _R																						
E _T		f _{HV}																						
Speed Inputs																								
Lane Width, LW (ft)		f _{LW} (mi/h)																						
Total Lateral Clearance, LC (ft)		f _{LC} (mi/h)																						
Access Points, A (A/mi)		f _A (mi/h)																						
Median Type, M		f _M (mi/h)																						
FFS (measured)		FFS (mi/h)																						
Base Free-Flow Speed, BFFS																								
Operations																								
Operational (LOS)		Design (N)																						
Flow Rate, v _p (pc/h/ln)		Required Number of Lanes, N																						
Speed, S (mi/h)		Flow Rate, v _p (pc/h)																						
D (pc/mi/ln)		Max Service Flow Rate (pc/h/ln)																						
LOS		Design LOS																						

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>C</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>L</td> </tr> <tr> <td>Design (H)</td> <td>FFS, LOS, v_p</td> <td>N</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>L</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v</td> </tr> </tbody> </table>		Application	Input	C	Operational (LOS)	FFS, N, v _p	L	Design (H)	FFS, LOS, v _p	N	Design (v _p)	FFS, LOS, N	v	Planning (LOS)	FFS, N, AADT	L	Planning (N)	FFS, LOS, AADT	N	Planning (v _p)	FFS, LOS, N	v
Application	Input	C																						
Operational (LOS)	FFS, N, v _p	L																						
Design (H)	FFS, LOS, v _p	N																						
Design (v _p)	FFS, LOS, N	v																						
Planning (LOS)	FFS, N, AADT	L																						
Planning (N)	FFS, LOS, AADT	N																						
Planning (v _p)	FFS, LOS, N	v																						
General Information		Site Information																						
Analyst Agency or Company Date Performed Analysis Time Period		Highway/Direction to Travel From/To Jurisdiction Analysis Year																						
Project Description																								
<input checked="" type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)																						
<input type="checkbox"/> Plan. (vp)																								
Flow Inputs																								
Volume, V (veh/h)		Peak-Hour Factor, PHF																						
AADT(veh/h)		%Trucks and Buses, P _T																						
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R																						
Peak-Hour Direction Prop, D		General Terrain:																						
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Median Type, M		f _M (mi/h)																						
FFS (measured)		FFS (mi/h)																						
Base Free-Flow Speed, BFFS		45.0																						
Operations		Design																						
Operational (LOS)		Design (N)																						
Flow Rate, v _p (pc/h/ln)		Required Number of Lanes, N																						
Speed, S (mi/h)		Flow Rate, v _p (pc/h)																						
D (pc/mi/ln)		Max Service Flow Rate (pc/h/ln)																						
LOS		Design LOS																						

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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue E. Cypress Rd to Delta Rd. City of Brentwood Cumulative
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 288 veh/h Directional split 57 / 43 Peak-hour factor, PHF 0.92 No-passing zone 10 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.986
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	317
v_p * highest directional split proportion ² (pc/h)	181
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 50.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 45.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.6
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	42.6
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	314
v_p * highest directional split proportion ² (pc/h)	179
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	24.1
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	6.2
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	30.3
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.10
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	117

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	432
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	2.7
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Delta Rd to Sunset Rd. City of Brentwood Cumulative
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 588 veh/h Directional split 59 / 41 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF \cdot f_G \cdot f_{HV})$	642
v_p * highest directional split proportion ² (pc/h)	379
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 50.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM} \cdot 0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS \cdot f_{LS} \cdot f_A)$ 45.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) $ATS=FFS \cdot 0.00776 v_p \cdot f_{np}$	40.7
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF \cdot f_G \cdot f_{HV})$	640
v_p * highest directional split proportion ² (pc/h)	378
Base percent time-spent-following, $BPTSF$ (%) $BPTSF=100(1-e^{-0.000879 v_p})$	43.0
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%) (Exh. 20-12)	0.0
Percent time-spent-following, $PTSF$ (%) $PTSF=BPTSF \cdot f_{d/np}$	43.0
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.20
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	160

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V \cdot L_1$	588
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	3.9
Notes	
1. If $V_p \geq 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p \geq 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Sunset Rd. to Chestnut Rd. City of Brentwood Cumulative
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 794 veh/h Directional split 61 / 39 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	866
v_p * highest directional split proportion ² (pc/h)	528
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 50.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 45.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	39.0
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	865
v_p * highest directional split proportion ² (pc/h)	528
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	53.2
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	53.2
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.27
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	324

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	1191
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	8.3
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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MULTILANE HIGHWAYS WORKSHEET(Direction 1)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>C</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>L</td> </tr> <tr> <td>Design (H)</td> <td>FFS, LOS, v_p</td> <td>N</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>L</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p</td> </tr> </tbody> </table>		Application	Input	C	Operational (LOS)	FFS, N, v _p	L	Design (H)	FFS, LOS, v _p	N	Design (v _p)	FFS, LOS, N	v _p	Planning (LOS)	FFS, N, AADT	L	Planning (N)	FFS, LOS, AADT	N	Planning (v _p)	FFS, LOS, N	v _p
Application	Input	C																						
Operational (LOS)	FFS, N, v _p	L																						
Design (H)	FFS, LOS, v _p	N																						
Design (v _p)	FFS, LOS, N	v _p																						
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Volume, V (veh/h)		Peak-Hour Factor, PHF																						
AADT(veh/h)		%Trucks and Buses, P _T																						
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R																						
Peak-Hour Direction Prop, D		General Terrain:																						
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Driver Type Adjustment		Up/Down %																						
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Calculate Flow Adjustments																								
f _p		E _R																						
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Speed Inputs																								
Lane Width, LW (ft)		f _{LW} (mi/h)																						
Total Lateral Clearance, LC (ft)		f _{LC} (mi/h)																						
Access Points, A (A/mi)		f _A (mi/h)																						
Median Type, M		f _M (mi/h)																						
FFS (measured)		FFS (mi/h)																						
Base Free-Flow Speed, BFFS		45.0																						
Operations		Design																						
Operational (LOS)		Design (N)																						
Flow Rate, v _p (pc/h/ln)		Required Number of Lanes, N																						
Speed, S (mi/h)		Flow Rate, v _p (pc/h)																						
D (pc/mi/ln)		Max Service Flow Rate (pc/h/ln)																						
LOS		Design LOS																						

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)																								
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Application	Input	C																						
Operational (LOS)	FFS, N, v _p	L																						
Design (H)	FFS, LOS, v _p	N																						
Design (v _p)	FFS, LOS, N	v																						
Planning (LOS)	FFS, N, AADT	L																						
Planning (N)	FFS, LOS, AADT	N																						
Planning (v _p)	FFS, LOS, N	v																						
General Information		Site Information																						
Analyst Agency or Company Date Performed Analysis Time Period		Highway/Direction to Travel From/To Jurisdiction Analysis Year																						
Project Description																								
<input checked="" type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)																						
<input type="checkbox"/> Plan. (vp)																								
Flow Inputs																								
Volume, V (veh/h)	359	Peak-Hour Factor, PHF	0.92																					
AADT(veh/h)		%Trucks and Buses, P _T	2																					
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Level																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f _p	1.00	E _R	1.2																					
E _T	1.5	f _{HV}	0.990																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f _{LW} (mi/h)																						
Total Lateral Clearance, LC (ft)	12.0	f _{LC} (mi/h)																						
Access Points, A (A/mi)	0	f _A (mi/h)																						
Median Type, M		f _M (mi/h)																						
FFS (measured)	45.0	FFS (mi/h)	45.0																					
Base Free-Flow Speed, BFFS																								
Operations		Design																						
Operational (LOS)		Design (N)																						
Flow Rate, v _p (pc/h/ln)	197	Required Number of Lanes, N																						
Speed, S (mi/h)	45.0	Flow Rate, v _p (pc/h)																						
D (pc/mi/ln)	4.4	Max Service Flow Rate (pc/h/ln)																						
LOS	A	Design LOS																						

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Balfour Rd. and SR-4 City of Brentwood Cumulative
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 835 veh/h Directional split 55 / 45 Peak-hour factor, PHF 0.92 No-passing zone 25 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	911
v_p * highest directional split proportion ² (pc/h)	501
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 40.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	1.4
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	32.3
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	909
v_p * highest directional split proportion ² (pc/h)	500
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	55.0
Adj. for directional distribution and no-passing zone, f_{dnp} (%)(Exh. 20-12)	8.2
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{dnp}$	63.2
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	C
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.28
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	227

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	835
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	7.0
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Chestnut Rd. to Balfour Rd. City of Brentwood Cumulative
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 862 veh/h Directional split 58 / 42 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	941
v_p * highest directional split proportion ² (pc/h)	546
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 40.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	33.4
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	939
v_p * highest directional split proportion ² (pc/h)	545
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	56.2
Adj. for directional distribution and no-passing zone, f_{dnp} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{dnp}$	56.2
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	C
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.29
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	117

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	431
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	3.5
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue E. Cypress Rd to Delta Rd. City of Brentwood Cumulative
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 467 veh/h Directional split 88 / 12 Peak-hour factor, PHF 0.92 No-passing zone 10 % Trucks and Buses, P_T 2% % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.986
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	515
v_p * highest directional split proportion ² (pc/h)	453
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 50.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 45.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.8
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	40.9
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	509
v_p * highest directional split proportion ² (pc/h)	448
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	36.1
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	9.0
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	45.1
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.16
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	190

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	701
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	4.6
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Delta Rd to Sunset Rd. City of Brentwood Cumulative
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 689 veh/h Directional split 63 / 37 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12 Shear North Arrow
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.996
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	752
v _p * highest directional split proportion ² (pc/h)	474
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS FFS=S _{FM} *0.00776(V _f /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	39.9
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	750
v _p * highest directional split proportion ² (pc/h)	473
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	48.3
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	48.3
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c v/c=V _p / 3,200	0.23
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	187

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	689
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	4.7
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Sunset Rd. to Chestnut Rd. City of Brentwood Cumulative
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 828 veh/h Directional split 54 / 46 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	904
v_p * highest directional split proportion ² (pc/h)	488
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 50.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 45.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	38.7
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	902
v_p * highest directional split proportion ² (pc/h)	487
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	54.7
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	54.7
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.28
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	337

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	1242
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	8.7
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Balfour Rd. and SR-4 City of Brentwood Cumulative
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 814 veh/h Directional split 58 / 42 Peak-hour factor, PHF 0.92 No-passing zone 25 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	888
v_p * highest directional split proportion ² (pc/h)	515
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 40.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	1.4
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	32.4
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	887
v_p * highest directional split proportion ² (pc/h)	514
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	54.1
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	7.9
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	62.1
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	C
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.28
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	221

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	814
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	6.8
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Chestnut Rd. to Balfour Rd. City of Brentwood Cumulative
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 828 veh/h Directional split 60 / 40 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	904
v_p * highest directional split proportion ² (pc/h)	542
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 40.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	33.7
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	902
v_p * highest directional split proportion ² (pc/h)	541
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	54.7
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	54.7
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.28
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	112

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	414
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	3.3
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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Appendix F:

*Analysis Worksheets for Cumulative
plus Proposed Project Conditions*

MULTILANE HIGHWAYS WORKSHEET(Direction 1)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>C</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>L</td> </tr> <tr> <td>Design (H)</td> <td>FFS, LOS, v_p</td> <td>N</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>L</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v</td> </tr> </tbody> </table>		Application	Input	C	Operational (LOS)	FFS, N, v _p	L	Design (H)	FFS, LOS, v _p	N	Design (v _p)	FFS, LOS, N	v	Planning (LOS)	FFS, N, AADT	L	Planning (N)	FFS, LOS, AADT	N	Planning (v _p)	FFS, LOS, N	v
Application	Input	C																						
Operational (LOS)	FFS, N, v _p	L																						
Design (H)	FFS, LOS, v _p	N																						
Design (v _p)	FFS, LOS, N	v																						
Planning (LOS)	FFS, N, AADT	L																						
Planning (N)	FFS, LOS, AADT	N																						
Planning (v _p)	FFS, LOS, N	v																						
General Information		Site Information																						
Analyst Agency or Company Date Performed Analysis Time Period		Highway/Direction to Travel From/To Jurisdiction Analysis Year																						
Project Description																								
<input checked="" type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N) <input type="checkbox"/> Plan. (vp)																						
Flow Inputs																								
Volume, V (veh/h)	195	Peak-Hour Factor, PHF	0.92																					
AAADT(veh/h)		%Trucks and Buses, P _T	2																					
Peak-Hour Prop of AAADT (veh/d)		%RVs, P _R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Level																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f _p	1.00	E _R	1.2																					
E _T	1.5	f _{HV}	0.990																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f _{LW} (mi/h)																						
Total Lateral Clearance, LC (ft)	12.0	f _{LC} (mi/h)																						
Access Points, A (A/mi)	0	f _A (mi/h)																						
Median Type, M		f _M (mi/h)																						
FFS (measured)	45.0	FFS (mi/h)	45.0																					
Base Free-Flow Speed, BFFS																								
Operations		Design																						
Operational (LOS)		Design (N)																						
Flow Rate, v _p (pc/h/ln)	107	Required Number of Lanes, N																						
Speed, S (mi/h)	45.0	Flow Rate, v _p (pc/h)																						
D (pc/mi/ln)	2.4	Max Service Flow Rate (pc/h/ln)																						
LOS	A	Design LOS																						

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>C</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>L</td> </tr> <tr> <td>Design (H)</td> <td>FFS, LOS, v_p</td> <td>N</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>L</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v</td> </tr> </tbody> </table>		Application	Input	C	Operational (LOS)	FFS, N, v _p	L	Design (H)	FFS, LOS, v _p	N	Design (v _p)	FFS, LOS, N	v	Planning (LOS)	FFS, N, AADT	L	Planning (N)	FFS, LOS, AADT	N	Planning (v _p)	FFS, LOS, N	v
Application	Input	C																						
Operational (LOS)	FFS, N, v _p	L																						
Design (H)	FFS, LOS, v _p	N																						
Design (v _p)	FFS, LOS, N	v																						
Planning (LOS)	FFS, N, AADT	L																						
Planning (N)	FFS, LOS, AADT	N																						
Planning (v _p)	FFS, LOS, N	v																						
General Information		Site Information																						
Analyst Agency or Company: Kimley-Horn and Associates Date Performed: 7/25/2008 Analysis Time Period: AM Peak		Highway/Direction to Travel: O'Hara Avenue (Southbo From/To: North of Central Boulevard Jurisdiction: City of Brentwood Analysis Year: Cumulative + PP																						
Project Description																								
<input checked="" type="checkbox"/> Oper.(LOS) <input type="checkbox"/> Des. (N) <input type="checkbox"/> Plan. (vp)																								
Flow Inputs																								
Volume, V (veh/h)	579	Peak-Hour Factor, PHF	0.92																					
AADT(veh/h)		%Trucks and Buses, P _T	2																					
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Level																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f _p	1.00	E _R	1.2																					
E _T	1.5	f _{HV}	0.990																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f _{LW} (mi/h)																						
Total Lateral Clearance, LC (ft)	12.0	f _{LC} (mi/h)																						
Access Points, A (A/mi)	0	f _A (mi/h)																						
Median Type, M		f _M (mi/h)																						
FFS (measured)	45.0	FFS (mi/h)	45.0																					
Base Free-Flow Speed, BFFS																								
Operations		Design																						
Operational (LOS)		Design (N)																						
Flow Rate, v _p (pc/h/ln)	317	Required Number of Lanes, N																						
Speed, S (mi/h)	45.0	Flow Rate, v _p (pc/h)																						
D (pc/mi/ln)	7.0	Max Service Flow Rate (pc/h/ln)																						
LOS	A	Design LOS																						

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Chestnut Rd. to Balfour Rd. City of Brentwood Cumulative + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 809 veh/h Directional split 58 / 42 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	883
v_p * highest directional split proportion ² (pc/h)	512
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 40.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	33.8
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	881
v_p * highest directional split proportion ² (pc/h)	511
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	53.9
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	53.9
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.28
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	110

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	405
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	3.2
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue E. Cypress Rd to Delta Rd. City of Brentwood Cumulative + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 282 veh/h Directional split 59 / 41 Peak-hour factor, PHF 0.92 No-passing zone 10 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	311
v _p * highest directional split proportion ² (pc/h)	183
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS FFS=S _{FM} *0.00776(V _f /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.6
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	42.7
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	307
v _p * highest directional split proportion ² (pc/h)	181
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	23.7
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	6.3
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	30.0
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	A
Volume to capacity ratio v/c v/c=V _p / 3,200	0.10
Peak 15-min veh-miles of travel,VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	115

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	423
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	2.7
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Delta Rd to Sunset Rd. City of Brentwood Cumulative + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 573 veh/h Directional split 57 / 43 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	625
v_p * highest directional split proportion ² (pc/h)	356
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 50.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 45.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p-f_{np}$	40.9
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	624
v_p * highest directional split proportion ² (pc/h)	356
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	42.2
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	42.2
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.20
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	156

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	573
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	3.8
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Sunset Rd. to Chestnut Rd. City of Brentwood Cumulative + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 743 veh/h Directional split 58 / 42 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	811
v_p * highest directional split proportion ² (pc/h)	470
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 50.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 45.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	39.4
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	809
v_p * highest directional split proportion ² (pc/h)	469
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	50.9
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	50.9
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.25
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	303

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	1115
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	7.7
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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MULTILANE HIGHWAYS WORKSHEET(Direction 1)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>C</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>L</td> </tr> <tr> <td>Design (H)</td> <td>FFS, LOS, v_p</td> <td>N</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>L</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v</td> </tr> </tbody> </table>		Application	Input	C	Operational (LOS)	FFS, N, v _p	L	Design (H)	FFS, LOS, v _p	N	Design (v _p)	FFS, LOS, N	v	Planning (LOS)	FFS, N, AADT	L	Planning (N)	FFS, LOS, AADT	N	Planning (v _p)	FFS, LOS, N	v
Application	Input	C																						
Operational (LOS)	FFS, N, v _p	L																						
Design (H)	FFS, LOS, v _p	N																						
Design (v _p)	FFS, LOS, N	v																						
Planning (LOS)	FFS, N, AADT	L																						
Planning (N)	FFS, LOS, AADT	N																						
Planning (v _p)	FFS, LOS, N	v																						
General Information		Site Information																						
Analyst Agency or Company Date Performed Analysis Time Period		Highway/Direction to Travel From/To Jurisdiction Analysis Year																						
Project Description																								
<input checked="" type="checkbox"/> Oper.(LOS) <input type="checkbox"/> Des. (N) <input type="checkbox"/> Plan. (vp)																								
Flow Inputs																								
Volume, V (veh/h)		Peak-Hour Factor, PHF																						
AADT(veh/h)		%Trucks and Buses, P _T																						
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R																						
Peak-Hour Direction Prop, D		General Terrain:																						
DDHV (veh/h)		Grade Length (mi)																						
Driver Type Adjustment		Up/Down %																						
		Number of Lanes																						
Calculate Flow Adjustments																								
f _p		E _R																						
E _T		f _{HV}																						
Speed Inputs																								
Lane Width, LW (ft)		f _{LW} (mi/h)																						
Total Lateral Clearance, LC (ft)		f _{LC} (mi/h)																						
Access Points, A (A/mi)		f _A (mi/h)																						
Median Type, M		f _M (mi/h)																						
FFS (measured)		FFS (mi/h)																						
Base Free-Flow Speed, BFFS		45.0																						
Operations		Design																						
Operational (LOS)		Design (N)																						
Flow Rate, v _p (pc/h/ln)		Required Number of Lanes, N																						
Speed, S (mi/h)		Flow Rate, v _p (pc/h)																						
D (pc/mi/ln)		Max Service Flow Rate (pc/h/ln)																						
LOS		Design LOS																						

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MULTILANE HIGHWAYS WORKSHEET(Direction 2)																								
	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>C</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>L</td> </tr> <tr> <td>Design (H)</td> <td>FFS, LOS, v_p</td> <td>N</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>L</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v</td> </tr> </tbody> </table>			Application	Input	C	Operational (LOS)	FFS, N, v _p	L	Design (H)	FFS, LOS, v _p	N	Design (v _p)	FFS, LOS, N	v	Planning (LOS)	FFS, N, AADT	L	Planning (N)	FFS, LOS, AADT	N	Planning (v _p)	FFS, LOS, N	v
Application	Input	C																						
Operational (LOS)	FFS, N, v _p	L																						
Design (H)	FFS, LOS, v _p	N																						
Design (v _p)	FFS, LOS, N	v																						
Planning (LOS)	FFS, N, AADT	L																						
Planning (N)	FFS, LOS, AADT	N																						
Planning (v _p)	FFS, LOS, N	v																						
General Information		Site Information																						
Analyst Agency or Company Date Performed Analysis Time Period		Highway/Direction to Travel From/To Jurisdiction Analysis Year																						
Project Description																								
<input checked="" type="checkbox"/> Oper.(LOS) <input type="checkbox"/> Des. (N) <input type="checkbox"/> Plan. (vp)																								
Flow Inputs																								
Volume, V (veh/h)		Peak-Hour Factor, PHF																						
AADT(veh/h)		%Trucks and Buses, P _T																						
Peak-Hour Prop of AADT (veh/d)		%RVs, P _R																						
Peak-Hour Direction Prop, D		General Terrain:																						
DDHV (veh/h)		Grade Length (mi)																						
Driver Type Adjustment		Up/Down %																						
		Number of Lanes																						
Calculate Flow Adjustments																								
f _p		E _R																						
E _T		f _{HV}																						
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)		f _{LW} (mi/h)																						
Total Lateral Clearance, LC (ft)		f _{LC} (mi/h)																						
Access Points, A (A/mi)		f _A (mi/h)																						
Median Type, M		f _M (mi/h)																						
FFS (measured)		FFS (mi/h)																						
Base Free-Flow Speed, BFFS		45.0																						
Operations		Design																						
Operational (LOS)		Design (N)																						
Flow Rate, v _p (pc/h/ln)		Required Number of Lanes, N																						
Speed, S (mi/h)		Flow Rate, v _p (pc/h)																						
D (pc/mi/ln)		Max Service Flow Rate (pc/h/ln)																						
LOS		Design LOS																						

TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	6/17/2008 Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Balfour Rd. and SR-4 City of Brentwood Cumulative + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 804 veh/h Directional split 56 / 44 Peak-hour factor, PHF 0.92 No-passing zone 25 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12 Shear North Arrow
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.996
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	877
v _p * highest directional split proportion ² (pc/h)	491
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 45.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS FFS=S _{FM} *0.00776(V _f /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 40.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	1.4
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	32.5
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	876
v _p * highest directional split proportion ² (pc/h)	491
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	53.7
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	8.3
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	62.0
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	C
Volume to capacity ratio v/c v/c=V _p /3,200	0.27
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	218

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	804
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	6.7
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Chestnut Rd. to Balfour Rd. City of Brentwood Cumulative + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 806 veh/h Directional split 58 / 42 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	880
v_p * highest directional split proportion ² (pc/h)	510
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 40.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	33.9
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	878
v_p * highest directional split proportion ² (pc/h)	509
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	53.8
Adj. for directional distribution and no-passing zone, f_{dnp} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{dnp}$	53.8
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.28
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	110

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	403
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	3.2
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue E. Cypress Rd to Delta Rd. City of Brentwood Cumulative + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 424 veh/h Directional split 86 / 14 Peak-hour factor, PHF 0.92 No-passing zone 10 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.7
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.986
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	467
v _p * highest directional split proportion ² (pc/h)	402
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS= S _{FM} *0.00776(V _f /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.8
Average travel speed, ATS (mi/h) ATS=FFS*0.00776v _p -f _{np}	41.2
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	462
v _p * highest directional split proportion ² (pc/h)	397
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	33.4
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	9.3
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	42.7
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c v/c=V _p / 3,200	0.15
Peak 15-min veh-miles of travel,VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	173

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	636
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	4.2
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Delta Rd to Sunset Rd. City of Brentwood Cumulative + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 620 veh/h Directional split 60 / 40 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.996
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	677
v _p * highest directional split proportion ² (pc/h)	406
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS FFS=S _{FM} *0.00776(V _f /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS-0.00776v _p -f _{np}	40.4
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+ P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	675
v _p * highest directional split proportion ² (pc/h)	405
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	44.8
Adj. for directional distribution and no-passing zone, f _{dnp} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{dnp}	44.8
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c v/c=V _p /3,200	0.21
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	168

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	620
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	4.2
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 PM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Sunset Rd. to Chestnut Rd. City of Brentwood Cumulative + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 776 veh/h Directional split 54 / 46 Peak-hour factor, PHF 0.92 No-passing zone 0 % Trucks and Buses, P _T 2 % % Recreational vehicles, P _R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f _G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E _R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.996
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	847
v _p * highest directional split proportion ² (pc/h)	457
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S _{FM} mi/h	Base free-flow speed, BFFS _{FM} 50.0 mi/h
Observed volume, V _f veh/h	Adj. for lane width and shoulder width ³ , f _{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS = S _{FM} *0.00776(V _f /f _{HV}) mi/h	Adj. for access points, f _A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS (FSS=BFFS-f _{LS} -f _A) 45.7 mi/h
Adj. for no-passing zones, f _{np} (mi/h) (Exhibit 20-11)	0.0
Average travel speed, ATS (mi/h) ATS=FFS-0.00776v _p -f _{np}	39.1
Percent Time-Spent-Following	
Grade Adjustment factor, f _G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E _T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E _R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f _{HV} f _{HV} =1/(1+P _T (E _T -1)+P _R (E _R -1))	0.998
Two-way flow rate ¹ , v _p (pc/h) v _p =V/(PHF * f _G * f _{HV})	845
v _p * highest directional split proportion ² (pc/h)	456
Base percent time-spent-following, BPTSF(%) BPTSF=100(1-e ^{-0.000879v_p})	52.4
Adj. for directional distribution and no-passing zone, f _{d/np} (%)(Exh. 20-12)	0.0
Percent time-spent-following, PTSF(%) PTSF=BPTSF+f _{d/np}	52.4
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	B
Volume to capacity ratio v/c v/c=V _p /3,200	0.26
Peak 15-min veh-miles of travel, VMT ₁₅ (veh- mi) VMT ₁₅ = 0.25L ₁ (V/PHF)	316

Peak-hour vehicle-miles of travel, VMT ₆₀ (veh- mi) VMT ₆₀ =V*L ₁	1164
Peak 15-min total travel time, TT ₁₅ (veh-h) TT ₁₅ = VMT ₁₅ /ATS	8.1
Notes	
1. If V _p >= 3,200 pc/h, terminate analysis-the LOS is F.	
2. If highest directional split V _p >= 1,700 pc/h, terminated analysis-the LOS is F.	
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TWO-WAY TWO-LANE HIGHWAY SEGMENT WORKSHEET	
General Information	
Analyst Agency or Company Date Performed Analysis Time Period	Kimley-Horn and Associates 6/17/2008 AM Peak
Site Information	
Highway From/To Jurisdiction Analysis Year	Sellers Avenue Balfour Rd. and SR-4 City of Brentwood Cumulative + Project
Project Description:	
Input Data	
	<input type="checkbox"/> Class I highway <input checked="" type="checkbox"/> Class II highway Terrain <input checked="" type="checkbox"/> Level <input type="checkbox"/> Rolling Two-way hourly volume 828 veh/h Directional split 56 / 44 Peak-hour factor, PHF 0.92 No-passing zone 25 % Trucks and Buses, P_T 2 % % Recreational vehicles, P_R 0% Access points/ mi 12
Average Travel Speed	
Grade adjustment factor, f_G (Exhibit 20-7)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-9)	1.2
Passenger-car equivalents for RVs, E_R (Exhibit 20-9)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.996
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	904
v_p * highest directional split proportion ² (pc/h)	506
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed
Field Measured speed, S_{FM} mi/h	Base free-flow speed, $BFFS_{FM}$ 45.0 mi/h
Observed volume, V_f veh/h	Adj. for lane width and shoulder width ³ , f_{LS} (Exhibit 20-5) 1.3 mi/h
Free-flow speed, FFS $FFS=S_{FM}*0.00776(V_f/f_{HV})$ mi/h	Adj. for access points, f_A (Exhibit 20-6) 3.0 mi/h
	Free-flow speed, FFS $(FSS=BFFS-f_{LS}-f_A)$ 40.7 mi/h
Adj. for no-passing zones, f_{np} (mi/h) (Exhibit 20-11)	1.4
Average travel speed, ATS (mi/h) $ATS=FFS*0.00776v_p/f_{np}$	32.3
Percent Time-Spent-Following	
Grade Adjustment factor, f_G (Exhibit 20-8)	1.00
Passenger-car equivalents for trucks, E_T (Exhibit 20-10)	1.1
Passenger-car equivalents for RVs, E_R (Exhibit 20-10)	1.0
Heavy-vehicle adjustment factor, f_{HV} $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998
Two-way flow rate ¹ , v_p (pc/h) $v_p=V/(PHF * f_G * f_{HV})$	902
v_p * highest directional split proportion ² (pc/h)	505
Base percent time-spent-following, BPTSF(%) $BPTSF=100(1-e^{-0.000879v_p})$	54.7
Adj. for directional distribution and no-passing zone, $f_{d/np}$ (%)(Exh. 20-12)	8.1
Percent time-spent-following, PTSF(%) $PTSF=BPTSF+f_{d/np}$	62.8
Level of Service and Other Performance Measures	
Level of service, LOS (Exhibit 20-3 for Class I or 20-4 for Class II)	C
Volume to capacity ratio v/c $v/c=V_p/3,200$	0.28
Peak 15-min veh-miles of travel, VMT_{15} (veh- mi) $VMT_{15}=0.25L_1(V/PHF)$	225

Peak-hour vehicle-miles of travel, VMT_{60} (veh- mi) $VMT_{60}=V*L_1$	828
Peak 15-min total travel time, TT_{15} (veh-h) $TT_{15}=VMT_{15}/ATS$	7.0
Notes	
1. If $V_p >= 3,200$ pc/h, terminate analysis-the LOS is F.	
2. If highest directional split $V_p >= 1,700$ pc/h, terminated analysis-the LOS is F.	
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Appendix G:

*Analysis Worksheets for
Peak-Hour Signal Warrants*

Scenario Report
Scenario: Existing AM
Command: Default Command
Volume: Existing AM
Geometry: Default Geometry
Impact Fee: Default Impact Fee
Trip Generation: Default Trip Generation
Trip Distribution: Default Trip Distribution
Paths: Default Paths
Routes: Default Routes
Configuration: Default Configuration

Signal Warrant Summary Report
Intersection Base Met Future Met
[Del / Vol] [Del / Vol]
1 O'hara Avenue @ Southern Project Ro No / No ??? / ???


```

-----
Peak Hour Delay Signal Warrant Report
*****
Intersection #1 O'hara Avenue @ Southern Project Roadway
*****
Base Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R     L - T - R     L - T - R     L - T - R
-----|-----|-----|-----|
Control:    Uncontrolled   Uncontrolled   Stop Sign     Stop Sign
Lanes:      1 0 0 1 0         1 0 0 1 0         1 0 0 0 1         1 0 0 0 1
Final Vol.: 0 289 0         0 384 0         0 0 0 0         0 0 0 0
ApproachDel: xxxxxx         xxxxxx         xxxxxx         xxxxxx
-----|-----|-----|-----|

```

```

-----
Peak Hour Volume Signal Warrant Report [Urban]
*****
Intersection #1 O'hara Avenue @ Southern Project Roadway
*****
Base Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R     L - T - R     L - T - R     L - T - R
-----|-----|-----|-----|
Control:    Uncontrolled   Uncontrolled   Stop Sign     Stop Sign
Lanes:      1 0 0 1 0         1 0 0 1 0         1 0 0 0 1         1 0 0 0 1
Final Vol.: 0 289 0         0 384 0         0 0 0 0         0 0 0 0
-----|-----|-----|-----|
Major Street Volume:          673
Minor Approach Volume:        0
Minor Approach Volume Threshold: 544

```


Scenario Report
Scenario: Existing PM
Command: Default Command
Volume: Existing PM
Geometry: Default Geometry
Impact Fee: Default Impact Fee
Trip Generation: Default Trip Generation
Trip Distribution: Default Trip Distribution
Paths: Default Paths
Routes: Default Routes
Configuration: Default Configuration

Signal Warrant Summary Report
Intersection Base Met Future Met
[Del / Vol] [Del / Vol]
1 O'hara Avenue @ Southern Project Ro No / No ??? / ???

```

-----
                          Peak Hour Delay Signal Warrant Report
*****
Intersection #1 O'hara Avenue @ Southern Project Roadway
*****
Base Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R    L - T - R    L - T - R    L - T - R
-----|-----|-----|-----|
Control:    Uncontrolled  Uncontrolled  Stop Sign    Stop Sign
Lanes:      1 0 0 1 0      1 0 0 1 0      1 0 0 0 1      1 0 0 0 1
Final Vol.: 0 718 0      0 447 0      0 0 0 0      0 0 0 0
ApproachDel:xxxxx      xxxxxx      xxxxxx      xxxxxx
-----|-----|-----|-----|

```

```

-----
                          Peak Hour Volume Signal Warrant Report [Urban]
*****
Intersection #1 O'hara Avenue @ Southern Project Roadway
*****
Base Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R    L - T - R    L - T - R    L - T - R
-----|-----|-----|-----|
Control:    Uncontrolled  Uncontrolled  Stop Sign    Stop Sign
Lanes:      1 0 0 1 0      1 0 0 1 0      1 0 0 0 1      1 0 0 0 1
Final Vol.: 0 718 0      0 447 0      0 0 0 0      0 0 0 0
-----|-----|-----|-----|
Major Street Volume:      1165
Minor Approach Volume:    0
Minor Approach Volume Threshold: 308

```

Scenario Report

Scenario: EPAP AM
Command: Default Command
Volume: EPAP AM
Geometry: Default Geometry
Impact Fee: Default Impact Fee
Trip Generation: Default Trip Generation
Trip Distribution: Default Trip Distribution
Paths: Default Paths
Routes: Default Routes
Configuration: Default Configuration

Signal Warrant Summary Report

Intersection	Base Met		Future Met
	[Del / Vol]		[Del / Vol]
# 1 O'hara Avenue @ Southern Project Ro	No / No		??? / ???

Peak Hour Delay Signal Warrant Report

Intersection #1 O'hara Avenue @ Southern Project Roadway

Base Volume Alternative: Peak Hour Warrant NOT Met

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Lanes, Final Vol., and ApproachDel.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #1 O'hara Avenue @ Southern Project Roadway

Base Volume Alternative: Peak Hour Warrant NOT Met

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Lanes, Final Vol., Major Street Volume, Minor Approach Volume, and Minor Approach Volume Threshold.

Scenario Report

Scenario: EPAP PM
Command: Default Command
Volume: EPAP PM
Geometry: Default Geometry
Impact Fee: Default Impact Fee
Trip Generation: Default Trip Generation
Trip Distribution: Default Trip Distribution
Paths: Default Paths
Routes: Default Routes
Configuration: Default Configuration

Signal Warrant Summary Report
Intersection Base Met Future Met
[Del / Vol] [Del / Vol]
1 O'hara Avenue @ Southern Project Ro No / No ??? / ???

Peak Hour Delay Signal Warrant Report

Intersection #1 O'hara Avenue @ Southern Project Roadway

Base Volume Alternative: Peak Hour Warrant NOT Met

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Lanes, Final Vol., and ApproachDel.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #1 O'hara Avenue @ Southern Project Roadway

Base Volume Alternative: Peak Hour Warrant NOT Met

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Lanes, Final Vol., Major Street Volume, Minor Approach Volume, and Minor Approach Volume Threshold.

Scenario Report

Scenario: EPAP + PP AM
Command: Default Command
Volume: EPAP + PP AM
Geometry: Default Geometry
Impact Fee: Default Impact Fee
Trip Generation: Default Trip Generation
Trip Distribution: Default Trip Distribution
Paths: Default Paths
Routes: Default Routes
Configuration: Default Configuration

Signal Warrant Summary Report
Intersection Base Met Future Met
[Del / Vol] [Del / Vol]
1 O'hara Avenue @ Southern Project Ro Yes / No ??? / ???

Peak Hour Delay Signal Warrant Report

 Intersection #1 O'hara Avenue @ Southern Project Roadway

Base Volume Alternative: Peak Hour Warrant Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Lanes:	1 0 0 1 0	1 0 0 1 0	1 0 0 0 1	1 0 0 0 1
Final Vol.:	130 289 130	173 384 173	83 0 110	110 0 83
ApproachDel:	xxxxxx	xxxxxx	108.0	255.1

Approach[eastbound] [lanes=2] [control=Stop]

Signal Warrant Rule #1: [vehicle-hours=5.8]

SUCCEED - Vehicle-hours >= 5 for two or more lane approach.

Signal Warrant Rule #2: [approach volume=193]

SUCCEED - Approach volume >= 150 for two or more lane approach.

Signal Warrant Rule #3: [approach count=4][total volume=1665]

SUCCEED - Total volume greater than or equal to 800 for intersection with four or more approaches.

Approach[westbound] [lanes=2] [control=Stop]

Signal Warrant Rule #1: [vehicle-hours=13.7]

SUCCEED - Vehicle-hours >= 5 for two or more lane approach.

Signal Warrant Rule #2: [approach volume=193]

SUCCEED - Approach volume >= 150 for two or more lane approach.

Signal Warrant Rule #3: [approach count=4][total volume=1665]

SUCCEED - Total volume greater than or equal to 800 for intersection with four or more approaches.

Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #1 O'hara Avenue @ Southern Project Roadway

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Lanes:	1 0 0 1 0	1 0 0 1 0	1 0 0 0 1	1 0 0 0 1
Final Vol.:	130 289 130	173 384 173	83 0 110	110 0 83

Major Street Volume: 1279

Minor Approach Volume: 193

Minor Approach Volume Threshold: 268

Scenario Report

Scenario: EPAP + PP PM
Command: Default Command
Volume: EPAP + PP PM
Geometry: Default Geometry
Impact Fee: Default Impact Fee
Trip Generation: Default Trip Generation
Trip Distribution: Default Trip Distribution
Paths: Default Paths
Routes: Default Routes
Configuration: Default Configuration

Signal Warrant Summary Report
Intersection Base Met Future Met
[Del / Vol] [Del / Vol]
1 O'hara Avenue @ Southern Project Ro Yes / Yes ??? / ???

Peak Hour Delay Signal Warrant Report

Intersection #1 O'hara Avenue @ Southern Project Roadway

Base Volume Alternative: Peak Hour Warrant Met

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Lanes, and Final Vol. ApproachDel values are 2116.4 and 428.6.

Approach[eastbound] [lanes=2] [control=Stop]
Signal Warrant Rule #1: [vehicle-hours=191.1]
SUCCEED - Vehicle-hours >= 5 for two or more lane approach.
Signal Warrant Rule #2: [approach volume=325]
SUCCEED - Approach volume >= 150 for two or more lane approach.
Signal Warrant Rule #3: [approach count=4][total volume=2253]
SUCCEED - Total volume greater than or equal to 800 for intersection with four or more approaches.

Approach[westbound] [lanes=2] [control=Stop]
Signal Warrant Rule #1: [vehicle-hours=38.7]
SUCCEED - Vehicle-hours >= 5 for two or more lane approach.
Signal Warrant Rule #2: [approach volume=325]
SUCCEED - Approach volume >= 150 for two or more lane approach.
Signal Warrant Rule #3: [approach count=4][total volume=2253]
SUCCEED - Total volume greater than or equal to 800 for intersection with four or more approaches.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #1 O'hara Avenue @ Southern Project Roadway

Base Volume Alternative: Peak Hour Warrant Met

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Uncontrolled, Stop Sign), Lanes, and Final Vol. ApproachDel values are 1603, 325, and 171.

Major Street Volume: 1603
Minor Approach Volume: 325
Minor Approach Volume Threshold: 171


```

-----
                          Peak Hour Delay Signal Warrant Report
*****
Intersection #1 O'hara Avenue @ Southern Project Roadway
*****
Base Volume Alternative: Peak Hour Warrant NOT Met
-----
Approach:  North Bound      South Bound      East Bound      West Bound
Movement:  L - T - R      L - T - R      L - T - R      L - T - R
-----
Control:   Uncontrolled    Uncontrolled    Stop Sign       Stop Sign
Lanes:     1 0 1 1 0      1 0 1 1 0      1 0 0 0 1      1 0 0 0 1
Final Vol.: 0 198      0 0 579 0      0 0 0 0 0      0 0 0 0 0
ApproachDel: xxxxxx      xxxxxx      xxxxxx      xxxxxx
-----

```

```

-----
                          Peak Hour Volume Signal Warrant Report [Urban]
*****
Intersection #1 O'hara Avenue @ Southern Project Roadway
*****
Base Volume Alternative: Peak Hour Warrant NOT Met
-----
Approach:  North Bound      South Bound      East Bound      West Bound
Movement:  L - T - R      L - T - R      L - T - R      L - T - R
-----
Control:   Uncontrolled    Uncontrolled    Stop Sign       Stop Sign
Lanes:     1 0 1 1 0      1 0 1 1 0      1 0 0 0 1      1 0 0 0 1
Final Vol.: 0 198      0 0 579 0      0 0 0 0 0      0 0 0 0 0
-----
Major Street Volume:          777
Minor Approach Volume:        0
Minor Approach Volume Threshold: 483
-----

```


Scenario: Scenario Report
Cumulative PM
Command: Default Command
Volume: Cumulative PM
Geometry: Default Geometry
Impact Fee: Default Impact Fee
Trip Generation: Default Trip Generation
Trip Distribution: Default Trip Distribution
Paths: Default Paths
Routes: Default Routes
Configuration: Default Configuration

Intersection	Signal Warrant	Summary Report	Future Met
		Base Met	[Del / Vol]
# 1 O'hara Avenue @ Southern Project Ro		[Del / Vol]	???
	No / No		???

```

-----
                          Peak Hour Delay Signal Warrant Report
*****
Intersection #1 O'hara Avenue @ Southern Project Roadway
*****
Base Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R    L - T - R    L - T - R    L - T - R
-----|-----|-----|-----|
Control:     Uncontrolled  Uncontrolled  Stop Sign    Stop Sign
Lanes:       1 0 1 1 0    1 0 1 1 0    1 0 0 0 1    1 0 0 0 1
Final Vol.:  0 543  0    0 359  0    0 0 0 0    0 0 0 0
ApproachDel:  xxxxxx    xxxxxx    xxxxxx    xxxxxx
-----|-----|-----|-----|

```

```

-----
                          Peak Hour Volume Signal Warrant Report [Urban]
*****
Intersection #1 O'hara Avenue @ Southern Project Roadway
*****
Base Volume Alternative: Peak Hour Warrant NOT Met
-----|-----|-----|-----|
Approach:   North Bound   South Bound   East Bound   West Bound
Movement:   L - T - R    L - T - R    L - T - R    L - T - R
-----|-----|-----|-----|
Control:     Uncontrolled  Uncontrolled  Stop Sign    Stop Sign
Lanes:       1 0 1 1 0    1 0 1 1 0    1 0 0 0 1    1 0 0 0 1
Final Vol.:  0 543  0    0 359  0    0 0 0 0    0 0 0 0
-----|-----|-----|-----|
Major Street Volume:          902
Minor Approach Volume:        0
Minor Approach Volume Threshold: 418

```

 Scenario Report
 Scenario: Cumulative + PP AM
 Command: Default Command
 Volume: Cumulative + PP AM
 Geometry: Default Geometry
 Impact Fee: Default Impact Fee
 Trip Generation: Default Trip Generation
 Trip Distribution: Default Trip Distribution
 Paths: Default Paths
 Routes: Default Routes
 Configuration: Default Configuration

 Signal Warrant Summary Report
 Intersection Base Met Future Met
 [Del / Vol] [Del / Vol]
 # 1 O'hara Avenue @ Southern Project Ro Yes / No ??? / ???

Peak Hour Delay Signal Warrant Report

Intersection #1 O'hara Avenue @ Southern Project Roadway

Base Volume Alternative: Peak Hour Warrant Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Lanes:	1 0 1 1 0	1 0 1 1 0	1 0 0 0 1	1 0 0 0 1
Final Vol.:	76 195 76	227 579 227	49 0 144	144 0 49
ApproachDel:	xxxxxx	xxxxxx	37.1	246.9

Approach[eastbound] [lanes=2] [control=Stop]
 Signal Warrant Rule #1: [vehicle-hours=2.0]
 FAIL - Vehicle-hours less than 5 for two or more lane approach.
 Signal Warrant Rule #2: [approach volume=193]
 SUCCEED - Approach volume >= 150 for two or more lane approach.
 Signal Warrant Rule #3: [approach count=4][total volume=1766]
 SUCCEED - Total volume greater than or equal to 800 for intersection with four or more approaches.

Approach[westbound] [lanes=2] [control=Stop]
 Signal Warrant Rule #1: [vehicle-hours=13.2]
 SUCCEED - Vehicle-hours >= 5 for two or more lane approach.
 Signal Warrant Rule #2: [approach volume=193]
 SUCCEED - Approach volume >= 150 for two or more lane approach.
 Signal Warrant Rule #3: [approach count=4][total volume=1766]
 SUCCEED - Total volume greater than or equal to 800 for intersection with four or more approaches.

Peak Hour Volume Signal Warrant Report [Urban]

Intersection #1 O'hara Avenue @ Southern Project Roadway

Base Volume Alternative: Peak Hour Warrant NOT Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Lanes:	1 0 1 1 0	1 0 1 1 0	1 0 0 0 1	1 0 0 0 1
Final Vol.:	76 195 76	227 579 227	49 0 144	144 0 49
Major Street Volume:	1380			
Minor Approach Volume:	193			
Minor Approach Volume Threshold:	236			

Major Street Volume: 1380
 Minor Approach Volume: 193
 Minor Approach Volume Threshold: 236

Scenario Report
Scenario: Cumulative + PP PM
Command: Default Command
Volume: Cumulative + PP PM
Geometry: Default Geometry
Impact Fee: Default Impact Fee
Trip Generation: Default Trip Generation
Trip Distribution: Default Trip Distribution
Paths: Default Paths
Routes: Default Routes
Configuration: Default Configuration

Signal Warrant Summary Report
Intersection Base Met Future Met
 [Del / Vol] [Del / Vol]
1 O'hara Avenue @ Southern Project Ro Yes / Yes ??? / ???

 Peak Hour Delay Signal Warrant Report

 Intersection #1 O'hara Avenue @ Southern Project Roadway

Base Volume Alternative: Peak Hour Warrant Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Lanes:	1 0 1 1 0	1 0 1 1 0	1 0 0 0 1	1 0 0 0 1
Final Vol.:	128 511 128	90 359 90	191 0 134	134 0 191
ApproachDel:	xxxxxx	xxxxxx	286.1	118.7

 Approach[eastbound] [lanes=2] [control=Stop]
 Signal Warrant Rule #1: [vehicle-hours=25.8]
 SUCCEED - Vehicle-hours >= 5 for two or more lane approach.
 Signal Warrant Rule #2: [approach volume=325]
 SUCCEED - Approach volume >= 150 for two or more lane approach.
 Signal Warrant Rule #3: [approach count=4][total volume=1956]
 SUCCEED - Total volume greater than or equal to 800 for intersection
 with four or more approaches.

 Approach[westbound] [lanes=2] [control=Stop]
 Signal Warrant Rule #1: [vehicle-hours=10.7]
 SUCCEED - Vehicle-hours >= 5 for two or more lane approach.
 Signal Warrant Rule #2: [approach volume=325]
 SUCCEED - Approach volume >= 150 for two or more lane approach.
 Signal Warrant Rule #3: [approach count=4][total volume=1956]
 SUCCEED - Total volume greater than or equal to 800 for intersection
 with four or more approaches.

 Peak Hour Volume Signal Warrant Report [Urban]

 Intersection #1 O'hara Avenue @ Southern Project Roadway

Base Volume Alternative: Peak Hour Warrant Met

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Lanes:	1 0 1 1 0	1 0 1 1 0	1 0 0 0 1	1 0 0 0 1
Final Vol.:	128 511 128	90 359 90	191 0 134	134 0 191

 Major Street Volume: 1306
 Minor Approach Volume: 325
 Minor Approach Volume Threshold: 259

APPENDIX F



Kimley-Horn
and Associates, Inc.

August 11, 2011

Mr. Nick Pappani
Raney Planning & Management, Inc.
1501 Sports Drive
Sacramento, California 95834

☐
Suite 200
11919 Foundation Place
Gold River, California
95670

Re: Trip Generation Comparison
Brentwood Boulevard Specific Plan Update
Brentwood, California

Dear Mr. Pappani:

As requested, we have prepared a trip generation comparison for the above referenced project. The purpose of this effort is to quantify the change in project trips resulting from the new, 2011 land use plan. The baseline for comparison is the previously completed study from 2008¹. Table 1 provides a summary of the baseline (2008) and new (2011) land uses for the proposed project. As depicted in Table 1, the 2011 project generally reduces the overall project development size, while only increasing the number of single-family units (+61) and commercial (+308.2 ksf) land uses

Table 1 – Project Land Use Comparison (2008, 2008-Scortino Ranch, and 2011)

Land Use (ITE Code)	Size (2008)	Size (2008 - Scortino)	Delta	Size (2011)	Delta (2011 vs Full 2008)	Delta (2011 vs 2008-Scortino)
General Light Industrial (110)	539.7 ksf	539.7 ksf	0.0 ksf	333.8 ksf	(205.9) ksf	(205.9) ksf
Single-Family Detached Housing (210)	421 units	421 units	0 units	482 units	61 units	61 units
Apartment (220)	1825 units	1825 units	0 units	1510 units	(315) units	(315) units
City Park (411)	8.1 acres	8.1 acres	0.0 acres	0.0 acres	(8.1) acres	(8.1) acres
General Office Building (710)	1940.7 ksf	1799.9 ksf	(140.8) ksf	417.3 ksf	(1523.4) ksf	(1382.6) ksf
Shopping Center (820)	381.4 ksf	271.4 ksf	(110.0) ksf	689.6 ksf	308.2 ksf	418.2 ksf

Furthermore, the baseline (2008) project was confirmed to have included the Scortino Ranch project site within the proposed project boundary. As such, the 2008 project included approximately 110 ksf of commercial and 141 ksf of office land uses. Table 1 also includes data from which it is possible to compare the 2011 project against the 2008 project without the Scortino Ranch project area (2008-Scortino).

Table 2 through Table 4 provide detailed trip generation characteristics for each of the project conditions (2008, 2008-Scortino, and 2011).

¹ *Traffic Impact Analysis, Brentwood Boulevard Specific Plan, Kimley-Horn and Associates, Inc., April 14, 2008*

Table 2 – Proposed Project Trip Generation (Full 2008)

Land Use (ITE Code)	Size	Daily Trips	AM Peak-Hour				PM Peak-Hour					
			Total Trips	IN		OUT		Total Trips	IN		OUT	
				%	Trips	%	Trips		%	Trips	%	Trips
General Light Industrial (110)	539.7 ksf	3,930	548	88%	482	12%	66	608	12%	73	88%	535
Single-Family Detached Housing (210)	421 units	3,902	304	25%	76	75%	228	391	63%	246	37%	145
Apartment (220)	1825 units	11,119	898	20%	180	80%	718	1,021	65%	664	35%	357
City Park (411)	8.1 acres	13	0	0%	0	100%	0	0	0%	0	100%	0
General Office Building (710)	1940.7 ksf	13,089	2,012	88%	1,770	12%	241	2,252	17%	383	83%	1,869
Shopping Center (820)	381.4 ksf	16,212	349	61%	213	39%	136	1,515	48%	727	52%	788
Subtotal New Trips (Full 2008):		48,264	4,111		2,721		1,390	5,788		2,093		3,695

Source: Trip Generation, 7th Edition, ITE.

Table 3 – Proposed Project Trip Generation (2008-Scortino Ranch)

Land Use (ITE Code)	Size	Daily Trips	AM Peak-Hour				PM Peak-Hour					
			Total Trips	IN		OUT		Total Trips	IN		OUT	
				%	Trips	%	Trips		%	Trips	%	Trips
General Light Industrial (110)	539.7 ksf	3,930	548	88%	482	12%	66	608	12%	73	88%	535
Single-Family Detached Housing (210)	421 units	3,902	304	25%	76	75%	228	391	63%	246	37%	145
Apartment (220)	1825 units	11,119	898	20%	180	80%	718	1,021	65%	664	35%	357
City Park (411)	8.1 acres	13	0	0%	0	100%	0	0	0%	0	100%	0
General Office Building (710)	1799.9 ksf	12,351	1,894	88%	1,667	12%	227	2,095	17%	356	83%	1,739
Shopping Center (820)	271.4 ksf	12,996	285	61%	174	39%	111	1,210	48%	581	52%	629
Subtotal New Trips (2008-Scortino):		44,310	3,928		2,578		1,351	5,325		1,920		3,405

Source: Trip Generation, 7th Edition, ITE
Note: Shaded cells identify Scortino Ranch land uses and changes to the 2008 project.

Table 4 – Proposed Project Trip Generation (2011)

Land Use (ITE Code)	Size	Daily Trips	AM Peak-Hour				PM Peak-Hour					
			Total Trips	IN		OUT		Total Trips	IN		OUT	
				%	Trips	%	Trips		%	Trips	%	Trips
General Light Industrial (110)	333.8 ksf	2,392	305	88%	268	12%	37	314	12%	38	88%	276
Single-Family Detached Housing (210)	482 units	4,419	347	25%	87	75%	260	441	63%	278	37%	163
Apartment (220)	1510 units	9,225	744	20%	149	80%	595	848	65%	551	35%	297
City Park (411)	0.0 acres	0	0	0%	0	100%	0	0	0%	0	100%	0
General Office Building (710)	417.3 ksf	4,008	588	88%	518	12%	71	546	17%	93	83%	453
Shopping Center (820)	689.6 ksf	23,825	499	61%	304	39%	194	2,239	48%	1,075	52%	1,164
Subtotal New Trips (2011):		43,869	2,482		1,325		1,157	4,389		2,035		2,354
Subtotal New Trips from Table 2 (Full 2008):		48,264	4,111					5,788				
Delta (2011 vs. Full 2008):		(4,395)	(1,629)					(1,399)				
Subtotal New Trips from Table 3 (2008-Scortino):		44,310	3,928					5,325				
Delta (2011 vs. 2008-Scortino):		(441)	(1,447)					(937)				

Source: Trip Generation, 7th Edition, ITE.



As summarized in Table 4, the new (2011) project is anticipated to generate fewer trips (Daily, AM, and PM) when compared to the 2008 project, both with and without the Scortino Ranch project.

The new (2011) project is anticipated to generate approximately 10 percent fewer daily trips, approximately 40 percent fewer AM peak-hour, and approximately 25 percent fewer PM peak-hour trips than the baseline (2008) project. When compared to the baseline project without Scortino Ranch, the proposed project is anticipated to generate approximately the same daily trips, approximately 35 percent fewer AM peak-hour trips, and approximately 15 percent fewer PM peak-hour trips.

Subsequent discussions with the City will determine if the magnitude of the trip reduction associated with the new (2011) project warrants revisions to, or an update of the previously completed traffic study for the proposed project¹.

Please contact me at (916) 878-7142 if you have any questions or require additional information.

Very truly yours,

KIMLEY-HORN AND ASSOCIATES, INC.

A handwritten signature in black ink that reads "Matthew D. Weir". The signature is written in a cursive, flowing style.

Matthew D. Weir, PE, TE, PTOE
PE No. C70216 & TR2424